

# Appendix K-1

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## Preliminary Hydrology Study



# **PRELIMINARY HYDROLOGY STUDY**

**For:**

## **Meridian Park Upper Plateau**

***Project Site Location/Address:***  
Cactus Avenue & Barton Street  
Riverside, CA

***Prepared For:***  
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**February 7, 2022**

Project No. 20-750

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## Section I

## Introduction

The following preliminary hydrology study has been prepared for the development of the Meridian Park Upper Plateau area located to the south of Alessandro Boulevard, East of Trailwind Road, North of Orange Terrace Parkway and west of Meridian Parkway. The development is located in the March Joint Powers Authority Jurisdictional Area of Riverside County and will connect to Cactus Avenue to the west, Barton Street to the North and South and Brown Street to the north. The subject site is approximately 722 acres of which 370 acres will be disturbed. The general location of the site is illustrated on the Vicinity Map, (see Figure 1 in Appendix A of this report).

## Section II

## Methodology

The peak storm discharge for the drainage sub areas were calculated using the Riverside County Hydrology Manual. Advanced Engineering Software (AES) implementing the rational method was used to calculate peak flows for the 100 year storm entering the proposed public storm drains. Water Surface Pressure Gradient (WSPG) by CivilDesign was used for hydraulic calculations pertaining to the proposed storm drain lines. Finally, FlowMaster Hydraulic Calculator Software by Bentley Systems was used to evaluate the efficiency of catch basins located in the public streets. Since this study is based on estimated developed conditions, hydrology for on-site parcels was based on a commercial development land cover at 90% impervious and 10% pervious. A relatively short flow path was estimated for each parcel as the development of each parcel is not currently known at this time. This results in a shorter time of concentration and therefore a more conservative result related to flow rate. A soil type of BC was assigned to the project site based on the Riverside County Flood Control and Water Conservation District Hydrology Manual Hydrologic Soils Group Map Plate C-1.16 (see portion of map in Appendix A). In such cases where a dual soil designation has been assigned, the more conservative value is recommended for use. For calculation purposes, a soil type of C was used for the project site. Soil group C is defined as soils having slow infiltration rates when thoroughly wetted and consisting chiefly of silty-loam soils with a layer that impedes downward movement of water, or soils with moderately fine to fine texture. These soils have a slow rate of water transmission.

## Section III

## Project Description

### Existing Site conditions

The existing site is approximately 728 acres of land consisting of rolling hills with fair open brush (soft wood shrubs) within the Santa Ana River watershed boundary. The site is surrounded by residential development to the north, east and south and commercial development to the west.

The site has been broken down into watersheds designated as 1-17 to clearly highlight how stormwater reaches the Santa Ana River. The table below shows watershed acreages along with explanations of outlet locations downstream. The watersheds flow into both County of Riverside and City of Riverside facilities.



Watershed	Acreage	Description	Downstream Destination
3	21.37	Surface flows to current dead end on Barton Street. No inlet structure, stormwater continues to flow down Barton Street into residential development.	Stormwater continues to flow north in residential development and outlets into Sycamore Canyon Wilderness Park. Creeks combine downstream and flow into Sycamore Dam to the northwest. Overflow from Sycamore Dam flows northwest to Santa Ana River.
4	4.01	Surface flows to residential development to the north. No drain inlet or pickup location.	
5	14.31	Surface flows to residential development to the north. No drain inlet or pickup location.	
5a	3.57	Surface flows to Vista Grand Drive. No drain inlet or pickup location.	
6	3.87	Surface flows to residential development to the north. No drain inlet or pickup location.	
7	38.16	Surface flows to current dead end on Alexander Street. No inlet structure, stormwater continues to flow down Alexander Street into residential development.	
7a	17.21	Surface flows to residential development to the north. No drain inlet or pickup location.	
18a	1.12	Surface flows to residential development to the north. No drain inlet or pickup location.	
18b	8.60	Surface flows to residential development to the north. No drain inlet or pickup location.	
18c	1.25	Surface flows to residential development to the north. No drain inlet or pickup location.	
8	72.12	Surface flows to inlet structure to a 42" storm drain bypassing an existing development. Outlets and surface flows towards the SW corner of Meridian Parkway and Alessandro Boulevard. Enters storm drain, flows under Meridian Parkway and outlets at North Detention Basin (a part of the 1 <sup>st</sup> phase of Meridian Business Park development).	Overflow from the detention basin flows Northwest through Sycamore Canyon Wilderness Park to Sycamore Dam to the northwest. Overflow from Sycamore Dam flows northwest Santa Ana River.
9	81.47	Surface flows to inlet structure and enters dual 36" storm drain pipes bypassing buildings to the west of Meridian Parkway. Storm drains continue to flow east under Meridian Parkway and into North Detention Basin (a part of the 1 <sup>st</sup> phase of Meridian Business Park development).	
10	31.11	Flows to inlet structure and enters 54" storm drain south of Cactus Avenue. Storm drain continues to flow east under Meridian Parkway and into North Detention Basin (a part of the 1 <sup>st</sup> phase of Meridian Business Park development).	
11	21.12	Flows to inlet structure and enters 30" storm drain that flows east under Authority Way and north under Meridian Parkway. Storm drain continues to flow east and outlets into North Detention Basin (a part of the 1 <sup>st</sup> phase of Meridian Business Park development).	
12	34.26	Flows to inlet structure and through existing residential development to the southeast and outlets to creek flowing to the east. Picked up by inlet structure to a 30" storm drain that flows east under Authority Way and north under Meridian Parkway. Storm drain continues to flow east and outlets into North Detention Basin (a part of the 1 <sup>st</sup> phase of Meridian Business Park development).	



13	11.00	Flows east through existing residential development and outlets near Bakal Drive and Orchard Park Drive. Flows southeast to regional Lot E/49 detention basin.	Overflow from Lot E/49 basin crosses under 215 freeway and enters Perris Valley Storm Drain. Further downstream stormwater passes through Canyon Lake, Lake Elsinore, and the Santa Ana River.  Creeks travel northwest towards Alessandro Dam. Overflow continues to flow through a creek northwest to Mary Street Dam. Overflow from Mary Street Dam continues underground and outlets at Santa Ana River.  Creeks travel northwest towards Alessandro Dam. Overflow continues to flow through a creek northwest to Mary Street Dam. Overflow from Mary Street Dam continues underground and outlets at Santa Ana River.
14	92.15	Flows east to inlet structure and enters 48" RCP flowing north under Barton Street. Storm drain turns to the west and outlets to existing creek that is a part of Watershed 17.	
15	15.74	Flows east to inlet structure and enters 48" RCP flowing north under Barton Street. Storm drain turns to the west and outlets to existing creek that is a part of Watershed 17.	
16	77.66	Flows to inlet structure and enters 57" storm drain through residential development flowing west and outlets to existing creek flowing Northwest.	
17	14.93	Surface flows to creek flowing northwest (see watersheds 14 & 15).	
1	15.66	Flows to inlet structure east of Cairn Street into 30" storm drain flowing east under Northrop Drive and north on Mission Grove Drive. Stormwater outlets at creek south of White Dove Lane.	
2	147.94	Flows to inlet structure at the top of Moray Court to a 72" storm drain. Stormwater flows west under Botany Bay Road and outlets south of Alice Springs Place into creek.	

Refer to the "Existing Hydrology Zones Map" in Appendix B for an illustration of the existing drainage patterns. Watersheds 2, 3, 4, 5, 6, 7, 8, 9, 14 and 16 will be updated per the Proposed Site Conditions highlighted in the next section. To better understand the outlets of these watersheds they are discussed in further detail below.

Watershed 2 converges towards a 2' deep ¼ ton grouted rip rap channel to the east of Moray Court leading to a concrete pad and headwall connected to an 84" reinforced concrete pipe (D-800) labeled as Line C per City of Riverside project no. D-636A. The line has a Q<sub>100</sub> capacity of 412 cfs and a Q<sub>10</sub> capacity of 259 cfs from the headwall structure. Line C travels west under Moray Court, transitioning to a 72" RCP, before flowing north under Coatbridge Drive and west on Botany Bay Road picking up additional stormwater from the streets and transitioning back to an 84" RCP. Line C then turns to the southwest and outlets into a creek to the south of Alice Springs Place flowing through the residential development and under Mission Grove Parkway and Trautwein Road. Downstream the creek inlets to Alessandro Dam, further downstream to Mary Street Dam and eventually the Santa Ana River.

Watershed 3 converges to the current dead end at Barton Street. Stormwater surface flows on to Barton Street and gutter flows to the north through the existing residential development before heading east on Alessandro Boulevard and entering the first existing catch basin. The 18" storm drain connected to the catch basin is designed for a Q<sub>100</sub> of 24.31 cfs per City of Riverside Storm drawing number D-579. After crossing under Alessandro Boulevard flowing to the north and converging with a 33" RCP (D-1000) flowing east, stormwater outlets through a headwall with a 10'x25' 2.7' thick rip rap pad into Sycamore Canyon Wilderness Park. The



creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Damn to the north, and later downstream the Santa Ana River.

Watershed 4 converges to a low spot leading to a concrete channel passing between 2 residential lots. Stormwater then flows north through a parkway drain into the streets of the existing residential development. Stormwater then gutter flows to the north under Alessandro Boulevard and into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Damn to the north, and later downstream the Santa Ana River.

Watershed 5 converges to a low spot leading to a concrete channel passing between 2 residential lots. Stormwater then flows north through a parkway drain into the streets of the existing residential development. Stormwater then gutter flows to the north under Alessandro Boulevard and into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Damn to the north, and later downstream the Santa Ana River.

Watershed 5a converges to the current dead end at Vista Grande Drive. Stormwater surface flows on to Vista Grande Drive and gutter flows to the north through the existing residential development before heading east on Alessandro Boulevard and entering the first existing catch basin to the east. The 36" storm drain connected to the catch basin is designed for a  $Q_{100}$  of 57.4 cfs per City of Riverside street drawing number R-3068. After crossing under Alessandro Boulevard, stormwater outlets through a headwall with a 32" ¼ ton thick rip rap pad into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Damn to the north, and later downstream the Santa Ana River.

Watershed 6 converges to a low spot leading to a concrete channel passing between 2 residential lots. Stormwater then flows north through a parkway drain into the streets of the existing residential development. Stormwater then gutter flows to the north under Alessandro Boulevard and into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Damn to the north, and later downstream the Santa Ana River.

Watershed 7 converges to a low spot surface flowing onto Alexander Street and into the existing residential development. Stormwater then gutter flows to the north under Alessandro Boulevard and into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Damn to the north, and later downstream the Santa Ana River.

Watershed 7a converges to a low spot leading to a concrete channel passing between 2 residential lots. Stormwater then flows north through a parkway drain into the streets of the existing residential development. Stormwater then gutter flows to the north under Alessandro Boulevard and into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Damn to the north, and later downstream the Santa Ana River.

Watershed 8 converges at a headwall structure built for Alessandro Commerce Center (County of Riverside Plot Plan No. 25422). The headwall follows Caltrans Std. D90 and features 6" cobble rip-rap at the inlet. The headwall is connected to a 42" RCP storm drain that per the





approved hydrology report has been designed for a  $Q_{100}$  of 136.74 cfs. After bypassing the Alessandro Commerce Center, the 42" storm drain outlets to 6" cobble rip rap and flows northwest towards the southwest corner of Alessandro Boulevard and Meridian Parkway. Stormwater enters a wingtype headwall leading to a 60" RCP designed for a  $Q_{100}$  of 183 cfs per MJPA project no. 4-0530 & drawing no. 4-821. Stormwater flows to the east and outlets at the LLMD maintained Lot 69 North Detention Basin. The basin inlets into a 10'x6' RCB maintained by Riverside County Flood Control crossing under Van Buren Boulevard which outlets to the north and outlets to an existing creek crossing under Sycamore Canyon Boulevard and entering into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Dam to the north, and later downstream the Santa Ana River.

Watershed 9 converges to a headwall and wingwall with trash rack per MJPA file no. MJ-092 public storm drain plans. The headwall leads to a dual 36" HDPE storm drain that bypasses the existing warehouse site. The dual 36" storm drains have been designed for a  $Q_{100}$  of 94 cfs at the headwall structure where Watershed 9 converges. Downstream the dual 36" storm drains outlet through a wingtype headwall to a rip rap lined channel that crosses through an EMWD easement. Downstream to the west of Meridian Parkway stormwater once again enters a wingtype headwall connected to a 54" RCP sized to handle a  $Q_{100}$  of 131 cfs per MJPA project no. 4-0530 & drawing no. 4-821. Stormwater flows to the east and outlets at the LLMD maintained Lot 69 North Detention Basin. The basin inlets into a 10'x6' RCB maintained by Riverside County Flood Control crossing under Van Buren Boulevard which outlets to the north and outlets to an existing creek crossing under Sycamore Canyon Boulevard and entering into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Dam to the north, and later downstream the Santa Ana River.

Watershed 14 converges to a grated catch basin connected to a 78" RCP which has been designed for a developed  $Q_{100}$  of 287.5 cfs and a native  $Q_{100}$  of 192.7 cfs per City of Riverside Public Storm Drain Plan No. D-748. The 78" storm drain line flows to the north under Barton Road before turning to the west and flowing under the existing church parking lot. The storm drain outlets via an energy dissipator – impact basin with grouted rip rap to an existing creek being a part of Watershed 17 (currently proposed to be undeveloped). The creek travels through the existing residential development to the west flowing under Trautwein Road. Further downstream the creek inlets to Alessandro Dam, Mary Street Dam and eventually the Santa Ana River.

Watershed 16 converges to a headwall protected by a sloped protection barrier consisting of ¼ ton rip-rap 3.5' thick. The headwall is connected to a 57" RCP (D-1200) and has been designed for a  $Q_{100}$  of 220.3 and a  $Q_{10}$  of 136.6 cfs. The storm drain continues west under Webster Road and outlets in a creek to the west of Ralston Place. The creek travels through the existing residential development to the west flowing under Trautwein Road. Further downstream the creek inlets to Alessandro Dam, Mary Street Dam and eventually the Santa Ana River.

Watersheds 18a, 18b, and 18c surface flow to existing residential lots located on Camino Del Sol. Stormwater then gutter flows to the north under Alessandro Boulevard and into Sycamore Canyon Wilderness Park. The creeks within Sycamore Canyon Wilderness Park combine and flow into Sycamore Dam to the north, and later downstream the Santa Ana River.



## Proposed Site Conditions

### Overall

The proposed development will consist of the construction of public streets, utilities, associated landscaping areas and mass graded pads. The proposed condition was designed to balance the watersheds as close to existing as possible. The public storm drain design along with conceptual grading for the project was used as a tool to reach this standard. Therefore, the development was once again broken up into 17 distinct watersheds each sharing the same outlets as existing. The following table summarizes both the existing and proposed areas of each watershed.

<b>Area Summaries</b>			
<b>Watershed</b>	<b>Existing (Acres)</b>	<b>Proposed (Acres)</b>	<b>Percent Change</b>
<b>1 (undisturbed)</b>	15.66	15.66	0.00%
<b>2</b>	147.94	148.88	+0.6%
<b>3</b>	21.37	21.26	-0.51%
<b>4</b>	4.01	3.09	-22.9%
<b>5</b>	14.31	15.00	+4.8%
<b>5a</b>	3.57	2.53	-29.1%
<b>6</b>	3.87	2.95	-23.8%
<b>7</b>	38.16	50.17	+31.5%
<b>7a</b>	17.21	6.11	-64.5%
<b>8</b>	72.12	75.06	+4.1%
<b>9</b>	81.47	83.11	+2.0%
<b>10</b>	31.11	31.11	0.0%
<b>11 (undisturbed)</b>	21.12	21.12	0.0%
<b>12 (undisturbed)</b>	34.26	34.26	0.0%
<b>13 (undisturbed)</b>	11.00	11.00	0.0%
<b>14</b>	92.15	87.88	-4.6%
<b>15 (undisturbed)</b>	15.74	15.74	0.0%
<b>16</b>	77.66	80.43	+3.6%
<b>17 (undisturbed)</b>	14.93	14.93	0.0%
<b>18a (undisturbed)</b>	1.12	1.12	0.0%
<b>18b</b>	8.60	5.99	-30.3%
<b>18c (undisturbed)</b>	1.25	1.25	0.0%

### Offsite



Approximately 370 acres of the site is proposed to be disturbed as part of this development including offsite roadways and future construction onsite. Runoff from proposed roadways constructed during the Meridian Upper Plateau project will be routed to detention tanks located within the landscape easements adjacent to the right of way. Reversed curb outlets located at the curb flow line will allow storm runoff to enter into the proposed earthen swale within the landscape easement that contains 18" atrium grates connected to each detention tank. The detention tanks are sized to hold the respective 100 year storm volumes and release it over 48 hours. An estimated conservative volume of 6,000 cubic-feet/acre was used to meet this criteria. All detention tanks will include 2" drain lines leading to a 12" storm drain header leading to the proposed Modular Wetland biotreatment units near catch basins at each street. The Modular Wetland Units will be connected to each catch basin. The catch basins will also act as an emergency collection point for stormwater if the modular wetland units, detention tanks or reverse curb outlets begin to back up for any reason.

Runoff from both roadways and laterals collecting stormwater from each parcel will be collected to proposed storm drain lines outletting prior to the watersheds outlets discussed in the existing condition. Storm drain Lines 16-1 and 16-2 will collect water from portions of Barton Road as well as Lot 15 and outlet within the existing creek of Watershed 16. Storm Drain Lines 2-1, 2-2 and 2-3 will be located in Watershed 2 and cover portions of Barton Road, Airman, Cactus Avenue and Linebacker Streets as well Lots 6, 7, 8, 14, 16 and 17. Storm Drain Line 3 will be located in Watershed 3 and cover Lot 1 along with portions of Arclight and Airman Streets. Storm Drain Line 5 will be located in Watershed 5 and cover portions of Arclight along with Lot 2. Storm Drain Line 7 is located in Watershed 7 and covers portions of Linebacker as well as Lot 3 and portions of Lot 4. Lines 8-1 and 8-2 are located in Watershed 8 and covers Lot 5 as well as portions of Lot 4. Storm Drain Line 14 is located in Watershed 14 and covers Lots 9, 10, 11. Rip rap will be placed at each outlet to slow down the velocity of the stormwater and not erode existing conditions.

Roadways within watershed 9 consist of Brown Avenue and Cactus Avenue from east of the intersection of Linebacker Drive to the connection at the existing limits of Cactus Avenue. This watershed will include 3 – 8' high clear span crossings underneath streets that address access for animals as well as maintain existing drainage courses. The locations can be seen in the proposed hydrology map in Appendix B. Storm drain lines 9-1 and 9-2 both collect runoff from catch basins on Cactus Avenue and portions of Brown Avenue and terminate into both clear span crossing located on Cactus Avenue. Cactus Avenue also has two locations where outlet structures will be built and directly tie into a pair of catch basins nearby. Brown Avenue will feature the remaining standalone clear span crossing at the existing drainage path. Two catch basins at the connection point to the existing limits of Brown Avenue will collect runoff and outlet out of a nearby outlet structure.

Watershed 3 currently surface flows on to Barton Road. With the extension and connection of Barton Road from the north and south in the proposed condition, a drain inlet will be installed and connected to a parkway drain onto Barton Road flowing north towards Alessandro Boulevard. This design will ensure stormwater from both the public storm drain headwall as well as the surrounding area within Watershed 3 will follow existing conditions.

Approximately 1 acre of Watershed 10 will consist of the proposed buildout of Cactus Avenue and include the connection point to the existing portion of Cactus Avenue to the east. Stormwater from this portion will continue to flow down to the existing Cactus Avenue and spill



into the existing reverse curb outlets serving the street. Ultimately this stormwater will lead to the regional North Detention Basin similar to watersheds 8,9,11,12 & 13.

### Onsite

Expected development onsite will consist of commercial buildings, parking areas and associated landscape. Since it is unknown at this time what the onsite design for parcels will look like, for this report, the onsite, un-detained flow rates have been computed based on the assumed ultimate buildout of each lot for commercial use at a 90% impervious surface as recommended by the Riverside County Hydrology Manual. Due to this increase in impervious surface area, an increase between pre and post development flow rate values is expected. Future development on each lot will need to address the 100 year pre vs. post development flow rate. Past Meridian projects have been designed using Geostorage underground systems utilizing stone, geosynthetics and reinforced concrete as well as systems that utilize HDPE pipe.

For purposes of this study, it was assumed that all detention basins offsite and future detention basins onsite are full and all runoff was flowing directly to both public street catch basins as well as laterals collecting runoff for each parcel. Refer to the “Proposed Hydrology Zones Map” and “Proposed Hydrology Map” in Appendix B for an illustration of the existing drainage patterns.

## Section IV

## Conclusion

The 100-year storm runoff flow rates were calculated using AES. Hydraulic calculations were done using WSPG along the length of the proposed storm drain system. Stormwater that outlets from these locations will continue to flow to the same areas as the existing conditions. The current road configuration and ultimate configuration of the lots are not determined at this time and may change depending on the planned developments for these lots. The table below gives a summary of maximum 100-year storm flow rates for each proposed storm drain line at the outlet.

The rough grading of the overall development will not increase the peak runoff from the existing condition. The installation of the public streets will be mitigated as described above to reduce the developed runoff down to undeveloped peak flow rates. Each development as they propose to construct impervious surface will be required to mitigate site runoff down to undeveloped flow rates as described above. The project as a whole will not increase the peak discharge rates from the undeveloped condition.

<b>Storm Drain Line</b>	<b>Max Flow at outlet (cfs)</b>	<b>Velocity at Outlet (fps)</b>	<b>Depth of Water at Outlet</b>	<b>Size of storm drain (in.)</b>
<b>16-1/16-2</b>	49.39	10.43	2.29	30
<b>2-1</b>	96.36	17.91	2.13	36
<b>2-2</b>	94.35	15.95	2.34	36
<b>2-3</b>	21.90	7.81	1.67	24
<b>3</b>	27.00	29.20	0.609	30



<b>5</b>	25.59	19.25	0.879	24
<b>7</b>	92.64	36.01	1.18	36
<b>8-1/8-2</b>	61.51	16.68	1.551	36
<b>14</b>	88.90	10.77	1.501	4' wide channel with 1:1 cross slopes

As stated previously, offsite runoff from roadways, sidewalks, and landscaped areas will be mitigated by detention tanks located within the landscape easements adjacent to the right of way. Reversed curb outlets located at the curb flow line will allow storm runoff to enter into the proposed earthen swale within the landscape easement that contains 18" atrium grates connected to each detention tank. The detention tanks are sized to hold the respective 100 year storm volumes and release it over 48 hours. An estimated conservative volume of 6,000 cubic-feet/acre was used to meet this criteria. All detention tanks will include 2" drain lines leading to a 6"-12" storm drain line leading to the proposed Modular Wetland biotreatment units near the beginning of the streets.

Existing downstream headwalls/storm drains accepting the outlet of each key watershed are listed below along with the corresponding totally 100 year flows for the proposed storm drain. The 100 year storm value used in the design of the existing storm drain as-built plans are also shown for each outlet.

<b>Watershed</b>	<b>Total Q<sub>100</sub> at Proposed Storm Drain outlet (cfs)</b>	<b>Q<sub>100</sub> used in the design of existing headwall structure downstream per record drawings (cfs)</b>	<b>Watershed Outlet/Notes</b>
<b>2</b>	212.6	412	Headwall and Rip Rap Pad to 84" RCP shown on City of Riverside Dwg. No. D636-A
<b>8</b>	61.51	136.74	Headwall to 42" RCP shown on County of Riverside Plot Plan 25422
<b>14</b>	88.90	Q <sub>100</sub> (Developed) = 287.5 Q <sub>100</sub> (Native) = 192.7	Grated catch basin to 78" RCP shown on City of Riverside Dwg. No. D-748
<b>16</b>	49.39	220.3	Headwall and concrete pad to 57" RCP shown on City of Riverside Dwg. No. D-660



**Catch Basin Sizing (FlowMaster)**

Catch Basin analysis was performed for each catch basin located on the public streets. The results are summarized in the table below. As discussed previously, the detention tanks located within the landscape easements adjacent to the streets were conservatively sized to collect the 100 year storm volume with runoff captured through reverse curb outlets located on each side of the street. The placement of catch basins on the street were included in the design for emergency cases where flows are higher than those created by the 100 year storm.

<b>Catch Basin</b>	<b>Flow Rate (cfs)</b>	<b>On Grade</b>	<b>Efficiency</b>	<b>Sag</b>	<b>Depth (inches)</b>
<b>1-2</b>	2.41	x	92.60	-	-
<b>3-4</b>	2.02	x	92.60	-	-
<b>5-6</b>	1.18	x	98.42	-	-
<b>7-8</b>	0.99	x	98.90	-	-
<b>9-10</b>	1.33	x	91.76	-	-
<b>11-12</b>	1.47	x	89.26	-	-
<b>13-14</b>	2.01	x	95.56	-	-
<b>15-16</b>	2.60	x	90.71	-	-
<b>17</b>	2.78	x	61.9	-	-
<b>18</b>	0.85	x	92.87	-	-
<b>19-20</b>	1.69	x	97.99	-	-
<b>21</b>	2.91	-	-	x	4.1
<b>22</b>	3.28	-	-	x	4.1
<b>23-24</b>	2.38	-	-	x	3.0
<b>25-26</b>	1.76	x	97.31	-	-
<b>27-28</b>	1.61	x	79.90	-	-
<b>29-30</b>	1.21	x	88.29	-	-
<b>31-32</b>	1.74	x	98.88	-	-
<b>33-34</b>	1.33	-	-	x	2.4
<b>35-36</b>	1.91	x	87.84	-	-
<b>37-38</b>	1.26	x	98.88	-	-
<b>39-40</b>	1.34	x	100.0	-	-

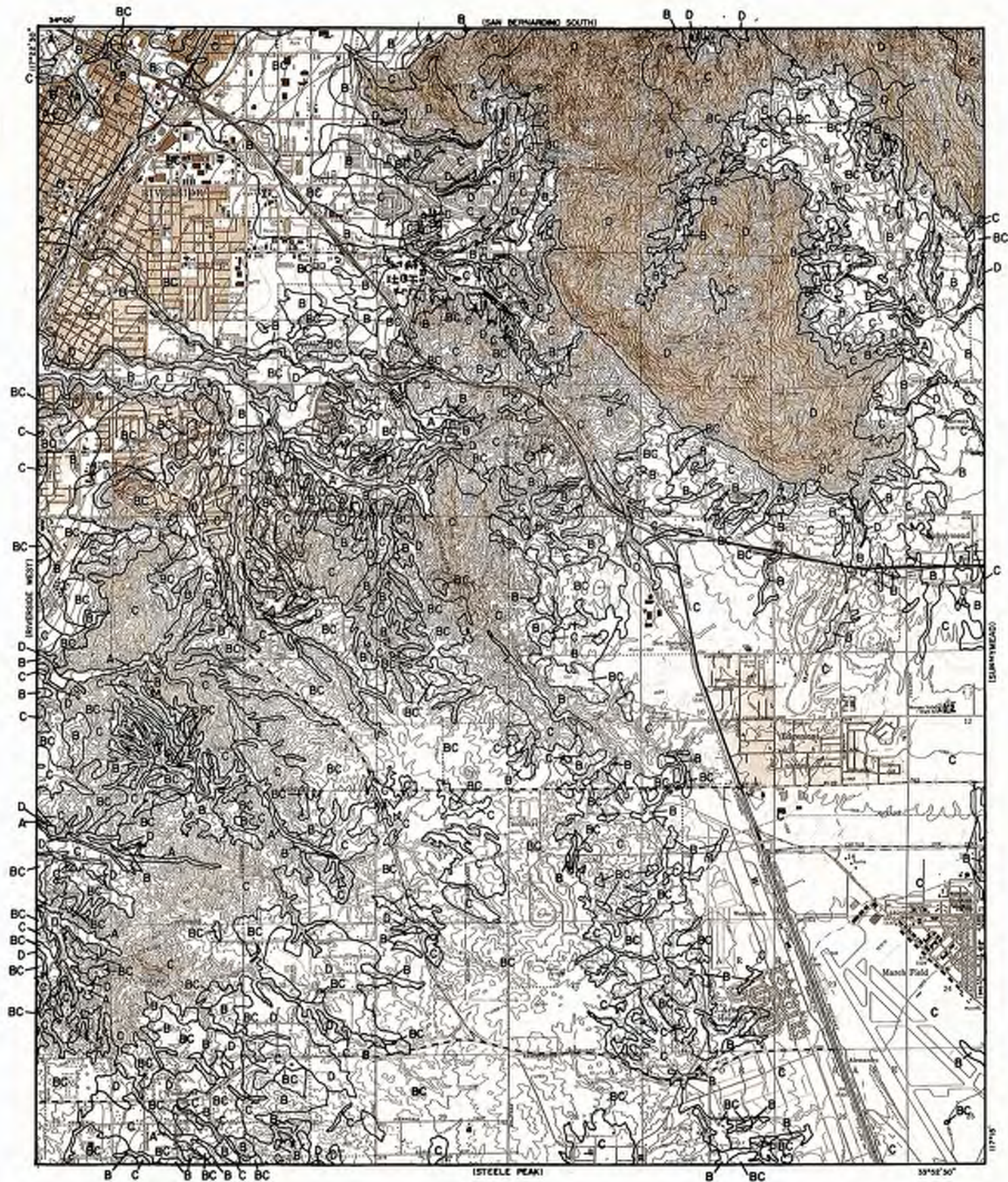
***APPENDIX A***

VICINITY & SOILS MAP





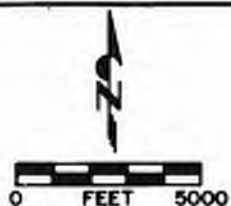




**LEGEND**

- SOILS GROUP BOUNDARY
- A SOILS GROUP DESIGNATION

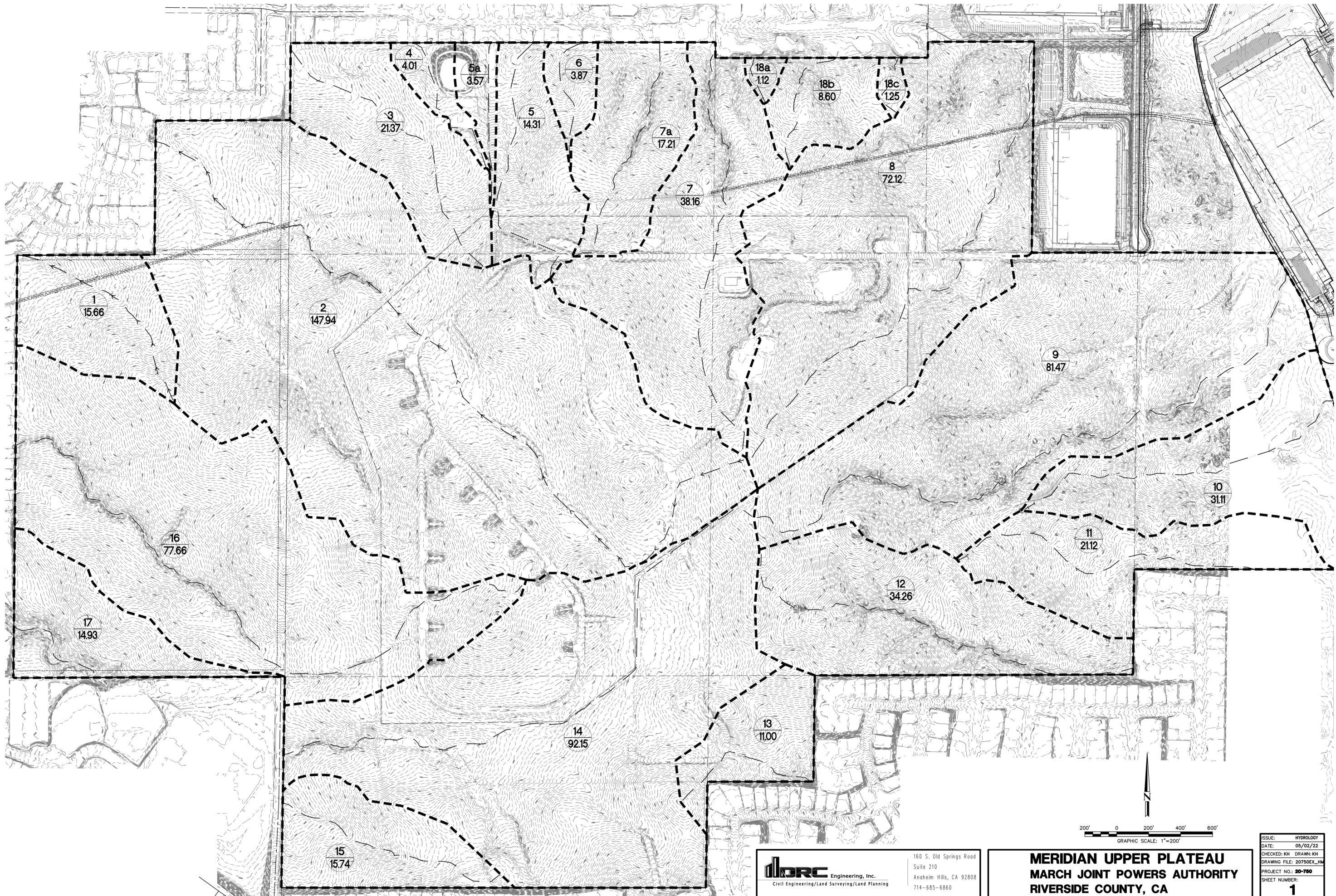
**RCFC & WCD**  
HYDROLOGY MANUAL



**HYDROLOGIC SOILS GROUP MAP  
FOR  
RIVERSIDE-EAST**

***APPENDIX B***

Existing Hydrology Zone Map  
Proposed Hydrology Zone Map  
Proposed Hydrology Map

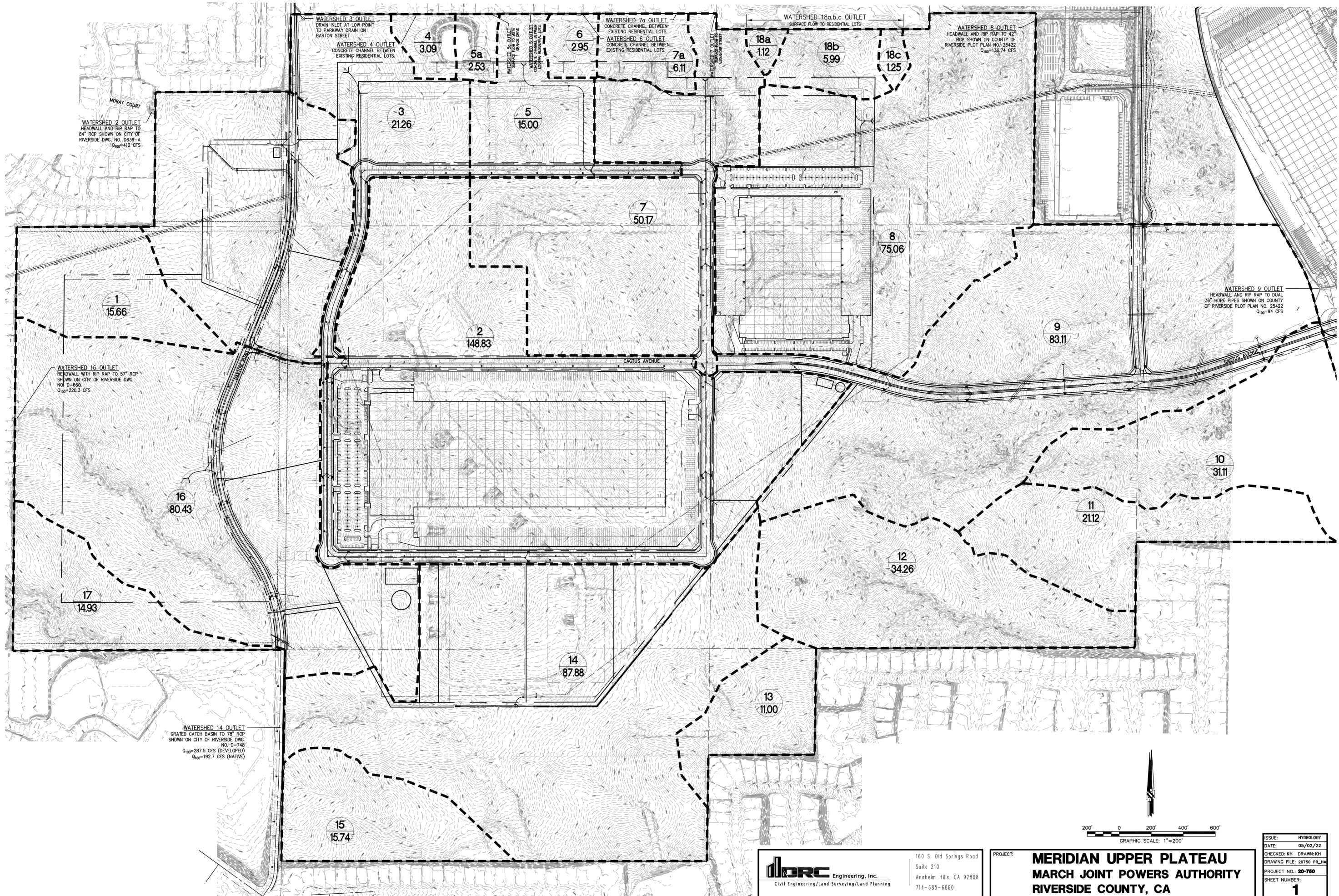


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 714-685-6860

**MERIDIAN UPPER PLATEAU**  
**MARCH JOINT POWERS AUTHORITY**  
**RIVERSIDE COUNTY, CA**  
 DRAWING NAME: **EXISTING HYDROLOGY ZONES MAP**

ISSUE:	HYDROLOGY
DATE:	05/02/22
CHECKED:	KH DRAWN: KH
DRAWING FILE:	20750EX_HH
PROJECT NO.:	20-750
SHEET NUMBER:	1
OF	1 SHEETS
SCALE:	AS SHOWN





WATERSHED 2 OUTLET  
HEADWALL AND RIP RAP TO  
84" RCP SHOWN ON CITY OF  
RIVERSIDE DWG. NO. D636-A  
Q<sub>100</sub>=412 CFS

WATERSHED 16 OUTLET  
HEADWALL WITH RIP RAP TO 57" RCP  
SHOWN ON CITY OF RIVERSIDE DWG.  
NO. D-660  
Q<sub>100</sub>=220.3 CFS

WATERSHED 14 OUTLET  
GRATED CATCH BASIN TO 78" RCP  
SHOWN ON CITY OF RIVERSIDE DWG.  
NO. D-748  
Q<sub>100</sub>=287.5 CFS (DEVELOPED)  
Q<sub>100</sub>=192.7 CFS (NATIVE)

WATERSHED 3 OUTLET  
DRAIN INLET AT LOW POINT  
TO PARKWAY DRAIN ON  
BARTON STREET

WATERSHED 4 OUTLET  
CONCRETE CHANNEL BETWEEN  
EXISTING RESIDENTIAL LOTS.

WATERSHED 5a OUTLET  
CONCRETE CHANNEL TO  
SHRUBS TO RAMP TO  
CONCRETE CHANNEL

WATERSHED 5 OUTLET  
CONCRETE CHANNEL BETWEEN  
EXISTING RESIDENTIAL LOTS.

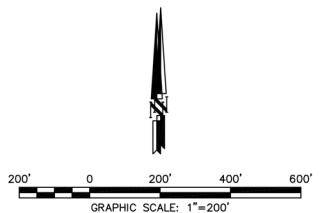
WATERSHED 7a OUTLET  
CONCRETE CHANNEL BETWEEN  
EXISTING RESIDENTIAL LOTS.

WATERSHED 6 OUTLET  
CONCRETE CHANNEL BETWEEN  
EXISTING RESIDENTIAL LOTS.

WATERSHED 18a,b,c OUTLET  
SURFACE FLOW TO RESIDENTIAL LOTS

WATERSHED 8 OUTLET  
HEADWALL AND RIP RAP TO 42"  
RCP SHOWN ON COUNTY OF  
RIVERSIDE PLOT PLAN NO. 25422  
Q<sub>100</sub>=136.74 CFS

WATERSHED 9 OUTLET  
HEADWALL AND RIP RAP TO DUAL  
36" HDPE PIPES SHOWN ON COUNTY  
OF RIVERSIDE PLOT PLAN NO. 25422  
Q<sub>100</sub>=94 CFS



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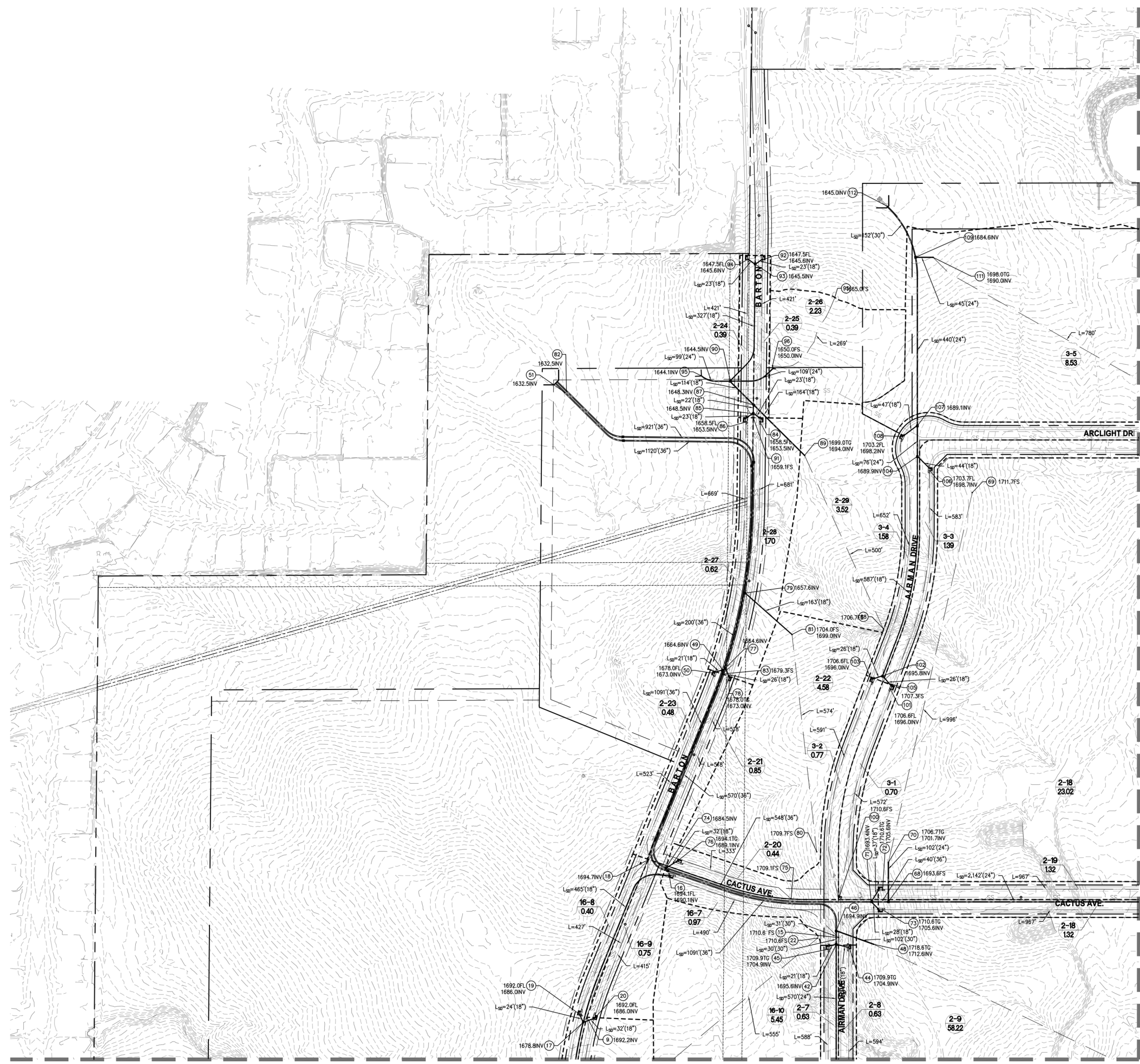
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Anheim Hills, CA 92808  
714-685-6860

PROJECT: **MERIDIAN UPPER PLATEAU  
MARCH JOINT POWERS AUTHORITY  
RIVERSIDE COUNTY, CA**

DRAWING NAME: **PROPOSED HYDROLOGY ZONES MAP**

ISSUE:	HYDROLOGY
DATE:	05/02/22
CHECKED:	KH DRAWN:KH
DRAWING FILE:	20750_PR_HH
PROJECT NO.:	20-750
SHEET NUMBER:	1
OF	1 SHEETS
SCALE:	AS SHOWN





### LEGEND

- FLOW PATH
- WATERSHED AREA
- HYDROLOGY NODE
- WATERSHED NAME
- WATERSHED ACREAGE

NO.	REVISION:	DATE:

**PROJECT:** MERIDIAN WEST CAMPUS  
 UPPER PLATEAU  
 MARCH JOINT POWERS AUTHORITY  
**PROPOSED HYDROLOGY MAP**

**DRAWING NAME:**

<b>ISSUE:</b>	HW
<b>DATE:</b>	8/24/2021
<b>CHECKED:</b>	KH
<b>DRAWN:</b>	KH
<b>DRAWING FILE:</b>	
<b>PROJECT NO.:</b>	20-780
<b>SHEET NUMBER:</b>	<b>1</b>
<b>OF</b>	6 SHEETS
<b>SCALE:</b>	AS SHOWN

NOT FOR CONSTRUCTION

FILENAME: M:\2020\20-780\_Meridian West Campus\Upper Plateau\March Joint Powers Authority\Hydrology Map.dwg PLOT DATE: 8/24/2021 10:45:00 AM PLOT BY: JORC

MATCHLINE - SEE SHEET 4

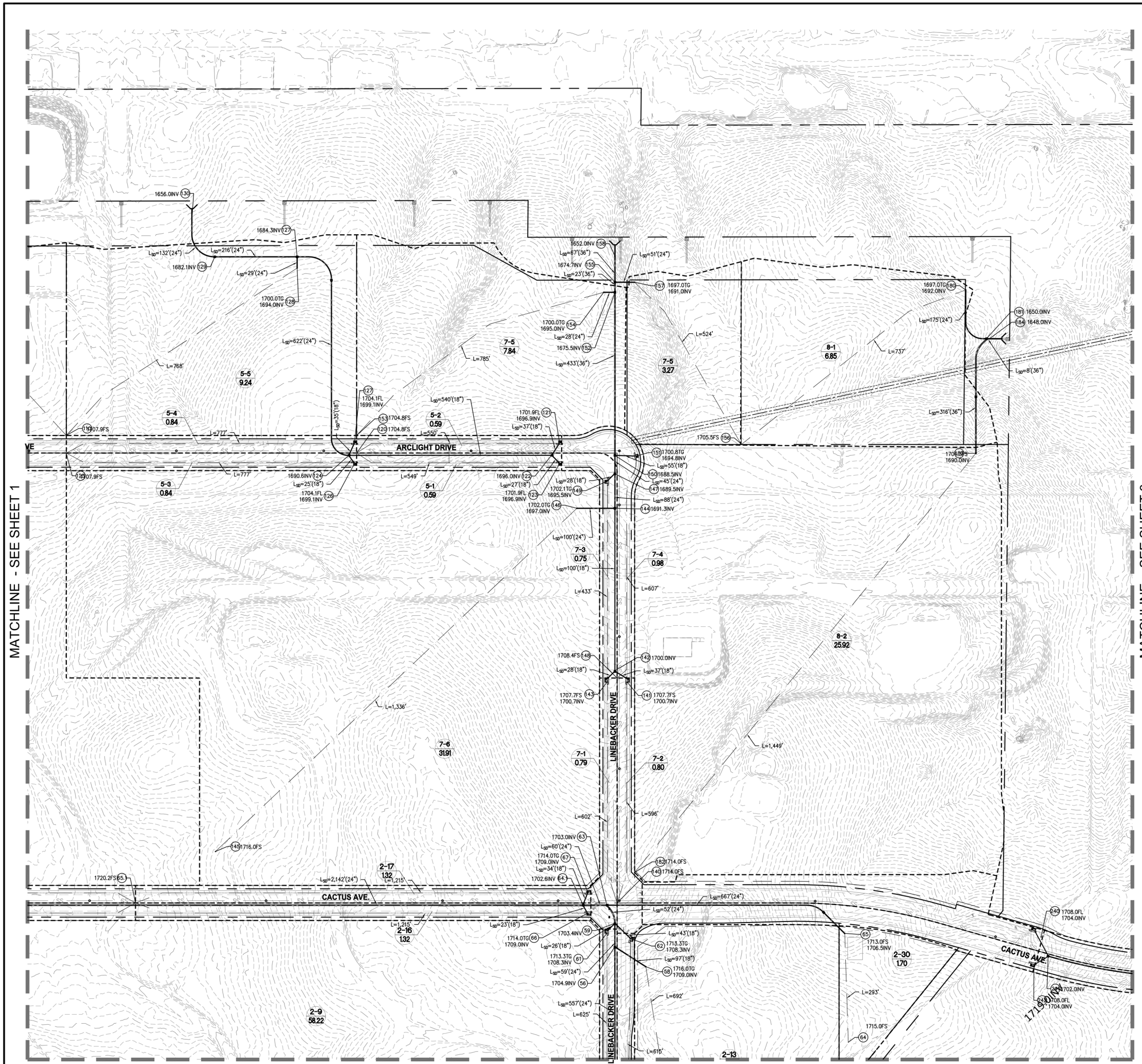
MATCHLINE - SEE SHEET 2





**LEGEND**

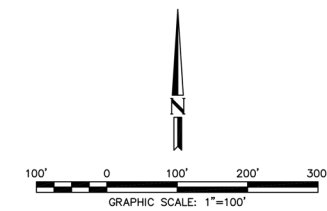
- FLOW PATH
- - - WATERSHED AREA
- ⊗ HYDROLOGY NODE
- X WATERSHED NAME
- XXX WATERSHED ACREAGE



MATCHLINE - SEE SHEET 1

MATCHLINE - SEE SHEET 3

MATCHLINE - SEE SHEET 5



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NO.	REVISION	DATE

**PROJECT:** MERIDIAN WEST CAMPUS  
 UPPER PLATEAU  
 MARCH JOINT POWERS AUTHORITY  
**PROPOSED HYDROLOGY MAP**

**DRAWING NAME:**

**ISSUE:** HW  
**DATE:** 8/24/2021  
**CHECKED:** KH **DRAWN:** KH  
**DRAWING FILE:**  
**PROJECT NO.:** 20-750  
**SHEET NUMBER:**  
**2**  
 OF 6 SHEETS  
**SCALE:** AS SHOWN

NOT FOR CONSTRUCTION





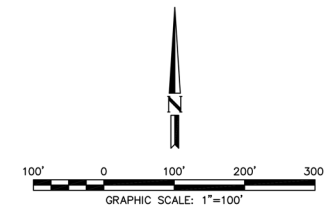


MATCHLINE - SEE SHEET 1



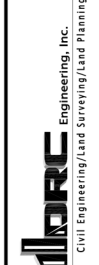
**LEGEND**

- FLOW PATH
- WATERSHED AREA
- HYDROLOGY NODE
- WATERSHED NAME
- WATERSHED ACREAGE



MATCHLINE - SEE SHEET 5

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NO.	REVISION:

**PROJECT:** MERIDIAN WEST CAMPUS  
 UPPER PLATEAU  
 MARCH JOINT POWERS AUTHORITY  
**PROPOSED HYDROLOGY MAP**

**DRAWING NAME:**

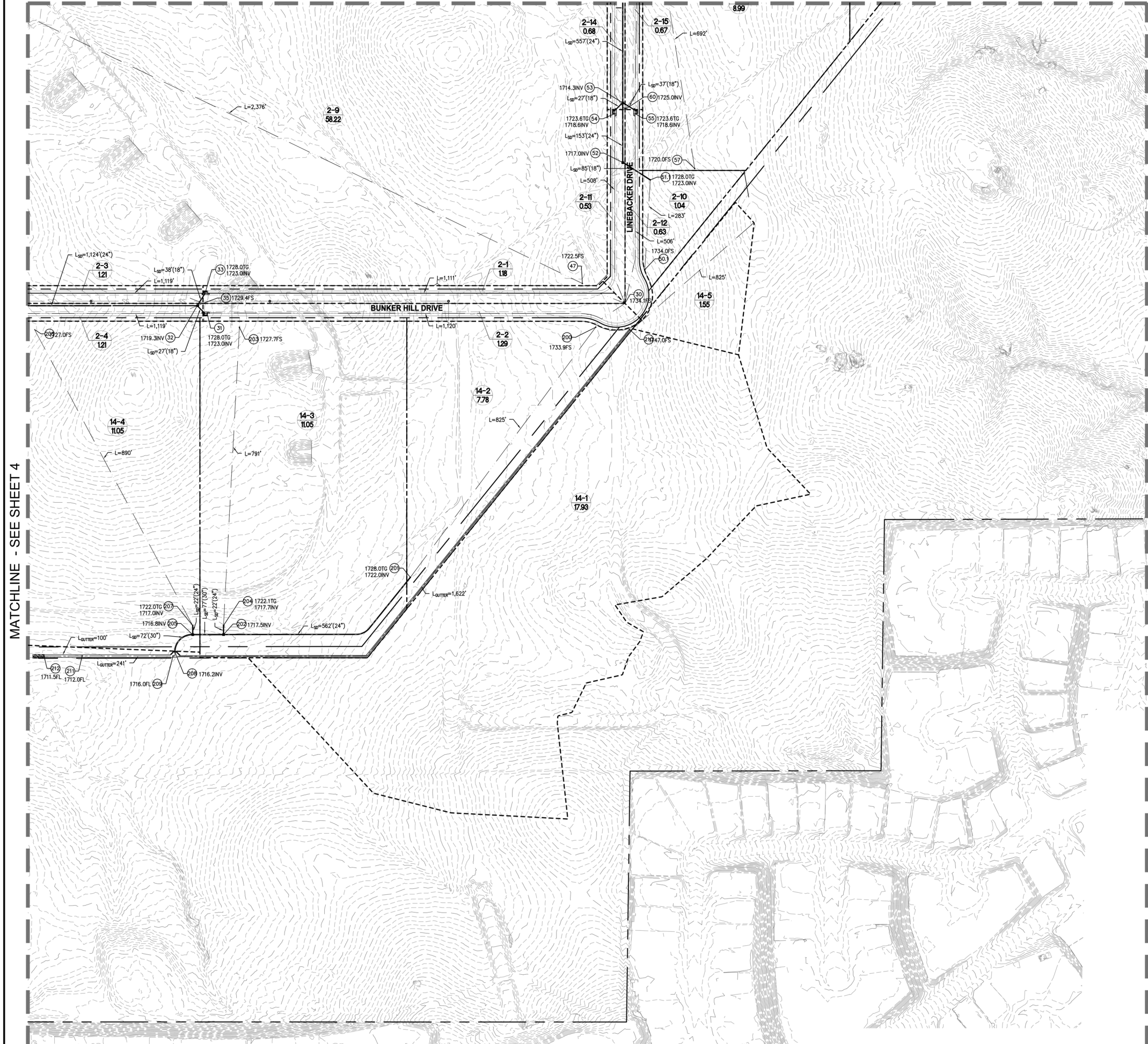
ISSUE:	HW
DATE:	8/24/2021
CHECKED:	KH DRAWN: KH
DRAWING FILE:	
PROJECT NO.:	20-750
SHEET NUMBER:	<b>4</b>
OF	6 SHEETS
SCALE:	AS SHOWN

FILE NAME: M:\2020\20-750\_Meridian West Campus Upper Plateau\2020 Proposed Hydrology Map.dwg, LAST SAVE: 8/24/2021 10:22am, CFX

NOT FOR CONSTRUCTION



MATCHLINE - SEE SHEET 2

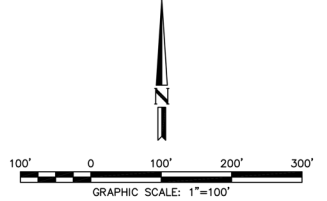


**LEGEND**

- FLOW PATH
- - - WATERSHED AREA
- (XX) HYDROLOGY NODE
- X WATERSHED NAME
- XXX WATERSHED ACREAGE

MATCHLINE - SEE SHEET 4

MATCHLINE - SEE SHEET 6



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NO.	REVISION	DATE

PROJECT: **MERIDIAN WEST CAMPUS  
 UPPER PLATEAU  
 MARCH JOINT POWERS AUTHORITY  
 PROPOSED HYDROLOGY MAP**

DRAWING NAME: \_\_\_\_\_

ISSUE: HW  
 DATE: 8/24/2021  
 CHECKED: KH DRAWN: KH  
 DRAWING FILE: \_\_\_\_\_  
 PROJECT NO.: 20-750  
 SHEET NUMBER: **5**  
 OF 6 SHEETS  
 SCALE: AS SHOWN

NOT FOR CONSTRUCTION





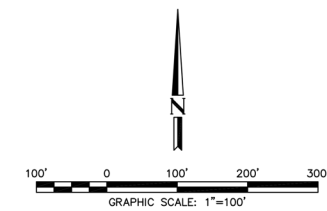
MATCHLINE - SEE SHEET 3

MATCHLINE - SEE SHEET 5



**LEGEND**

- FLOW PATH
- - - WATERSHED AREA
- ⊗ HYDROLOGY NODE
- X WATERSHED NAME
- XXX WATERSHED ACREAGE



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**JORC**  
 Engineering, Inc.  
 Civil Engineering/Land Surveying/Land Planning

NO.	REVISION	DATE

**PROJECT:** MERIDIAN WEST CAMPUS  
 UPPER PLATEAU  
 MARCH JOINT POWERS AUTHORITY  
**PROPOSED HYDROLOGY MAP**

**DRAWING NAME:**

**ISSUE:** HW  
**DATE:** 8/24/2021  
**CHECKED:** KH **DRAWN:** KH  
**DRAWING FILE:**  
**PROJECT NO.:** 20-750  
**SHEET NUMBER:**  
**6**  
 OF 6 SHEETS  
**SCALE:** AS SHOWN

FILENAME: M:\2020\20-750\_Meridian West Campus\Map\20-750-Proposed Hydrology Map.dwg, LAST SAVED: 08/24/2021 10:22:00 AM, PLOTTED BY: KKHAN, 08/24/2021 10:22:00 AM, PLOT SCALE: 1"=100'

NOT FOR CONSTRUCTION



***APPENDIX C***

Proposed Condition 100-year Hydrology Calculations (AES – Rational Method)

\*\*\*\*\*

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM BASED ON  
RIVERSIDE COUNTY FLOOD CONTROL & WATER CONSERVATION DISTRICT  
(RCFC&WCD) 1978 HYDROLOGY MANUAL  
(c) Copyright 1982-2015 Advanced Engineering Software (aes)  
(Rational Tabling Version 22.0)  
Release Date: 07/01/2015 License ID 1510

Analysis prepared by:

DRC Engineering, Inc.  
160 South Old Springs Road, Suite 210  
Anaheim Hills, CA 92808  
714-685-6860

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* Meridian Upper Plateau \*  
\* 100 year proposed \*  
\* Public Storm Drain \*  
\*\*\*\*\*

FILE NAME: 20750P.DAT  
TIME/DATE OF STUDY: 08:21 09/14/2021

-----  
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
-----

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 12.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 1.00  
10-YEAR STORM 10-MINUTE INTENSITY(INCH/HOUR) = 1.880  
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.700  
100-YEAR STORM 10-MINUTE INTENSITY(INCH/HOUR) = 2.680  
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.000  
SLOPE OF 10-YEAR INTENSITY-DURATION CURVE = 0.5513834  
SLOPE OF 100-YEAR INTENSITY-DURATION CURVE = 0.5501947

COMPUTED RAINFALL INTENSITY DATA:  
STORM EVENT = 100.00 1-HOUR INTENSITY(INCH/HOUR) = 1.000  
SLOPE OF INTENSITY DURATION CURVE = 0.5502

RCFC&WCD HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD  
NOTE: COMPUTE CONFLUENCE VALUES ACCORDING TO RCFC&WCD HYDROLOGY MANUAL  
AND IGNORE OTHER CONFLUENCE COMBINATIONS FOR DOWNSTREAM ANALYSES

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- CROWN TO		STREET-CROSSFALL:			CURB GUTTER-GEOMETRIES:			MANNING FACTOR (n)	
	WIDTH (FT)	CROSSFALL (FT)	IN- SIDE	OUT- SIDE	PARK- WAY	HEIGHT (FT)	WIDTH (FT)	LIP (FT)		HIKE (FT)
1	30.0	20.0	0.018	0.018	0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)

\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

-----+  
| Watershed 16 |  
| |  
| |  
+-----

```

+-----+
*****
FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
      ASSUMED INITIAL SUBAREA UNIFORM
      DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 586.00
UPSTREAM ELEVATION(FEET) = 1688.60
DOWNSTREAM ELEVATION(FEET) = 1684.60
ELEVATION DIFFERENCE(FEET) = 4.00
TC = 0.303*[(586.00**3)/(4.00)]**.2 = 10.517
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.607
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8832
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 1.91
TOTAL AREA(ACRES) = 0.83 TOTAL RUNOFF(CFS) = 1.91

*****
FLOW PROCESS FROM NODE      2.00 TO NODE      3.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1679.00 DOWNSTREAM(FEET) = 1678.70
FLOW LENGTH(FEET) = 18.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.43
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.91
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 10.57
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 604.00 FEET.

*****
FLOW PROCESS FROM NODE      3.00 TO NODE      3.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.57
RAINFALL INTENSITY(INCH/HR) = 2.60
TOTAL STREAM AREA(ACRES) = 0.83
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.91

*****
FLOW PROCESS FROM NODE      1.00 TO NODE      4.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
      ASSUMED INITIAL SUBAREA UNIFORM
      DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 586.00
UPSTREAM ELEVATION(FEET) = 1688.60
DOWNSTREAM ELEVATION(FEET) = 1684.10
ELEVATION DIFFERENCE(FEET) = 4.50

```

$TC = 0.303 * [(586.00^{**3}) / (4.50)]^{**2} = 10.273$   
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.641  
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8834  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 2.87  
 TOTAL AREA (ACRES) = 1.23 TOTAL RUNOFF (CFS) = 2.87

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 4.00 TO NODE 3.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====  
 ELEVATION DATA: UPSTREAM (FEET) = 1679.10 DOWNSTREAM (FEET) = 1679.00  
 FLOW LENGTH (FEET) = 29.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.6 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.42  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 2.87  
 PIPE TRAVEL TIME (MIN.) = 0.14 Tc (MIN.) = 10.41  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 615.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 10.41  
 RAINFALL INTENSITY (INCH/HR) = 2.62  
 TOTAL STREAM AREA (ACRES) = 1.23  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.87

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.91	10.57	2.599	0.83
2	2.87	10.41	2.621	1.23

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	4.75	10.41	2.621
2	4.76	10.57	2.599

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.75 Tc (MIN.) = 10.41  
 TOTAL AREA (ACRES) = 2.1  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 615.00 FEET.

```

*****
FLOW PROCESS FROM NODE      3.00 TO NODE      5.00 IS CODE = 41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1678.70  DOWNSTREAM(FEET) = 1678.40
FLOW LENGTH(FEET) = 20.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.73
GIVEN PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.75
PIPE TRAVEL TIME(MIN.) = 0.05  Tc(MIN.) = 10.46
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 635.00 FEET.

*****
FLOW PROCESS FROM NODE      5.00 TO NODE      5.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.46
RAINFALL INTENSITY(INCH/HR) = 2.61
TOTAL STREAM AREA(ACRES) = 2.06
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.75

*****
FLOW PROCESS FROM NODE      6.00 TO NODE      7.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 1130.00
UPSTREAM ELEVATION(FEET) = 1722.00
DOWNSTREAM ELEVATION(FEET) = 1708.00
ELEVATION DIFFERENCE(FEET) = 14.00
TC = 0.533*[(1130.00**3)/(14.00)]**.2 = 21.332
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.766
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6721
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 10.66
TOTAL AREA(ACRES) = 8.98  TOTAL RUNOFF(CFS) = 10.66

*****
FLOW PROCESS FROM NODE      7.00 TO NODE      5.00 IS CODE = 41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1700.00  DOWNSTREAM(FEET) = 1678.40
FLOW LENGTH(FEET) = 135.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 5.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.32
GIVEN PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.66
PIPE TRAVEL TIME(MIN.) = 0.12  Tc(MIN.) = 21.45

```

LONGEST FLOWPATH FROM NODE 6.00 TO NODE 5.00 = 1265.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 21.45  
RAINFALL INTENSITY(INCH/HR) = 1.76  
TOTAL STREAM AREA(ACRES) = 8.98  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.66

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.75	10.46	2.614	2.06
2	10.66	21.45	1.761	8.98

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	9.95	10.46	2.614
2	13.86	21.45	1.761

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 13.86 Tc(MIN.) = 21.45  
TOTAL AREA(ACRES) = 11.0  
LONGEST FLOWPATH FROM NODE 6.00 TO NODE 5.00 = 1265.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 8.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1678.40 DOWNSTREAM(FEET) = 1675.50  
FLOW LENGTH(FEET) = 558.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.0 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.83  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 13.86  
PIPE TRAVEL TIME(MIN.) = 1.60 Tc(MIN.) = 23.04  
LONGEST FLOWPATH FROM NODE 6.00 TO NODE 8.00 = 1823.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<



=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 23.04  
RAINFALL INTENSITY(INCH/HR) = 1.69  
TOTAL STREAM AREA(ACRES) = 11.04  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.86

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 10.00 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 586.00  
UPSTREAM ELEVATION(FEET) = 1692.20  
DOWNSTREAM ELEVATION(FEET) = 1687.00  
ELEVATION DIFFERENCE(FEET) = 5.20  
TC =  $0.303 * [(586.00^{**3}) / (5.20)]^{**.2} = 9.980$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.683  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8836  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.82  
TOTAL AREA(ACRES) = 1.19 TOTAL RUNOFF(CFS) = 2.82

\*\*\*\*\*

FLOW PROCESS FROM NODE 10.00 TO NODE 8.00 IS CODE = 41

-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1682.00 DOWNSTREAM(FEET) = 1675.50  
FLOW LENGTH(FEET) = 29.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.9 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.23  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.82  
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 10.01  
LONGEST FLOWPATH FROM NODE 9.00 TO NODE 8.00 = 615.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 10.01  
RAINFALL INTENSITY(INCH/HR) = 2.68  
TOTAL STREAM AREA(ACRES) = 1.19  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.82

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 10.50 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM

DEVELOPMENT IS COMMERCIAL  
 TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 586.00  
 UPSTREAM ELEVATION(FEET) = 1692.20  
 DOWNSTREAM ELEVATION(FEET) = 1687.00  
 ELEVATION DIFFERENCE(FEET) = 5.20  
 TC = 0.303\*[(586.00\*\*3)/(5.20)]\*\*.2 = 9.980  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.683  
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8836  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF(CFS) = 1.33  
 TOTAL AREA(ACRES) = 0.56 TOTAL RUNOFF(CFS) = 1.33

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 10.50 TO NODE 8.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1682.00 DOWNSTREAM(FEET) = 1675.50  
 FLOW LENGTH(FEET) = 18.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.8 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 14.37  
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 1.33  
 PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 10.00  
 LONGEST FLOWPATH FROM NODE 9.00 TO NODE 8.00 = 604.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
 TIME OF CONCENTRATION(MIN.) = 10.00  
 RAINFALL INTENSITY(INCH/HR) = 2.68  
 TOTAL STREAM AREA(ACRES) = 0.56  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.33

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	13.86	23.04	1.693	11.04
2	2.82	10.01	2.678	1.19
3	1.33	10.00	2.680	0.56

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
---------------	--------------	-----------	-----------------------

1	10.16	10.00	2.680
2	10.17	10.01	2.678
3	16.48	23.04	1.693

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 16.48 Tc (MIN.) = 23.04  
 TOTAL AREA (ACRES) = 12.8  
 LONGEST FLOWPATH FROM NODE 6.00 TO NODE 8.00 = 1823.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 11.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1675.50 DOWNSTREAM (FEET) = 1674.30  
 FLOW LENGTH (FEET) = 219.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 19.3 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.08  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 16.48  
 PIPE TRAVEL TIME (MIN.) = 0.60 Tc (MIN.) = 23.64  
 LONGEST FLOWPATH FROM NODE 6.00 TO NODE 11.00 = 2042.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 23.64  
 RAINFALL INTENSITY (INCH/HR) = 1.67  
 TOTAL STREAM AREA (ACRES) = 12.79  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 16.48

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 12.00 TO NODE 13.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
 DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
 $TC = K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**0.2}$   
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 608.00  
 UPSTREAM ELEVATION (FEET) = 1724.50  
 DOWNSTREAM ELEVATION (FEET) = 1708.00  
 ELEVATION DIFFERENCE (FEET) = 16.50  
 $TC = 0.533 * [(608.00^{**3}) / (16.50)]^{**0.2} = 14.231$   
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.207  
 UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7079  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 7.20  
 TOTAL AREA (ACRES) = 4.61 TOTAL RUNOFF (CFS) = 7.20

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 13.00 TO NODE 11.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

```

=====
ELEVATION DATA: UPSTREAM(FEET) = 1700.00  DOWNSTREAM(FEET) = 1674.30
FLOW LENGTH(FEET) = 156.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 4.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 17.41
GIVEN PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.20
PIPE TRAVEL TIME(MIN.) = 0.15  Tc(MIN.) = 14.38
LONGEST FLOWPATH FROM NODE 12.00 TO NODE 11.00 = 764.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 11.00 TO NODE 11.00 IS CODE = 1
-----

```

```

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

```

```

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.38
RAINFALL INTENSITY(INCH/HR) = 2.19
TOTAL STREAM AREA(ACRES) = 4.61
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.20

```

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	16.48	23.64	1.669	12.79
2	7.20	14.38	2.194	4.61

```

*****WARNING*****
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.
*****

```

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	17.23	14.38	2.194
2	21.96	23.64	1.669

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

```

PEAK FLOW RATE(CFS) = 21.96  Tc(MIN.) = 23.64
TOTAL AREA(ACRES) = 17.4
LONGEST FLOWPATH FROM NODE 6.00 TO NODE 11.00 = 2042.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 11.00 TO NODE 14.00 IS CODE = 41
-----

```

```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 1674.30  DOWNSTREAM(FEET) = 1674.00
FLOW LENGTH(FEET) = 73.00  MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.99
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)

```

GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 21.96  
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 23.82  
LONGEST FLOWPATH FROM NODE 6.00 TO NODE 14.00 = 2115.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 14.00 TO NODE 14.00 IS CODE = 10

-----  
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<  
=====

\*\*\*\*\*  
FLOW PROCESS FROM NODE 14.00 TO NODE 14.00 IS CODE = 13

-----  
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<  
=====

\*\*\*\*\*  
FLOW PROCESS FROM NODE 15.00 TO NODE 16.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 490.00  
UPSTREAM ELEVATION(FEET) = 1710.60  
DOWNSTREAM ELEVATION(FEET) = 1694.10  
ELEVATION DIFFERENCE(FEET) = 16.50  
TC =  $0.303 * [(490.00^{**3}) / (16.50)]^{**2}$  = 7.116  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.232  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8859  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.78  
TOTAL AREA(ACRES) = 0.97 TOTAL RUNOFF(CFS) = 2.78

\*\*\*\*\*  
FLOW PROCESS FROM NODE 16.00 TO NODE 19.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) = 1690.10 DOWNSTREAM(FEET) = 1678.80  
FLOW LENGTH(FEET) = 465.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.0 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.90  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.78  
PIPE TRAVEL TIME(MIN.) = 1.12 Tc(MIN.) = 8.24  
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 19.00 = 955.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 17.00 TO NODE 17.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 8.24  
RAINFALL INTENSITY(INCH/HR) = 2.98

TOTAL STREAM AREA(ACRES) = 0.97  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.78

\*\*\*\*\*

FLOW PROCESS FROM NODE 18.00 TO NODE 19.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 415.00  
UPSTREAM ELEVATION(FEET) = 1694.70  
DOWNSTREAM ELEVATION(FEET) = 1692.00  
ELEVATION DIFFERENCE(FEET) = 2.70  
TC =  $0.303 * [(415.00^{**3}) / (2.70)]^{**.2} = 9.250$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.797  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8841  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 0.99  
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 0.99

\*\*\*\*\*

FLOW PROCESS FROM NODE 19.00 TO NODE 17.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1686.00 DOWNSTREAM(FEET) = 1678.80  
FLOW LENGTH(FEET) = 24.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.6 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.31  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 0.99  
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 9.28  
LONGEST FLOWPATH FROM NODE 18.00 TO NODE 17.00 = 439.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 17.00 TO NODE 17.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 9.28  
RAINFALL INTENSITY(INCH/HR) = 2.79  
TOTAL STREAM AREA(ACRES) = 0.40  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.99

\*\*\*\*\*

FLOW PROCESS FROM NODE 18.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 415.00  
UPSTREAM ELEVATION(FEET) = 1694.70  
DOWNSTREAM ELEVATION(FEET) = 1692.00

ELEVATION DIFFERENCE (FEET) = 2.70  
 TC = 0.303\*[( 415.00\*\*3)/( 2.70)]\*\*.2 = 9.250  
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.797  
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8841  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 1.85  
 TOTAL AREA (ACRES) = 0.75 TOTAL RUNOFF (CFS) = 1.85

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20.00 TO NODE 17.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1686.00 DOWNSTREAM (FEET) = 1678.80  
 FLOW LENGTH (FEET) = 32.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.4 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 13.47  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 1.85  
 PIPE TRAVEL TIME (MIN.) = 0.04 Tc (MIN.) = 9.29  
 LONGEST FLOWPATH FROM NODE 18.00 TO NODE 17.00 = 447.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 17.00 TO NODE 17.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
 TIME OF CONCENTRATION (MIN.) = 9.29  
 RAINFALL INTENSITY (INCH/HR) = 2.79  
 TOTAL STREAM AREA (ACRES) = 0.75  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.85

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.78	8.24	2.982	0.97
2	0.99	9.28	2.792	0.40
3	1.85	9.29	2.791	0.75

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	5.30	8.24	2.982
2	5.44	9.28	2.792
3	5.44	9.29	2.791

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 5.30 Tc (MIN.) = 8.24  
TOTAL AREA (ACRES) = 2.1  
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 17.00 = 955.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 17.00 TO NODE 21.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1678.80 DOWNSTREAM(FEET) = 1676.96  
FLOW LENGTH(FEET) = 191.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 8.0 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.80  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 5.30  
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 8.79  
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 21.00 = 1146.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 21.00 TO NODE 21.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 8.79  
RAINFALL INTENSITY(INCH/HR) = 2.88  
TOTAL STREAM AREA(ACRES) = 2.12  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.30

\*\*\*\*\*  
FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 555.00  
UPSTREAM ELEVATION(FEET) = 1710.60  
DOWNSTREAM ELEVATION(FEET) = 1710.00  
ELEVATION DIFFERENCE(FEET) = 0.60  
TC =  $0.533 * [(555.00^{**3}) / (0.60)]^{**2}$  = 26.143  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.579  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6525  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 5.62  
TOTAL AREA(ACRES) = 5.45 TOTAL RUNOFF(CFS) = 5.62

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.00 TO NODE 21.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1705.60 DOWNSTREAM(FEET) = 1676.96  
FLOW LENGTH(FEET) = 108.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 3.6 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.11



GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 5.62  
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 26.24  
LONGEST FLOWPATH FROM NODE 22.00 TO NODE 21.00 = 663.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 21.00 TO NODE 21.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 26.24  
RAINFALL INTENSITY(INCH/HR) = 1.58  
TOTAL STREAM AREA(ACRES) = 5.45  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.62

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	5.30	8.79	2.878	2.12
2	5.62	26.24	1.576	5.45

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.18	8.79	2.878
2	8.52	26.24	1.576

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 8.52 Tc(MIN.) = 26.24  
TOTAL AREA(ACRES) = 7.6  
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 21.00 = 1146.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 21.00 TO NODE 14.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1676.96 DOWNSTREAM(FEET) = 1674.00  
FLOW LENGTH(FEET) = 115.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 7.9 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.43  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 8.52  
PIPE TRAVEL TIME(MIN.) = 0.20 Tc(MIN.) = 26.44  
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 14.00 = 1261.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 14.00 TO NODE 14.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

\*\* MAIN STREAM CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	8.52	26.44	1.570	7.57
LONGEST FLOWPATH FROM NODE			15.00 TO NODE	14.00 = 1261.00 FEET.

\*\* MEMORY BANK # 1 CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	21.96	23.82	1.662	17.40
LONGEST FLOWPATH FROM NODE			6.00 TO NODE	14.00 = 2115.00 FEET.

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	29.64	23.82	1.662
2	29.26	26.44	1.570

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 29.64 Tc (MIN.) = 23.82  
 TOTAL AREA (ACRES) = 25.0

FLOW PROCESS FROM NODE 14.00 TO NODE 24.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1674.00 DOWNSTREAM(FEET) = 1671.00  
 FLOW LENGTH(FEET) = 291.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 18.7 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.21  
 GIVEN PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 29.64  
 PIPE TRAVEL TIME(MIN.) = 0.53 Tc(MIN.) = 24.35  
 LONGEST FLOWPATH FROM NODE 6.00 TO NODE 24.00 = 2406.00 FEET.

FLOW PROCESS FROM NODE 24.00 TO NODE 24.00 IS CODE = 13

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<

```

+-----+
| Watershed 2 (Storm Drain Line 2-1) |
|                                     |
|                                     |
+-----+

```

\*\*\*\*\*

FLOW PROCESS FROM NODE 30.00 TO NODE 30.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM DEVELOPMENT IS COMMERCIAL

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 1111.00
UPSTREAM ELEVATION(FEET) = 1734.10
DOWNSTREAM ELEVATION(FEET) = 1728.00
ELEVATION DIFFERENCE(FEET) = 6.10
TC = 0.303\*[(1111.00\*\*3)/(6.10)]\*\*.2 = 14.189
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.211
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8808
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 2.30
TOTAL AREA(ACRES) = 1.18 TOTAL RUNOFF(CFS) = 2.30

\*\*\*\*\*

FLOW PROCESS FROM NODE 33.00 TO NODE 32.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1723.00 DOWNSTREAM(FEET) = 1719.30
FLOW LENGTH(FEET) = 38.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.68
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.30
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 14.25
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 32.00 = 1149.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 32.00 TO NODE 32.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.25
RAINFALL INTENSITY(INCH/HR) = 2.21
TOTAL STREAM AREA(ACRES) = 1.18
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.30

\*\*\*\*\*

FLOW PROCESS FROM NODE 30.00 TO NODE 31.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM DEVELOPMENT IS COMMERCIAL

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 1111.00
UPSTREAM ELEVATION(FEET) = 1734.10
DOWNSTREAM ELEVATION(FEET) = 1728.00
ELEVATION DIFFERENCE(FEET) = 6.10
TC = 0.303\*[(1111.00\*\*3)/(6.10)]\*\*.2 = 14.189
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.211

COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8808  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF(CFS) = 2.51  
 TOTAL AREA(ACRES) = 1.29 TOTAL RUNOFF(CFS) = 2.51

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====  
 ELEVATION DATA: UPSTREAM(FEET) = 1723.00 DOWNSTREAM(FEET) = 1719.30  
 FLOW LENGTH(FEET) = 27.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.1 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 12.38  
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 2.51  
 PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 14.23  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 32.00 = 1138.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 32.00 TO NODE 32.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION(MIN.) = 14.23  
 RAINFALL INTENSITY(INCH/HR) = 2.21  
 TOTAL STREAM AREA(ACRES) = 1.29  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.51

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.30	14.25	2.206	1.18
2	2.51	14.23	2.208	1.29

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	4.81	14.23	2.208
2	4.81	14.25	2.206

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE(CFS) = 4.81 Tc(MIN.) = 14.23  
 TOTAL AREA(ACRES) = 2.5  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 32.00 = 1149.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 32.00 TO NODE 34.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1719.30 DOWNSTREAM(FEET) = 1714.80  
FLOW LENGTH(FEET) = 1124.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 9.6 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.11  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 4.81  
PIPE TRAVEL TIME(MIN.) = 4.56 Tc(MIN.) = 18.79  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 34.00 = 2273.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 34.00 TO NODE 34.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 18.79  
RAINFALL INTENSITY(INCH/HR) = 1.89  
TOTAL STREAM AREA(ACRES) = 2.47  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.81

\*\*\*\*\*  
FLOW PROCESS FROM NODE 35.00 TO NODE 36.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
 $TC = K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**0.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 1119.00  
UPSTREAM ELEVATION(FEET) = 1729.40  
DOWNSTREAM ELEVATION(FEET) = 1724.10  
ELEVATION DIFFERENCE(FEET) = 5.30  
 $TC = 0.303 * [(1119.00^{**3}) / (5.30)]^{**0.2} = 14.656$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.172  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8805  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.31  
TOTAL AREA(ACRES) = 1.21 TOTAL RUNOFF(CFS) = 2.31

\*\*\*\*\*  
FLOW PROCESS FROM NODE 36.00 TO NODE 34.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1718.10 DOWNSTREAM(FEET) = 1714.80  
FLOW LENGTH(FEET) = 27.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.1 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.61  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.31  
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 14.70  
LONGEST FLOWPATH FROM NODE 35.00 TO NODE 34.00 = 1146.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 34.00 TO NODE 34.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.70
RAINFALL INTENSITY(INCH/HR) = 2.17
TOTAL STREAM AREA(ACRES) = 1.21
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.31

\*\*\*\*\*

FLOW PROCESS FROM NODE 35.00 TO NODE 37.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 1119.00
UPSTREAM ELEVATION(FEET) = 1729.40
DOWNSTREAM ELEVATION(FEET) = 1724.10
ELEVATION DIFFERENCE(FEET) = 5.30
TC = 0.303\*[(1119.00\*\*3)/(5.30)]\*\*.2 = 14.656
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.172
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8805
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 2.31
TOTAL AREA(ACRES) = 1.21 TOTAL RUNOFF(CFS) = 2.31

\*\*\*\*\*

FLOW PROCESS FROM NODE 37.00 TO NODE 34.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1724.10 DOWNSTREAM(FEET) = 1714.80
FLOW LENGTH(FEET) = 38.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.80
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.31
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 14.70
LONGEST FLOWPATH FROM NODE 35.00 TO NODE 34.00 = 1157.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 34.00 TO NODE 34.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 14.70
RAINFALL INTENSITY(INCH/HR) = 2.17
TOTAL STREAM AREA(ACRES) = 1.21
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.31

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.81	18.79	1.894	2.47
2	2.31	14.70	2.168	1.21
3	2.31	14.70	2.168	1.21

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	8.39	14.70	2.168
2	8.39	14.70	2.168
3	8.85	18.79	1.894

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 8.85 Tc (MIN.) = 18.79  
 TOTAL AREA (ACRES) = 4.9  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 34.00 = 2273.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 34.00 TO NODE 38.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1714.80 DOWNSTREAM (FEET) = 1708.20  
 FLOW LENGTH (FEET) = 598.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.1 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.01  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 8.85  
 PIPE TRAVEL TIME (MIN.) = 1.42 Tc (MIN.) = 20.21  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 38.00 = 2871.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 20.21  
 RAINFALL INTENSITY (INCH/HR) = 1.82  
 TOTAL STREAM AREA (ACRES) = 4.89  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 8.85

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM

DEVELOPMENT IS COMMERCIAL  
 TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 557.00  
 UPSTREAM ELEVATION (FEET) = 1724.50  
 DOWNSTREAM ELEVATION (FEET) = 1718.60  
 ELEVATION DIFFERENCE (FEET) = 5.90  
 TC =  $0.303 * [(557.00^{**3}) / (5.90)]^{**2}$  = 9.439  
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.766  
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8840  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 1.54  
 TOTAL AREA (ACRES) = 0.63 TOTAL RUNOFF (CFS) = 1.54

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 40.00 TO NODE 38.00 IS CODE = 41  
 -----

>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<  
 >>>> USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT) <<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1712.60 DOWNSTREAM (FEET) = 1708.40  
 FLOW LENGTH (FEET) = 37.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.6 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 9.99  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 1.54  
 PIPE TRAVEL TIME (MIN.) = 0.06 Tc (MIN.) = 9.50  
 LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 594.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 1  
 -----

>>>> DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE <<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 9.50  
 RAINFALL INTENSITY (INCH/HR) = 2.76  
 TOTAL STREAM AREA (ACRES) = 0.63  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.54

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 39.00 TO NODE 41.00 IS CODE = 21  
 -----

>>>> RATIONAL METHOD INITIAL SUBAREA ANALYSIS <<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
 DEVELOPMENT IS COMMERCIAL  
 TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 709.00  
 UPSTREAM ELEVATION (FEET) = 1724.50  
 DOWNSTREAM ELEVATION (FEET) = 1718.60  
 ELEVATION DIFFERENCE (FEET) = 5.90  
 TC =  $0.303 * [(709.00^{**3}) / (5.90)]^{**2}$  = 10.909  
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.555  
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8829  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 1.87  
 TOTAL AREA (ACRES) = 0.83 TOTAL RUNOFF (CFS) = 1.87

\*\*\*\*\*



FLOW PROCESS FROM NODE 41.00 TO NODE 38.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1712.60 DOWNSTREAM(FEET) = 1708.20  
FLOW LENGTH(FEET) = 27.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.6 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.05  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.87  
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 10.95  
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 38.00 = 736.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 38.00 TO NODE 38.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
TIME OF CONCENTRATION(MIN.) = 10.95  
RAINFALL INTENSITY(INCH/HR) = 2.55  
TOTAL STREAM AREA(ACRES) = 0.83  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.87

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	8.85	20.21	1.820	4.89
2	1.54	9.50	2.757	0.63
3	1.87	10.95	2.550	0.83

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.33	9.50	2.757
2	8.09	10.95	2.550
3	11.20	20.21	1.820

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
PEAK FLOW RATE(CFS) = 11.20 Tc(MIN.) = 20.21  
TOTAL AREA(ACRES) = 6.3  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 38.00 = 2871.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 38.00 TO NODE 42.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====
ELEVATION DATA: UPSTREAM(FEET) = 1708.20 DOWNSTREAM(FEET) = 1695.60
FLOW LENGTH(FEET) = 570.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 9.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.63
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 11.20
PIPE TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 21.19
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 42.00 = 3441.00 FEET.

\*\*\*\*\*
FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 21.19
RAINFALL INTENSITY(INCH/HR) = 1.77
TOTAL STREAM AREA(ACRES) = 6.35
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.20

\*\*\*\*\*
FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 21
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====
ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 594.00
UPSTREAM ELEVATION(FEET) = 1718.80
DOWNSTREAM ELEVATION(FEET) = 1709.90
ELEVATION DIFFERENCE(FEET) = 8.90
TC = 0.303\*[(594.00\*\*3)/(8.90)]\*\*.2 = 9.036
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.834
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8843
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 1.58
TOTAL AREA(ACRES) = 0.63 TOTAL RUNOFF(CFS) = 1.58

\*\*\*\*\*
FLOW PROCESS FROM NODE 44.00 TO NODE 42.00 IS CODE = 41
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====
ELEVATION DATA: UPSTREAM(FEET) = 1704.90 DOWNSTREAM(FEET) = 1695.60
FLOW LENGTH(FEET) = 33.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.85
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.58
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 9.08
LONGEST FLOWPATH FROM NODE 43.00 TO NODE 42.00 = 627.00 FEET.

\*\*\*\*\*
FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

```

=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.08
RAINFALL INTENSITY(INCH/HR) = 2.83
TOTAL STREAM AREA(ACRES) = 0.63
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.58

```

```

*****
FLOW PROCESS FROM NODE 43.00 TO NODE 45.00 IS CODE = 21
=====

```

```

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

```

```

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 588.00
UPSTREAM ELEVATION(FEET) = 1718.80
DOWNSTREAM ELEVATION(FEET) = 1709.90
ELEVATION DIFFERENCE(FEET) = 8.90
TC = 0.303*[(588.00**3)/(8.90)]**.2 = 8.981
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.843
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8843
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 1.58
TOTAL AREA(ACRES) = 0.63 TOTAL RUNOFF(CFS) = 1.58

```

```

*****
FLOW PROCESS FROM NODE 45.00 TO NODE 42.00 IS CODE = 41
=====

```

```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 1704.90 DOWNSTREAM(FEET) = 1695.60
FLOW LENGTH(FEET) = 21.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.26
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.58
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 9.00
LONGEST FLOWPATH FROM NODE 43.00 TO NODE 42.00 = 609.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 42.00 TO NODE 42.00 IS CODE = 1
=====

```

```

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

```

```

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.00
RAINFALL INTENSITY(INCH/HR) = 2.84
TOTAL STREAM AREA(ACRES) = 0.63
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.58

```

```

** CONFLUENCE DATA **

```

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	11.20	21.19	1.773	6.35
2	1.58	9.08	2.827	0.63

3 1.58 9.00 2.839 0.63

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.91	9.00	2.839
2	7.95	9.08	2.827
3	13.18	21.19	1.773

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 13.18 Tc (MIN.) = 21.19  
TOTAL AREA (ACRES) = 7.6  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 42.00 = 3441.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 42.00 TO NODE 46.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1695.60 DOWNSTREAM (FEET) = 1694.90  
FLOW LENGTH (FEET) = 31.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 30.0 INCH PIPE IS 9.4 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 9.99  
GIVEN PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 13.18  
PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 21.25  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 46.00 = 3472.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 21.25  
RAINFALL INTENSITY (INCH/HR) = 1.77  
TOTAL STREAM AREA (ACRES) = 7.61  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 13.18

\*\*\*\*\*  
FLOW PROCESS FROM NODE 47.00 TO NODE 48.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC = K \* [(LENGTH\*\*3) / (ELEVATION CHANGE)]\*\*.2  
INITIAL SUBAREA FLOW-LENGTH (FEET) = 2376.00  
UPSTREAM ELEVATION (FEET) = 1722.50

DOWNSTREAM ELEVATION (FEET) = 1718.60  
 ELEVATION DIFFERENCE (FEET) = 3.90  
 $TC = 0.533 * [(2376.00^{**3}) / (3.90)]^{**0.2} = 43.022$   
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 1.201  
 UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6005  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 41.98  
 TOTAL AREA (ACRES) = 58.22 TOTAL RUNOFF (CFS) = 41.98

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 48.00 TO NODE 46.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====  
 ELEVATION DATA: UPSTREAM (FEET) = 1712.60 DOWNSTREAM (FEET) = 1694.90  
 FLOW LENGTH (FEET) = 102.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 10.1 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 28.79  
 GIVEN PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 41.98  
 PIPE TRAVEL TIME (MIN.) = 0.06 Tc (MIN.) = 43.08  
 LONGEST FLOWPATH FROM NODE 47.00 TO NODE 46.00 = 2478.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 46.00 TO NODE 46.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 43.08  
 RAINFALL INTENSITY (INCH/HR) = 1.20  
 TOTAL STREAM AREA (ACRES) = 58.22  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 41.98

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	13.18	21.25	1.770	7.61
2	41.98	43.08	1.200	58.22

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	33.88	21.25	1.770
2	50.91	43.08	1.200

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 50.91 Tc (MIN.) = 43.08

TOTAL AREA (ACRES) = 65.8  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 46.00 = 3472.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 46.00 TO NODE 49.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1694.90 DOWNSTREAM(FEET) = 1664.60  
FLOW LENGTH(FEET) = 1091.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 17.1 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.38  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 50.91  
PIPE TRAVEL TIME(MIN.) = 1.18 Tc(MIN.) = 44.26  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 49.00 = 4563.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 49.00 TO NODE 49.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 44.26  
RAINFALL INTENSITY(INCH/HR) = 1.18  
TOTAL STREAM AREA(ACRES) = 65.83  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 50.91

\*\*\*\*\*  
FLOW PROCESS FROM NODE 18.00 TO NODE 50.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC = K \* [(LENGTH\*\*3) / (ELEVATION CHANGE)]\*\*0.2  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 523.00  
UPSTREAM ELEVATION(FEET) = 1694.70  
DOWNSTREAM ELEVATION(FEET) = 1678.00  
ELEVATION DIFFERENCE(FEET) = 16.70  
TC = 0.303 \* [(523.00\*\*3) / (16.70)]\*\*0.2 = 7.381  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.167  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8857  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.35  
TOTAL AREA(ACRES) = 0.48 TOTAL RUNOFF(CFS) = 1.35

\*\*\*\*\*  
FLOW PROCESS FROM NODE 50.00 TO NODE 49.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1673.00 DOWNSTREAM(FEET) = 1664.60  
FLOW LENGTH(FEET) = 21.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.8 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.93  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 1.35  
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 7.40  
LONGEST FLOWPATH FROM NODE 18.00 TO NODE 49.00 = 544.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 49.00 TO NODE 49.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 7.40  
RAINFALL INTENSITY(INCH/HR) = 3.16  
TOTAL STREAM AREA(ACRES) = 0.48  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.35

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	50.91	44.26	1.182	65.83
2	1.35	7.40	3.162	0.48

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	9.86	7.40	3.162
2	51.42	44.26	1.182

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 51.42 Tc(MIN.) = 44.26  
TOTAL AREA(ACRES) = 66.3  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 49.00 = 4563.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 49.00 TO NODE 51.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1664.60 DOWNSTREAM(FEET) = 1632.50  
FLOW LENGTH(FEET) = 1120.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 17.0 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.60  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 51.42  
PIPE TRAVEL TIME(MIN.) = 1.20 Tc(MIN.) = 45.46  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 51.00 = 5683.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 51.00 TO NODE 51.00 IS CODE = 13

-----  
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<  
=====

+-----+  
| Watershed 2 (Storm Drain Line 2-2) |  
| |  
| |  
+-----+

\*\*\*\*\*  
FLOW PROCESS FROM NODE 50.10 TO NODE 51.10 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 283.00  
UPSTREAM ELEVATION(FEET) = 1734.00  
DOWNSTREAM ELEVATION(FEET) = 1728.00  
ELEVATION DIFFERENCE(FEET) = 6.00  
TC =  $0.533 * [(283.00^{**3}) / (6.00)]^{**.2} = 11.011$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.542  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7283  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.93  
TOTAL AREA(ACRES) = 1.04 TOTAL RUNOFF(CFS) = 1.93

\*\*\*\*\*  
FLOW PROCESS FROM NODE 51.10 TO NODE 52.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) = 1723.00 DOWNSTREAM(FEET) = 1717.00  
FLOW LENGTH(FEET) = 85.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.2 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.04  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.93  
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 11.17  
LONGEST FLOWPATH FROM NODE 50.10 TO NODE 52.00 = 368.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 52.00 TO NODE 53.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) = 1717.00 DOWNSTREAM(FEET) = 1714.30  
FLOW LENGTH(FEET) = 153.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 4.1 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.37  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.93  
PIPE TRAVEL TIME(MIN.) = 0.47 Tc(MIN.) = 11.64  
LONGEST FLOWPATH FROM NODE 50.10 TO NODE 53.00 = 521.00 FEET.

\*\*\*\*\*



FLOW PROCESS FROM NODE 53.00 TO NODE 530.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.64
RAINFALL INTENSITY(INCH/HR) = 2.46
TOTAL STREAM AREA(ACRES) = 1.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.93

\*\*\*\*\*

FLOW PROCESS FROM NODE 30.00 TO NODE 54.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 508.00
UPSTREAM ELEVATION(FEET) = 1734.10
DOWNSTREAM ELEVATION(FEET) = 1723.60
ELEVATION DIFFERENCE(FEET) = 10.50
TC = 0.303\*[(508.00\*\*3)/(10.50)]\*\*.2 = 7.959
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.039
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8852
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 1.43
TOTAL AREA(ACRES) = 0.53 TOTAL RUNOFF(CFS) = 1.43

\*\*\*\*\*

FLOW PROCESS FROM NODE 54.00 TO NODE 53.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1718.60 DOWNSTREAM(FEET) = 1714.30
FLOW LENGTH(FEET) = 27.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.02
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.43
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 8.00
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 53.00 = 535.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.00
RAINFALL INTENSITY(INCH/HR) = 3.03
TOTAL STREAM AREA(ACRES) = 0.53
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.43

\*\*\*\*\*

FLOW PROCESS FROM NODE 30.00 TO NODE 55.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 506.00  
UPSTREAM ELEVATION(FEET) = 1734.10  
DOWNSTREAM ELEVATION(FEET) = 1723.60  
ELEVATION DIFFERENCE(FEET) = 10.50  
TC = 0.303\*[( 506.00\*\*3)/( 10.50)]\*\*.2 = 7.940  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.043  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8852  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.70  
TOTAL AREA(ACRES) = 0.63 TOTAL RUNOFF(CFS) = 1.70

\*\*\*\*\*

FLOW PROCESS FROM NODE 55.00 TO NODE 53.00 IS CODE = 41

-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1718.60 DOWNSTREAM(FEET) = 1714.30  
FLOW LENGTH(FEET) = 37.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.7 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.38  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.70  
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 8.00  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 53.00 = 543.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 53.00 TO NODE 53.00 IS CODE = 1

-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
TIME OF CONCENTRATION(MIN.) = 8.00  
RAINFALL INTENSITY(INCH/HR) = 3.03  
TOTAL STREAM AREA(ACRES) = 0.63  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.70

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.93	11.64	2.465	1.04
2	1.43	8.00	3.030	0.53
3	1.70	8.00	3.030	0.63

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	4.45	8.00	3.030
2	4.45	8.00	3.030
3	4.47	11.64	2.465

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.47 Tc (MIN.) = 11.64  
TOTAL AREA (ACRES) = 2.2  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 53.00 = 543.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 53.00 TO NODE 56.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1714.30 DOWNSTREAM (FEET) = 1704.90  
FLOW LENGTH (FEET) = 557.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 6.3 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.75  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 4.47  
PIPE TRAVEL TIME (MIN.) = 1.37 Tc (MIN.) = 13.02  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 56.00 = 1100.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 56.00 TO NODE 56.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 13.02  
RAINFALL INTENSITY (INCH/HR) = 2.32  
TOTAL STREAM AREA (ACRES) = 2.20  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.47

\*\*\*\*\*  
FLOW PROCESS FROM NODE 57.00 TO NODE 58.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 692.00  
UPSTREAM ELEVATION (FEET) = 1720.00  
DOWNSTREAM ELEVATION (FEET) = 1716.00  
ELEVATION DIFFERENCE (FEET) = 4.00  
TC =  $0.533 * [(692.00 ** 3) / (4.00)] ** .2 = 20.420$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 1.809  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6762  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 11.00  
TOTAL AREA (ACRES) = 8.99 TOTAL RUNOFF (CFS) = 11.00

\*\*\*\*\*  
FLOW PROCESS FROM NODE 58.00 TO NODE 56.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1709.00 DOWNSTREAM(FEET) = 1704.90  
FLOW LENGTH(FEET) = 97.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.1 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.28  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 11.00  
PIPE TRAVEL TIME(MIN.) = 0.13 Tc(MIN.) = 20.55  
LONGEST FLOWPATH FROM NODE 57.00 TO NODE 56.00 = 789.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 56.00 TO NODE 56.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 20.55  
RAINFALL INTENSITY(INCH/HR) = 1.80  
TOTAL STREAM AREA(ACRES) = 8.99  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.00

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.47	13.02	2.318	2.20
2	11.00	20.55	1.803	8.99

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	11.43	13.02	2.318
2	14.47	20.55	1.803

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 14.47 Tc(MIN.) = 20.55  
TOTAL AREA(ACRES) = 11.2  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 56.00 = 1100.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 56.00 TO NODE 62.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1704.90 DOWNSTREAM(FEET) = 1703.40  
FLOW LENGTH(FEET) = 59.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.6 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 10.85  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 14.47  
PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 20.64  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 62.00 = 1159.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 59.00 TO NODE 59.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 20.64  
RAINFALL INTENSITY (INCH/HR) = 1.80  
TOTAL STREAM AREA (ACRES) = 11.19  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 14.47

\*\*\*\*\*  
FLOW PROCESS FROM NODE 60.00 TO NODE 61.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 625.00  
UPSTREAM ELEVATION (FEET) = 1725.00  
DOWNSTREAM ELEVATION (FEET) = 1713.30  
ELEVATION DIFFERENCE (FEET) = 11.70  
TC =  $0.303 * [(625.00 ** 3) / (11.70)] ** .2 = 8.820$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.872  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8845  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 1.73  
TOTAL AREA (ACRES) = 0.68 TOTAL RUNOFF (CFS) = 1.73

\*\*\*\*\*  
FLOW PROCESS FROM NODE 61.00 TO NODE 59.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1708.30 DOWNSTREAM (FEET) = 1703.40  
FLOW LENGTH (FEET) = 26.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.4 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 12.39  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.73  
PIPE TRAVEL TIME (MIN.) = 0.03 Tc (MIN.) = 8.86  
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 59.00 = 651.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 59.00 TO NODE 59.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 8.86  
RAINFALL INTENSITY(INCH/HR) = 2.87  
TOTAL STREAM AREA(ACRES) = 0.68  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.73

\*\*\*\*\*  
FLOW PROCESS FROM NODE 60.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 615.00  
UPSTREAM ELEVATION(FEET) = 1725.00  
DOWNSTREAM ELEVATION(FEET) = 1713.30  
ELEVATION DIFFERENCE(FEET) = 11.70  
TC =  $0.303 * [(615.00^{**3}) / (11.70)]^{**.2} = 8.735$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.887  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8845  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.71  
TOTAL AREA(ACRES) = 0.67 TOTAL RUNOFF(CFS) = 1.71

\*\*\*\*\*  
FLOW PROCESS FROM NODE 62.00 TO NODE 59.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1708.30 DOWNSTREAM(FEET) = 1703.40  
FLOW LENGTH(FEET) = 43.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.7 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.35  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.71  
PIPE TRAVEL TIME(MIN.) = 0.07 Tc(MIN.) = 8.80  
LONGEST FLOWPATH FROM NODE 60.00 TO NODE 59.00 = 658.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 59.00 TO NODE 59.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
TIME OF CONCENTRATION(MIN.) = 8.80  
RAINFALL INTENSITY(INCH/HR) = 2.87  
TOTAL STREAM AREA(ACRES) = 0.67  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.71

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	14.47	20.64	1.799	11.19
2	1.73	8.86	2.865	0.68
3	1.71	8.80	2.874	0.67

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	9.60	8.80	2.874
2	9.64	8.86	2.865
3	16.63	20.64	1.799

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 16.63 Tc (MIN.) = 20.64  
TOTAL AREA (ACRES) = 12.5  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 59.00 = 1159.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 59.00 TO NODE 63.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1703.40 DOWNSTREAM (FEET) = 1703.00  
FLOW LENGTH (FEET) = 52.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.8 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.07  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 16.63  
PIPE TRAVEL TIME (MIN.) = 0.12 Tc (MIN.) = 20.76  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 63.00 = 1211.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 63.00 TO NODE 63.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 20.76  
RAINFALL INTENSITY (INCH/HR) = 1.79  
TOTAL STREAM AREA (ACRES) = 12.54  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 16.63

\*\*\*\*\*

FLOW PROCESS FROM NODE 64.00 TO NODE 65.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 293.00  
UPSTREAM ELEVATION (FEET) = 1715.00  
DOWNSTREAM ELEVATION (FEET) = 1713.00  
ELEVATION DIFFERENCE (FEET) = 2.00  
TC =  $0.533 * [(293.00 ** 3) / (2.00)] ** .2 = 14.006$

100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.227  
 UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7092  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 2.68  
 TOTAL AREA (ACRES) = 1.70 TOTAL RUNOFF (CFS) = 2.68

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 65.00 TO NODE 63.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1706.50 DOWNSTREAM (FEET) = 1703.00  
 FLOW LENGTH (FEET) = 667.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 6.6 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.85  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 2.68  
 PIPE TRAVEL TIME (MIN.) = 2.89 Tc (MIN.) = 16.90  
 LONGEST FLOWPATH FROM NODE 64.00 TO NODE 63.00 = 960.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 63.00 TO NODE 63.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 16.90  
 RAINFALL INTENSITY (INCH/HR) = 2.01  
 TOTAL STREAM AREA (ACRES) = 1.70  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.68

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	16.63	20.76	1.793	12.54
2	2.68	16.90	2.008	1.70

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	16.21	16.90	2.008
2	19.02	20.76	1.793

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 19.02 Tc (MIN.) = 20.76  
 TOTAL AREA (ACRES) = 14.2  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 63.00 = 1211.00 FEET.



\*\*\*\*\*

FLOW PROCESS FROM NODE 63.00 TO NODE 64.10 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1703.00 DOWNSTREAM(FEET) = 1702.60
FLOW LENGTH(FEET) = 60.00 MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.06
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 19.02
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 20.93
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 64.10 = 1271.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 64.10 TO NODE 64.10 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 20.93
RAINFALL INTENSITY(INCH/HR) = 1.79
TOTAL STREAM AREA(ACRES) = 14.24
PEAK FLOW RATE(CFS) AT CONFLUENCE = 19.02

\*\*\*\*\*

FLOW PROCESS FROM NODE 65.10 TO NODE 66.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 1215.00
UPSTREAM ELEVATION(FEET) = 1720.20
DOWNSTREAM ELEVATION(FEET) = 1714.00
ELEVATION DIFFERENCE(FEET) = 6.20
TC = 0.303\*[(1215.00\*\*3)/(6.20)]\*\*.2 = 14.923
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.150
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8804
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 2.50
TOTAL AREA(ACRES) = 1.32 TOTAL RUNOFF(CFS) = 2.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 66.00 TO NODE 64.10 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1709.00 DOWNSTREAM(FEET) = 1702.60
FLOW LENGTH(FEET) = 23.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.83
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.50
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 14.95

LONGEST FLOWPATH FROM NODE 65.10 TO NODE 64.10 = 1238.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 64.10 TO NODE 64.10 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 14.95  
RAINFALL INTENSITY(INCH/HR) = 2.15  
TOTAL STREAM AREA(ACRES) = 1.32  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 65.10 TO NODE 67.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 1215.00  
UPSTREAM ELEVATION(FEET) = 1720.20  
DOWNSTREAM ELEVATION(FEET) = 1714.00  
ELEVATION DIFFERENCE(FEET) = 6.20  
TC =  $0.303 * [(1215.00^{**3}) / (6.20)]^{**2}$  = 14.923  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.150  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8804  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.50  
TOTAL AREA(ACRES) = 1.32 TOTAL RUNOFF(CFS) = 2.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 67.00 TO NODE 64.10 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1709.00 DOWNSTREAM(FEET) = 1702.60  
FLOW LENGTH(FEET) = 34.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.9 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.81  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.50  
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 14.96  
LONGEST FLOWPATH FROM NODE 65.10 TO NODE 64.10 = 1249.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 64.10 TO NODE 64.10 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
TIME OF CONCENTRATION(MIN.) = 14.96  
RAINFALL INTENSITY(INCH/HR) = 2.15  
TOTAL STREAM AREA(ACRES) = 1.32  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.50

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	19.02	20.93	1.785	14.24
2	2.50	14.95	2.148	1.32
3	2.50	14.96	2.147	1.32

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	18.58	14.95	2.148
2	18.60	14.96	2.147
3	23.18	20.93	1.785

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 23.18 Tc (MIN.) = 20.93  
 TOTAL AREA (ACRES) = 16.9  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 64.10 = 1271.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 64.10 TO NODE 68.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1702.60 DOWNSTREAM (FEET) = 1693.60  
 FLOW LENGTH (FEET) = 2142.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.38  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 23.18  
 PIPE TRAVEL TIME (MIN.) = 4.84 Tc (MIN.) = 25.77  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 68.00 = 3413.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 68.00 TO NODE 68.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 25.77  
 RAINFALL INTENSITY (INCH/HR) = 1.59  
 TOTAL STREAM AREA (ACRES) = 16.88  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 23.18

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 69.00 TO NODE 70.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====
ASSUMED INITIAL SUBAREA UNIFORM

DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2

INITIAL SUBAREA FLOW-LENGTH(FEET) = 996.00

UPSTREAM ELEVATION(FEET) = 1711.70

DOWNSTREAM ELEVATION(FEET) = 1706.70

ELEVATION DIFFERENCE(FEET) = 5.00

TC = 0.533\*[(996.00\*\*3)/(5.00)]\*\*.2 = 24.298

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.644

UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6597

SOIL CLASSIFICATION IS "C"

SUBAREA RUNOFF(CFS) = 24.97

TOTAL AREA(ACRES) = 23.02 TOTAL RUNOFF(CFS) = 24.97

\*\*\*\*\*

FLOW PROCESS FROM NODE 70.00 TO NODE 68.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1701.70 DOWNSTREAM(FEET) = 1693.60

FLOW LENGTH(FEET) = 102.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.4 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 19.06

GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 24.97

PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 24.39

LONGEST FLOWPATH FROM NODE 69.00 TO NODE 68.00 = 1098.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 68.00 TO NODE 68.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 24.39

RAINFALL INTENSITY(INCH/HR) = 1.64

TOTAL STREAM AREA(ACRES) = 23.02

PEAK FLOW RATE(CFS) AT CONFLUENCE = 24.97

\*\* CONFLUENCE DATA \*\*

Table with 5 columns: STREAM NUMBER, RUNOFF (CFS), Tc (MIN.), INTENSITY (INCH/HOUR), AREA (ACRE). Rows 1 and 2.

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	46.91	24.39	1.641
2	47.40	25.77	1.592

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 46.91 Tc (MIN.) = 24.39  
 TOTAL AREA (ACRES) = 39.9  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 68.00 = 3413.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 68.00 TO NODE 71.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1693.60 DOWNSTREAM (FEET) = 1693.40  
 FLOW LENGTH (FEET) = 40.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 29.3 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.61  
 GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 46.91  
 PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 24.47  
 LONGEST FLOWPATH FROM NODE 30.00 TO NODE 71.00 = 3453.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 71.00 TO NODE 71.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 24.47  
 RAINFALL INTENSITY (INCH/HR) = 1.64  
 TOTAL STREAM AREA (ACRES) = 39.90  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 46.91

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 65.10 TO NODE 72.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
 DEVELOPMENT IS COMMERCIAL  
 $TC = K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 967.00  
 UPSTREAM ELEVATION (FEET) = 1720.20  
 DOWNSTREAM ELEVATION (FEET) = 1710.60  
 ELEVATION DIFFERENCE (FEET) = 9.60  
 $TC = 0.303 * [(967.00 ** 3) / (9.60)] ** .2 = 11.923$   
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.433  
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8822  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 2.83  
 TOTAL AREA (ACRES) = 1.32 TOTAL RUNOFF (CFS) = 2.83

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 72.00 TO NODE 71.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1710.60 DOWNSTREAM(FEET) = 1693.40  
FLOW LENGTH(FEET) = 37.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.4 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.71  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.83  
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 11.95  
LONGEST FLOWPATH FROM NODE 65.10 TO NODE 71.00 = 1004.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 71.00 TO NODE 71.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 11.95  
RAINFALL INTENSITY(INCH/HR) = 2.43  
TOTAL STREAM AREA(ACRES) = 1.32  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.83

\*\*\*\*\*  
FLOW PROCESS FROM NODE 65.10 TO NODE 73.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
 $TC = K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**0.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 967.00  
UPSTREAM ELEVATION(FEET) = 1720.20  
DOWNSTREAM ELEVATION(FEET) = 1710.60  
ELEVATION DIFFERENCE(FEET) = 9.60  
 $TC = 0.303 * [(967.00^{**3}) / (9.60)]^{**0.2} = 11.923$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.433  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8822  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.83  
TOTAL AREA(ACRES) = 1.32 TOTAL RUNOFF(CFS) = 2.83

\*\*\*\*\*  
FLOW PROCESS FROM NODE 73.00 TO NODE 71.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1705.60 DOWNSTREAM(FEET) = 1693.40  
FLOW LENGTH(FEET) = 28.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.5 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.28  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.83  
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 11.95  
LONGEST FLOWPATH FROM NODE 65.10 TO NODE 71.00 = 995.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 71.00 TO NODE 71.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
TIME OF CONCENTRATION(MIN.) = 11.95  
RAINFALL INTENSITY(INCH/HR) = 2.43  
TOTAL STREAM AREA(ACRES) = 1.32  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.83

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	46.91	24.47	1.638	39.90
2	2.83	11.95	2.429	1.32
3	2.83	11.95	2.430	1.32

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	28.56	11.95	2.430
2	28.58	11.95	2.429
3	50.72	24.47	1.638

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 50.72 Tc(MIN.) = 24.47  
TOTAL AREA(ACRES) = 42.5  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 71.00 = 3453.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 71.00 TO NODE 74.00 IS CODE = 41  
\*\*\*\*\*

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1693.40 DOWNSTREAM(FEET) = 1684.50  
FLOW LENGTH(FEET) = 548.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 20.0 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.55  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 50.72  
PIPE TRAVEL TIME(MIN.) = 0.73 Tc(MIN.) = 25.20  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 74.00 = 4001.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1  
\*\*\*\*\*

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 25.20  
RAINFALL INTENSITY(INCH/HR) = 1.61  
TOTAL STREAM AREA(ACRES) = 42.54  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 50.72

\*\*\*\*\*  
FLOW PROCESS FROM NODE 75.00 TO NODE 76.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 333.00  
UPSTREAM ELEVATION(FEET) = 1709.10  
DOWNSTREAM ELEVATION(FEET) = 1694.10  
ELEVATION DIFFERENCE(FEET) = 15.00  
TC =  $0.303 * [(333.00 ** 3) / (15.00)] ** .2 = 5.752$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.633  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8873  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.42  
TOTAL AREA(ACRES) = 0.44 TOTAL RUNOFF(CFS) = 1.42

\*\*\*\*\*  
FLOW PROCESS FROM NODE 76.00 TO NODE 74.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1689.10 DOWNSTREAM(FEET) = 1684.50  
FLOW LENGTH(FEET) = 32.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.3 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.60  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.42  
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 5.80  
LONGEST FLOWPATH FROM NODE 75.00 TO NODE 74.00 = 365.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 74.00 TO NODE 74.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 5.80  
RAINFALL INTENSITY(INCH/HR) = 3.62  
TOTAL STREAM AREA(ACRES) = 0.44  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.42

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	50.72	25.20	1.612	42.54
2	1.42	5.80	3.616	0.44

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED



ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	13.10	5.80	3.616
2	51.36	25.20	1.612

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 51.36 Tc (MIN.) = 25.20  
TOTAL AREA (ACRES) = 43.0  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 74.00 = 4001.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 74.00 TO NODE 77.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1684.50 DOWNSTREAM (FEET) = 1664.60  
FLOW LENGTH (FEET) = 570.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 16.1 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 16.78  
GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 51.36  
PIPE TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 25.77  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 77.00 = 4571.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 77.00 TO NODE 77.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 25.77  
RAINFALL INTENSITY (INCH/HR) = 1.59  
TOTAL STREAM AREA (ACRES) = 42.98  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 51.36

\*\*\*\*\*

FLOW PROCESS FROM NODE 18.00 TO NODE 78.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 518.00  
UPSTREAM ELEVATION (FEET) = 1694.70  
DOWNSTREAM ELEVATION (FEET) = 1678.00  
ELEVATION DIFFERENCE (FEET) = 16.70  
TC =  $0.303 * [(518.00 ** 3) / (16.70)] ** .2 = 7.339$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.177  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8857

SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.39  
TOTAL AREA(ACRES) = 0.85 TOTAL RUNOFF(CFS) = 2.39

\*\*\*\*\*  
FLOW PROCESS FROM NODE 78.00 TO NODE 77.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1673.00 DOWNSTREAM(FEET) = 1664.60  
FLOW LENGTH(FEET) = 26.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.5 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.47  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.39  
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 7.37  
LONGEST FLOWPATH FROM NODE 18.00 TO NODE 77.00 = 544.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 77.00 TO NODE 77.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 7.37  
RAINFALL INTENSITY(INCH/HR) = 3.17  
TOTAL STREAM AREA(ACRES) = 0.85  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.39

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	51.36	25.77	1.592	42.98
2	2.39	7.37	3.171	0.85

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	17.07	7.37	3.171
2	52.56	25.77	1.592

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
PEAK FLOW RATE(CFS) = 52.56 Tc(MIN.) = 25.77  
TOTAL AREA(ACRES) = 43.8  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 77.00 = 4571.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 77.00 TO NODE 79.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET)	=	1664.60	DOWNSTREAM(FEET)	=	1657.60
FLOW LENGTH(FEET)	=	200.00	MANNING'S N	=	0.013
DEPTH OF FLOW IN	36.0 INCH PIPE IS	16.3 INCHES			
PIPE-FLOW VELOCITY(FEET/SEC.)	=	16.90			
GIVEN PIPE DIAMETER(INCH)	=	36.00	NUMBER OF PIPES	=	1
PIPE-FLOW(CFS)	=	52.56			
PIPE TRAVEL TIME(MIN.)	=	0.20	Tc(MIN.)	=	25.97
LONGEST FLOWPATH FROM NODE	30.00 TO NODE	79.00	=	4771.00 FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS	=	2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM	1 ARE:	
TIME OF CONCENTRATION(MIN.)	=	25.97
RAINFALL INTENSITY(INCH/HR)	=	1.59
TOTAL STREAM AREA(ACRES)	=	43.83
PEAK FLOW RATE(CFS) AT CONFLUENCE	=	52.56

\*\*\*\*\*  
FLOW PROCESS FROM NODE 80.00 TO NODE 81.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER

TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2		
INITIAL SUBAREA FLOW-LENGTH(FEET)	=	574.00
UPSTREAM ELEVATION(FEET)	=	1709.70
DOWNSTREAM ELEVATION(FEET)	=	1704.00
ELEVATION DIFFERENCE(FEET)	=	5.70
TC = 0.533*[(574.00**3)/(5.70)]**.2	=	17.005
100 YEAR RAINFALL INTENSITY(INCH/HOUR)	=	2.001
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT	=	.6927
SOIL CLASSIFICATION IS	"C"	
SUBAREA RUNOFF(CFS)	=	6.35
TOTAL AREA(ACRES)	=	4.58
TOTAL RUNOFF(CFS)	=	6.35

\*\*\*\*\*  
FLOW PROCESS FROM NODE 81.00 TO NODE 79.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET)	=	1699.00	DOWNSTREAM(FEET)	=	1657.60
FLOW LENGTH(FEET)	=	163.00	MANNING'S N	=	0.013
DEPTH OF FLOW IN	18.0 INCH PIPE IS	4.2 INCHES			
PIPE-FLOW VELOCITY(FEET/SEC.)	=	20.20			
GIVEN PIPE DIAMETER(INCH)	=	18.00	NUMBER OF PIPES	=	1
PIPE-FLOW(CFS)	=	6.35			
PIPE TRAVEL TIME(MIN.)	=	0.13	Tc(MIN.)	=	17.14
LONGEST FLOWPATH FROM NODE	80.00 TO NODE	79.00	=	737.00 FEET.	

\*\*\*\*\*

FLOW PROCESS FROM NODE 79.00 TO NODE 79.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 17.14  
RAINFALL INTENSITY(INCH/HR) = 1.99  
TOTAL STREAM AREA(ACRES) = 4.58  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.35

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	52.56	25.97	1.585	43.83
2	6.35	17.14	1.992	4.58

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	41.04	17.14	1.992
2	57.61	25.97	1.585

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 57.61 Tc(MIN.) = 25.97  
TOTAL AREA(ACRES) = 48.4  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 79.00 = 4771.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 79.00 TO NODE 82.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1657.60 DOWNSTREAM(FEET) = 1632.50  
FLOW LENGTH(FEET) = 921.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 18.5 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.75  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 57.61  
PIPE TRAVEL TIME(MIN.) = 0.97 Tc(MIN.) = 26.94  
LONGEST FLOWPATH FROM NODE 30.00 TO NODE 82.00 = 5692.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 82.00 TO NODE 82.00 IS CODE = 13

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<

+-----+

| Watershed 2 (Storm Drain Line 2-3) |

| |  
| |  
+-----+  
\*\*\*\*\*

FLOW PROCESS FROM NODE 83.00 TO NODE 84.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 681.00  
UPSTREAM ELEVATION (FEET) = 1679.30  
DOWNSTREAM ELEVATION (FEET) = 1658.50  
ELEVATION DIFFERENCE (FEET) = 20.80  
TC =  $0.303 * [(681.00^{**3}) / (20.80)]^{**.2} = 8.277$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.974  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8849  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 4.47  
TOTAL AREA (ACRES) = 1.70 TOTAL RUNOFF (CFS) = 4.47

\*\*\*\*\*

FLOW PROCESS FROM NODE 84.00 TO NODE 85.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1653.50 DOWNSTREAM (FEET) = 1648.50  
FLOW LENGTH (FEET) = 23.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.7 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 17.27  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 4.47  
PIPE TRAVEL TIME (MIN.) = 0.02 Tc (MIN.) = 8.30  
LONGEST FLOWPATH FROM NODE 83.00 TO NODE 85.00 = 704.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 85.00 TO NODE 85.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 8.30  
RAINFALL INTENSITY (INCH/HR) = 2.97  
TOTAL STREAM AREA (ACRES) = 1.70  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.47

\*\*\*\*\*

FLOW PROCESS FROM NODE 83.00 TO NODE 86.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 669.00

UPSTREAM ELEVATION (FEET) = 1679.30  
 DOWNSTREAM ELEVATION (FEET) = 1658.50  
 ELEVATION DIFFERENCE (FEET) = 20.80  
 $TC = 0.303 * [(669.00^{**3}) / (20.80)]^{**0.2} = 8.189$   
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.991  
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8850  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 1.64  
 TOTAL AREA (ACRES) = 0.62 TOTAL RUNOFF (CFS) = 1.64

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 86.00 TO NODE 85.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====  
 ELEVATION DATA: UPSTREAM (FEET) = 1653.50 DOWNSTREAM (FEET) = 1648.50  
 FLOW LENGTH (FEET) = 23.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.3 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 12.80  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 1.64  
 PIPE TRAVEL TIME (MIN.) = 0.03 Tc (MIN.) = 8.22  
 LONGEST FLOWPATH FROM NODE 83.00 TO NODE 85.00 = 692.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 85.00 TO NODE 85.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 8.22  
 RAINFALL INTENSITY (INCH/HR) = 2.99  
 TOTAL STREAM AREA (ACRES) = 0.62  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.64

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.47	8.30	2.970	1.70
2	1.64	8.22	2.985	0.62

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	6.07	8.22	2.985
2	6.11	8.30	2.970

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.11 Tc (MIN.) = 8.30  
TOTAL AREA (ACRES) = 2.3  
LONGEST FLOWPATH FROM NODE 83.00 TO NODE 85.00 = 704.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 85.00 TO NODE 87.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1648.50 DOWNSTREAM(FEET) = 1648.30  
FLOW LENGTH(FEET) = 22.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.2 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.94  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 6.11  
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 8.36  
LONGEST FLOWPATH FROM NODE 83.00 TO NODE 87.00 = 726.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 87.00 TO NODE 87.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 8.36  
RAINFALL INTENSITY(INCH/HR) = 2.96  
TOTAL STREAM AREA(ACRES) = 2.32  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.11

\*\*\*\*\*  
FLOW PROCESS FROM NODE 88.00 TO NODE 89.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH**3) / (ELEVATION CHANGE)]**.2$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 500.00  
UPSTREAM ELEVATION(FEET) = 1706.70  
DOWNSTREAM ELEVATION(FEET) = 1699.00  
ELEVATION DIFFERENCE(FEET) = 7.70  
TC =  $0.533 * [(500.00**3) / (7.70)]**.2 = 14.740$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.165  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7049  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 5.37  
TOTAL AREA(ACRES) = 3.52 TOTAL RUNOFF(CFS) = 5.37

\*\*\*\*\*  
FLOW PROCESS FROM NODE 89.00 TO NODE 87.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1694.00 DOWNSTREAM(FEET) = 1648.30  
FLOW LENGTH(FEET) = 164.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.8 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.89

GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 5.37  
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 14.88  
LONGEST FLOWPATH FROM NODE 88.00 TO NODE 87.00 = 664.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 87.00 TO NODE 87.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 14.88  
RAINFALL INTENSITY(INCH/HR) = 2.15  
TOTAL STREAM AREA(ACRES) = 3.52  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.37

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.11	8.36	2.957	2.32
2	5.37	14.88	2.154	3.52

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	9.13	8.36	2.957
2	9.82	14.88	2.154

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 9.13 Tc(MIN.) = 8.36  
TOTAL AREA(ACRES) = 5.8  
LONGEST FLOWPATH FROM NODE 83.00 TO NODE 87.00 = 726.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 87.00 TO NODE 90.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1648.30 DOWNSTREAM(FEET) = 1644.50  
FLOW LENGTH(FEET) = 114.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.7 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.72  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 9.13  
PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 8.54  
LONGEST FLOWPATH FROM NODE 83.00 TO NODE 90.00 = 840.00 FEET.

\*\*\*\*\*



FLOW PROCESS FROM NODE 90.00 TO NODE 90.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

\*\*\*\*\*

FLOW PROCESS FROM NODE 90.00 TO NODE 90.00 IS CODE = 13

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<

\*\*\*\*\*

FLOW PROCESS FROM NODE 91.00 TO NODE 92.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM DEVELOPMENT IS COMMERCIAL

TC = K \* [(LENGTH\*\*3) / (ELEVATION CHANGE)]\*\*.2  
INITIAL SUBAREA FLOW-LENGTH (FEET) = 421.00  
UPSTREAM ELEVATION (FEET) = 1659.10  
DOWNSTREAM ELEVATION (FEET) = 1647.50  
ELEVATION DIFFERENCE (FEET) = 11.60  
TC = 0.303 \* [( 421.00\*\*3) / ( 11.60)]\*\*.2 = 6.970  
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.269  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8861  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 1.13  
TOTAL AREA (ACRES) = 0.39 TOTAL RUNOFF (CFS) = 1.13

\*\*\*\*\*

FLOW PROCESS FROM NODE 92.00 TO NODE 93.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1644.00 DOWNSTREAM (FEET) = 1643.80  
FLOW LENGTH (FEET) = 23.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.1 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.69  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.13  
PIPE TRAVEL TIME (MIN.) = 0.10 Tc (MIN.) = 7.07  
LONGEST FLOWPATH FROM NODE 91.00 TO NODE 93.00 = 444.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 93.00 TO NODE 93.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 7.07  
RAINFALL INTENSITY (INCH/HR) = 3.24  
TOTAL STREAM AREA (ACRES) = 0.39  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.13

\*\*\*\*\*

FLOW PROCESS FROM NODE 91.00 TO NODE 94.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====
ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 421.00
UPSTREAM ELEVATION(FEET) = 1659.10
DOWNSTREAM ELEVATION(FEET) = 1647.50
ELEVATION DIFFERENCE(FEET) = 11.60
TC = 0.303\*[( 421.00\*\*3)/( 11.60)]\*\*.2 = 6.970
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.269
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8861
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 1.13
TOTAL AREA(ACRES) = 0.39 TOTAL RUNOFF(CFS) = 1.13

\*\*\*\*\*
FLOW PROCESS FROM NODE 94.00 TO NODE 93.00 IS CODE = 41
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====
ELEVATION DATA: UPSTREAM(FEET) = 1644.00 DOWNSTREAM(FEET) = 1643.80
FLOW LENGTH(FEET) = 23.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.69
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.13
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 7.07
LONGEST FLOWPATH FROM NODE 91.00 TO NODE 93.00 = 444.00 FEET.

\*\*\*\*\*
FLOW PROCESS FROM NODE 93.00 TO NODE 93.00 IS CODE = 1
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.07
RAINFALL INTENSITY(INCH/HR) = 3.24
TOTAL STREAM AREA(ACRES) = 0.39
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.13

\*\* CONFLUENCE DATA \*\*

Table with 5 columns: STREAM NUMBER, RUNOFF (CFS), Tc (MIN.), INTENSITY (INCH/HOUR), AREA (ACRE). Rows 1 and 2.

\*\*\*\*\*WARNING\*\*\*\*\*
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	2.26	7.07	3.242
2	2.26	7.07	3.242

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 2.26 Tc (MIN.) = 7.07  
 TOTAL AREA (ACRES) = 0.8  
 LONGEST FLOWPATH FROM NODE 91.00 TO NODE 93.00 = 444.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 93.00 TO NODE 90.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1645.50 DOWNSTREAM (FEET) = 1644.50  
 FLOW LENGTH (FEET) = 327.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.8 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.08  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 2.26  
 PIPE TRAVEL TIME (MIN.) = 1.77 Tc (MIN.) = 8.85  
 LONGEST FLOWPATH FROM NODE 91.00 TO NODE 90.00 = 771.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 90.00 TO NODE 90.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 8.85  
 RAINFALL INTENSITY (INCH/HR) = 2.87  
 TOTAL STREAM AREA (ACRES) = 0.78  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.26

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 95.00 TO NODE 96.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
 DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
 $TC = K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**0.2}$   
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 269.00  
 UPSTREAM ELEVATION (FEET) = 1665.00  
 DOWNSTREAM ELEVATION (FEET) = 1650.00  
 ELEVATION DIFFERENCE (FEET) = 15.00  
 $TC = 0.533 * [(269.00^{**3}) / (15.00)]^{**0.2} = 8.893$   
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.859  
 UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7441  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 4.74  
 TOTAL AREA (ACRES) = 2.23 TOTAL RUNOFF (CFS) = 4.74

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 96.00 TO NODE 90.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1650.00 DOWNSTREAM(FEET) = 1644.50  
FLOW LENGTH(FEET) = 109.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 5.0 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.14  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 4.74  
PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 9.07  
LONGEST FLOWPATH FROM NODE 95.00 TO NODE 90.00 = 378.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 90.00 TO NODE 90.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

-----  
TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 9.07  
RAINFALL INTENSITY(INCH/HR) = 2.83  
TOTAL STREAM AREA(ACRES) = 2.23  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.74

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.26	8.85	2.867	0.78
2	4.74	9.07	2.828	2.23

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	6.88	8.85	2.867
2	6.97	9.07	2.828

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 6.97 Tc(MIN.) = 9.07  
TOTAL AREA(ACRES) = 3.0  
LONGEST FLOWPATH FROM NODE 91.00 TO NODE 90.00 = 771.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 90.00 TO NODE 90.00 IS CODE = 11  
-----

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

-----  
\*\* MAIN STREAM CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.97	9.07	2.828	3.01

LONGEST FLOWPATH FROM NODE 91.00 TO NODE 90.00 = 771.00 FEET.

\*\* MEMORY BANK # 2 CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	9.13	8.54	2.923	5.84

LONGEST FLOWPATH FROM NODE 83.00 TO NODE 90.00 = 840.00 FEET.

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	15.69	8.54	2.923
2	15.80	9.07	2.828

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 15.69 Tc (MIN.) = 8.54  
 TOTAL AREA (ACRES) = 8.9

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 90.00 TO NODE 95.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1644.50 DOWNSTREAM (FEET) = 1644.10  
 FLOW LENGTH (FEET) = 99.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.99  
 PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 15.69  
 PIPE TRAVEL TIME (MIN.) = 0.33 Tc (MIN.) = 8.87  
 LONGEST FLOWPATH FROM NODE 83.00 TO NODE 95.00 = 939.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 95.00 TO NODE 95.00 IS CODE = 13  
 -----

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<

+-----+  
 | Watershed 3 |  
 | |  
 | |  
 +-----+

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM DEVELOPMENT IS COMMERCIAL  
 TC = K \* [(LENGTH\*\*3) / (ELEVATION CHANGE)]\*\*.2

INITIAL SUBAREA FLOW-LENGTH(FEET) = 572.00  
UPSTREAM ELEVATION(FEET) = 1710.60  
DOWNSTREAM ELEVATION(FEET) = 1706.60  
ELEVATION DIFFERENCE(FEET) = 4.00  
TC = 0.303\*[( 572.00\*\*3)/( 4.00)]\*\*.2 = 10.366  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.628  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8833  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.62  
TOTAL AREA(ACRES) = 0.70 TOTAL RUNOFF(CFS) = 1.62

\*\*\*\*\*  
FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1696.00 DOWNSTREAM(FEET) = 1695.80  
FLOW LENGTH(FEET) = 26.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.1 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.92  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.62  
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 10.48  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 598.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 102.00 TO NODE 102.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 10.48  
RAINFALL INTENSITY(INCH/HR) = 2.61  
TOTAL STREAM AREA(ACRES) = 0.70  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.62

\*\*\*\*\*  
FLOW PROCESS FROM NODE 100.00 TO NODE 103.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 591.00  
UPSTREAM ELEVATION(FEET) = 1710.60  
DOWNSTREAM ELEVATION(FEET) = 1706.60  
ELEVATION DIFFERENCE(FEET) = 4.00  
TC = 0.303\*[( 591.00\*\*3)/( 4.00)]\*\*.2 = 10.571  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.599  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8831  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.77  
TOTAL AREA(ACRES) = 0.77 TOTAL RUNOFF(CFS) = 1.77

\*\*\*\*\*  
FLOW PROCESS FROM NODE 103.00 TO NODE 102.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1696.00 DOWNSTREAM(FEET) = 1695.80  
FLOW LENGTH(FEET) = 26.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.3 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.03  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.77  
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 10.68  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 617.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 102.00 TO NODE 102.00 IS CODE = 1

-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 10.68  
RAINFALL INTENSITY(INCH/HR) = 2.58  
TOTAL STREAM AREA(ACRES) = 0.77  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.77

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.62	10.48	2.612	0.70
2	1.77	10.68	2.585	0.77

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	3.36	10.48	2.612
2	3.38	10.68	2.585

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
PEAK FLOW RATE(CFS) = 3.38 Tc(MIN.) = 10.68  
TOTAL AREA(ACRES) = 1.5  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 617.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 102.00 TO NODE 104.00 IS CODE = 41

-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1695.80 DOWNSTREAM(FEET) = 1689.90  
FLOW LENGTH(FEET) = 587.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.0 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 5.30  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 3.38  
PIPE TRAVEL TIME (MIN.) = 1.85 Tc (MIN.) = 12.52  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 1204.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 104.00 TO NODE 104.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 12.52  
RAINFALL INTENSITY (INCH/HR) = 2.37  
TOTAL STREAM AREA (ACRES) = 1.47  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.38

\*\*\*\*\*  
FLOW PROCESS FROM NODE 105.00 TO NODE 106.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 583.00  
UPSTREAM ELEVATION (FEET) = 1707.30  
DOWNSTREAM ELEVATION (FEET) = 1703.70  
ELEVATION DIFFERENCE (FEET) = 3.60  
TC =  $0.303 * [(583.00^{**3}) / (3.60)]^{**2}$  = 10.708  
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.581  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8830  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 3.17  
TOTAL AREA (ACRES) = 1.39 TOTAL RUNOFF (CFS) = 3.17

\*\*\*\*\*  
FLOW PROCESS FROM NODE 106.00 TO NODE 104.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1698.70 DOWNSTREAM (FEET) = 1689.90  
FLOW LENGTH (FEET) = 44.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.2 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 15.13  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 3.17  
PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 10.76  
LONGEST FLOWPATH FROM NODE 105.00 TO NODE 104.00 = 627.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 104.00 TO NODE 104.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:



TIME OF CONCENTRATION(MIN.) = 10.76  
 RAINFALL INTENSITY(INCH/HR) = 2.57  
 TOTAL STREAM AREA(ACRES) = 1.39  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.17

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	3.38	12.52	2.368	1.47
2	3.17	10.76	2.575	1.39

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	6.07	10.76	2.575
2	6.29	12.52	2.368

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE(CFS) = 6.29 Tc(MIN.) = 12.52  
 TOTAL AREA(ACRES) = 2.9  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 1204.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 104.00 TO NODE 107.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====  
 ELEVATION DATA: UPSTREAM(FEET) = 1689.90 DOWNSTREAM(FEET) = 1689.10  
 FLOW LENGTH(FEET) = 76.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 8.5 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.28  
 GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 6.29  
 PIPE TRAVEL TIME(MIN.) = 0.20 Tc(MIN.) = 12.73  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 107.00 = 1280.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 107.00 TO NODE 107.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION(MIN.) = 12.73  
 RAINFALL INTENSITY(INCH/HR) = 2.35  
 TOTAL STREAM AREA(ACRES) = 2.86  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.29

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 105.00 TO NODE 108.00 IS CODE = 21

=====  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 652.00  
UPSTREAM ELEVATION(FEET) = 1707.30  
DOWNSTREAM ELEVATION(FEET) = 1703.20  
ELEVATION DIFFERENCE(FEET) = 4.10  
TC = 0.303\*[(652.00\*\*3)/(4.10)]\*\*.2 = 11.158  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.523  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8827  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 3.52  
TOTAL AREA(ACRES) = 1.58 TOTAL RUNOFF(CFS) = 3.52

\*\*\*\*\*  
FLOW PROCESS FROM NODE 108.00 TO NODE 107.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) = 1698.20 DOWNSTREAM(FEET) = 1689.10  
FLOW LENGTH(FEET) = 47.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.4 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.45  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 3.52  
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 11.21  
LONGEST FLOWPATH FROM NODE 105.00 TO NODE 107.00 = 699.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 107.00 TO NODE 107.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<  
=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 11.21  
RAINFALL INTENSITY(INCH/HR) = 2.52  
TOTAL STREAM AREA(ACRES) = 1.58  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.52

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.29	12.73	2.347	2.86
2	3.52	11.21	2.517	1.58

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	9.06	11.21	2.517
2	9.57	12.73	2.347

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 9.57 Tc (MIN.) = 12.73  
TOTAL AREA (ACRES) = 4.4  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 107.00 = 1280.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 107.00 TO NODE 109.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1689.10 DOWNSTREAM (FEET) = 1684.60  
FLOW LENGTH (FEET) = 440.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.8 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.96  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 9.57  
PIPE TRAVEL TIME (MIN.) = 1.05 Tc (MIN.) = 13.78  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 109.00 = 1720.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 109.00 TO NODE 109.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 13.78  
RAINFALL INTENSITY (INCH/HR) = 2.25  
TOTAL STREAM AREA (ACRES) = 4.44  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 9.57

\*\*\*\*\*  
FLOW PROCESS FROM NODE 110.00 TO NODE 111.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 780.00  
UPSTREAM ELEVATION (FEET) = 1707.90  
DOWNSTREAM ELEVATION (FEET) = 1698.00  
ELEVATION DIFFERENCE (FEET) = 9.90  
TC =  $0.533 * [(780.00 ** 3) / (9.90)] ** .2 = 18.303$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 1.922  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6861  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 11.25  
TOTAL AREA (ACRES) = 8.53 TOTAL RUNOFF (CFS) = 11.25

\*\*\*\*\*  
FLOW PROCESS FROM NODE 111.00 TO NODE 109.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1690.00 DOWNSTREAM(FEET) = 1684.60  
FLOW LENGTH(FEET) = 45.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 6.1 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 17.72  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 11.25  
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 18.35  
LONGEST FLOWPATH FROM NODE 110.00 TO NODE 109.00 = 825.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 109.00 TO NODE 109.00 IS CODE = 1

-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 18.35  
RAINFALL INTENSITY(INCH/HR) = 1.92  
TOTAL STREAM AREA(ACRES) = 8.53  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.25

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	9.57	13.78	2.247	4.44
2	11.25	18.35	1.919	8.53

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	18.02	13.78	2.247
2	19.42	18.35	1.919

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
PEAK FLOW RATE(CFS) = 19.42 Tc(MIN.) = 18.35  
TOTAL AREA(ACRES) = 13.0  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 109.00 = 1720.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 109.00 TO NODE 112.00 IS CODE = 41

-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1684.60 DOWNSTREAM(FEET) = 1645.00  
FLOW LENGTH(FEET) = 152.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 30.0 INCH PIPE IS 6.2 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 26.68  
GIVEN PIPE DIAMETER (INCH) = 30.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 19.42  
PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 18.44  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 112.00 = 1872.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 112.00 TO NODE 112.00 IS CODE = 13

-----  
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<  
=====

+-----+  
| Watershed 5 |  
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\*\*\*\*\*  
FLOW PROCESS FROM NODE 120.00 TO NODE 121.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 550.00  
UPSTREAM ELEVATION (FEET) = 1704.80  
DOWNSTREAM ELEVATION (FEET) = 1701.90  
ELEVATION DIFFERENCE (FEET) = 2.90  
TC =  $0.303 * [(550.00^{**3}) / (2.90)]^{**2}$  = 10.798  
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.569  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8830  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 1.34  
TOTAL AREA (ACRES) = 0.59 TOTAL RUNOFF (CFS) = 1.34

\*\*\*\*\*  
FLOW PROCESS FROM NODE 121.00 TO NODE 122.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM (FEET) = 1696.90 DOWNSTREAM (FEET) = 1696.00  
FLOW LENGTH (FEET) = 37.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.5 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.58  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.34  
PIPE TRAVEL TIME (MIN.) = 0.11 Tc (MIN.) = 10.91  
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 122.00 = 587.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 122.00 TO NODE 122.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 10.91

RAINFALL INTENSITY (INCH/HR) = 2.55  
TOTAL STREAM AREA (ACRES) = 0.59  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.34

\*\*\*\*\*  
FLOW PROCESS FROM NODE 120.00 TO NODE 123.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 549.00  
UPSTREAM ELEVATION (FEET) = 1704.80  
DOWNSTREAM ELEVATION (FEET) = 1701.90  
ELEVATION DIFFERENCE (FEET) = 2.90  
TC =  $0.303 * [(549.00^{**3}) / (2.90)]^{**2} = 10.786$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.571  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8830  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 1.34  
TOTAL AREA (ACRES) = 0.59 TOTAL RUNOFF (CFS) = 1.34

\*\*\*\*\*  
FLOW PROCESS FROM NODE 123.00 TO NODE 122.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1696.90 DOWNSTREAM (FEET) = 1696.00  
FLOW LENGTH (FEET) = 27.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.2 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.25  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.34  
PIPE TRAVEL TIME (MIN.) = 0.07 Tc (MIN.) = 10.86  
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 122.00 = 576.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 122.00 TO NODE 122.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION (MIN.) = 10.86  
RAINFALL INTENSITY (INCH/HR) = 2.56  
TOTAL STREAM AREA (ACRES) = 0.59  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.34

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.34	10.91	2.555	0.59
2	1.34	10.86	2.561	0.59

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA

WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	2.67	10.86	2.561
2	2.67	10.91	2.555

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 2.67 Tc (MIN.) = 10.86  
TOTAL AREA (ACRES) = 1.2  
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 122.00 = 587.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 122.00 TO NODE 124.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1696.00 DOWNSTREAM (FEET) = 1690.60  
FLOW LENGTH (FEET) = 540.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.2 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.96  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 2.67  
PIPE TRAVEL TIME (MIN.) = 1.81 Tc (MIN.) = 12.67  
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 124.00 = 1127.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 124.00 TO NODE 124.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 12.67  
RAINFALL INTENSITY (INCH/HR) = 2.35  
TOTAL STREAM AREA (ACRES) = 1.18  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.67

\*\*\*\*\*

FLOW PROCESS FROM NODE 125.00 TO NODE 126.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 777.00  
UPSTREAM ELEVATION (FEET) = 1707.90  
DOWNSTREAM ELEVATION (FEET) = 1704.10  
ELEVATION DIFFERENCE (FEET) = 3.80  
TC =  $0.303 * [(777.00 ** 3) / (3.80)] ** .2 = 12.586$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.361  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8818  
SOIL CLASSIFICATION IS "C"

SUBAREA RUNOFF(CFS) = 1.75  
TOTAL AREA(ACRES) = 0.84 TOTAL RUNOFF(CFS) = 1.75

\*\*\*\*\*  
FLOW PROCESS FROM NODE 126.00 TO NODE 124.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1699.10 DOWNSTREAM(FEET) = 1690.60  
FLOW LENGTH(FEET) = 25.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.1 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.28  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.75  
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 12.61  
LONGEST FLOWPATH FROM NODE 125.00 TO NODE 124.00 = 802.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 124.00 TO NODE 124.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 12.61  
RAINFALL INTENSITY(INCH/HR) = 2.36  
TOTAL STREAM AREA(ACRES) = 0.84  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.75

\*\*\*\*\*  
FLOW PROCESS FROM NODE 125.00 TO NODE 127.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 777.00  
UPSTREAM ELEVATION(FEET) = 1707.90  
DOWNSTREAM ELEVATION(FEET) = 1704.10  
ELEVATION DIFFERENCE(FEET) = 3.80  
TC = 0.303\*[( 777.00\*\*3)/( 3.80)]\*\*.2 = 12.586  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.361  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8818  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.75  
TOTAL AREA(ACRES) = 0.84 TOTAL RUNOFF(CFS) = 1.75

\*\*\*\*\*  
FLOW PROCESS FROM NODE 127.00 TO NODE 124.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1699.10 DOWNSTREAM(FEET) = 1690.60  
FLOW LENGTH(FEET) = 35.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.3 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.58  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1



PIPE-FLOW(CFS) = 1.75  
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 12.63  
LONGEST FLOWPATH FROM NODE 125.00 TO NODE 124.00 = 812.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 124.00 TO NODE 124.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
TIME OF CONCENTRATION(MIN.) = 12.63  
RAINFALL INTENSITY(INCH/HR) = 2.36  
TOTAL STREAM AREA(ACRES) = 0.84  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.75

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.67	12.67	2.353	1.18
2	1.75	12.61	2.359	0.84
3	1.75	12.63	2.357	0.84

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	6.16	12.61	2.359
2	6.16	12.63	2.357
3	6.16	12.67	2.353

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 6.16 Tc(MIN.) = 12.67  
TOTAL AREA(ACRES) = 2.9  
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 124.00 = 1127.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 124.00 TO NODE 127.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1690.60 DOWNSTREAM(FEET) = 1684.30  
FLOW LENGTH(FEET) = 622.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 8.5 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.16  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 6.16  
PIPE TRAVEL TIME(MIN.) = 1.68 Tc(MIN.) = 14.36  
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 127.00 = 1749.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 127.00 TO NODE 127.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.36
RAINFALL INTENSITY(INCH/HR) = 2.20
TOTAL STREAM AREA(ACRES) = 2.86
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.16

\*\*\*\*\*

FLOW PROCESS FROM NODE 110.00 TO NODE 128.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER
TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 768.00
UPSTREAM ELEVATION(FEET) = 1707.90
DOWNSTREAM ELEVATION(FEET) = 1700.00
ELEVATION DIFFERENCE(FEET) = 7.90
TC = 0.533\*[(768.00\*\*3)/(7.90)]\*\*.2 = 18.971
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.884
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6829
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 11.89
TOTAL AREA(ACRES) = 9.24 TOTAL RUNOFF(CFS) = 11.89

\*\*\*\*\*

FLOW PROCESS FROM NODE 128.00 TO NODE 127.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1694.00 DOWNSTREAM(FEET) = 1684.30
FLOW LENGTH(FEET) = 29.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 4.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 25.89
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 11.89
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 18.99
LONGEST FLOWPATH FROM NODE 110.00 TO NODE 127.00 = 797.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 127.00 TO NODE 127.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 18.99
RAINFALL INTENSITY(INCH/HR) = 1.88
TOTAL STREAM AREA(ACRES) = 9.24
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.89

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.16	14.36	2.196	2.86
2	11.89	18.99	1.883	9.24

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	15.15	14.36	2.196
2	17.17	18.99	1.883

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 17.17 Tc (MIN.) = 18.99  
 TOTAL AREA (ACRES) = 12.1  
 LONGEST FLOWPATH FROM NODE 120.00 TO NODE 127.00 = 1749.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 127.00 TO NODE 129.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1684.30 DOWNSTREAM(FEET) = 1682.10  
 FLOW LENGTH(FEET) = 216.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.5 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.98  
 GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 17.17  
 PIPE TRAVEL TIME(MIN.) = 0.45 Tc(MIN.) = 19.44  
 LONGEST FLOWPATH FROM NODE 120.00 TO NODE 129.00 = 1965.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 129.00 TO NODE 130.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1682.10 DOWNSTREAM(FEET) = 1656.00  
 FLOW LENGTH(FEET) = 132.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 6.7 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 23.90  
 GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 17.17  
 PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 19.53  
 LONGEST FLOWPATH FROM NODE 120.00 TO NODE 130.00 = 2097.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 130.00 TO NODE 130.00 IS CODE = 13  
 -----

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<  
 =====

-----+  
| Watershed 7 |  
| |  
| |  
-----+

\*\*\*\*\*  
FLOW PROCESS FROM NODE 140.00 TO NODE 141.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**.2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 596.00  
UPSTREAM ELEVATION(FEET) = 1714.00  
DOWNSTREAM ELEVATION(FEET) = 1707.70  
ELEVATION DIFFERENCE(FEET) = 6.30  
TC =  $0.303 * [(596.00^{**3}) / (6.30)]^{**.2} = 9.702$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.725  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8838  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 1.93  
TOTAL AREA(ACRES) = 0.80 TOTAL RUNOFF(CFS) = 1.93

\*\*\*\*\*  
FLOW PROCESS FROM NODE 141.00 TO NODE 142.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1700.70 DOWNSTREAM(FEET) = 1700.00  
FLOW LENGTH(FEET) = 37.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.4 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.69  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.93  
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 9.81  
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 142.00 = 633.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 142.00 TO NODE 142.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 9.81  
RAINFALL INTENSITY(INCH/HR) = 2.71  
TOTAL STREAM AREA(ACRES) = 0.80  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.93

\*\*\*\*\*  
FLOW PROCESS FROM NODE 140.00 TO NODE 143.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

```

TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 602.00
UPSTREAM ELEVATION(FEET) = 1714.00
DOWNSTREAM ELEVATION(FEET) = 1707.70
ELEVATION DIFFERENCE(FEET) = 6.30
TC = 0.303*[(602.00**3)/(6.30)]**.2 = 9.761
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.716
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8837
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 1.90
TOTAL AREA(ACRES) = 0.79 TOTAL RUNOFF(CFS) = 1.90

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*****
FLOW PROCESS FROM NODE 143.00 TO NODE 142.00 IS CODE = 41
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 1700.70 DOWNSTREAM(FEET) = 1700.00
FLOW LENGTH(FEET) = 28.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.24
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.90
PIPE TRAVEL TIME(MIN.) = 0.07 Tc(MIN.) = 9.84
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 142.00 = 630.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 142.00 TO NODE 142.00 IS CODE = 1
-----

```

```

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====

```

```

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.84
RAINFALL INTENSITY(INCH/HR) = 2.70
TOTAL STREAM AREA(ACRES) = 0.79
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.90

```

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.93	9.81	2.708	0.80
2	1.90	9.84	2.705	0.79

```

*****WARNING*****
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.
*****

```

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	3.82	9.81	2.708
2	3.82	9.84	2.705

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 3.82 Tc (MIN.) = 9.81  
TOTAL AREA (ACRES) = 1.6  
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 142.00 = 633.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 142.00 TO NODE 144.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1700.00 DOWNSTREAM (FEET) = 1691.30  
FLOW LENGTH (FEET) = 88.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 3.8 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 12.05  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 3.82  
PIPE TRAVEL TIME (MIN.) = 0.12 Tc (MIN.) = 9.93  
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 144.00 = 721.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 144.00 TO NODE 144.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 9.93  
RAINFALL INTENSITY (INCH/HR) = 2.69  
TOTAL STREAM AREA (ACRES) = 1.59  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.82

\*\*\*\*\*

FLOW PROCESS FROM NODE 145.00 TO NODE 146.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 1336.00  
UPSTREAM ELEVATION (FEET) = 1716.00  
DOWNSTREAM ELEVATION (FEET) = 1702.00  
ELEVATION DIFFERENCE (FEET) = 14.00  
TC =  $0.533 * [(1336.00 ** 3) / (14.00)] ** .2 = 23.586$   
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 1.671  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6626  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 35.34  
TOTAL AREA (ACRES) = 31.91 TOTAL RUNOFF (CFS) = 35.34

\*\*\*\*\*

FLOW PROCESS FROM NODE 146.00 TO NODE 144.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1697.00 DOWNSTREAM (FEET) = 1691.30  
FLOW LENGTH (FEET) = 100.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 14.2 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 18.33  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 35.34  
PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 23.68  
LONGEST FLOWPATH FROM NODE 145.00 TO NODE 144.00 = 1436.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 144.00 TO NODE 144.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION (MIN.) = 23.68  
RAINFALL INTENSITY (INCH/HR) = 1.67  
TOTAL STREAM AREA (ACRES) = 31.91  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 35.34

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	3.82	9.93	2.690	1.59
2	35.34	23.68	1.668	31.91

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	18.64	9.93	2.690
2	37.71	23.68	1.668

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
PEAK FLOW RATE (CFS) = 37.71 Tc (MIN.) = 23.68  
TOTAL AREA (ACRES) = 33.5  
LONGEST FLOWPATH FROM NODE 145.00 TO NODE 144.00 = 1436.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 144.00 TO NODE 147.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1691.30 DOWNSTREAM (FEET) = 1689.50  
FLOW LENGTH (FEET) = 88.00 MANNING'S N = 0.013  
ASSUME FULL-FLOWING PIPELINE  
PIPE-FLOW VELOCITY (FEET/SEC.) = 12.00  
PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 37.71  
PIPE TRAVEL TIME (MIN.) = 0.12 Tc (MIN.) = 23.80

LONGEST FLOWPATH FROM NODE 145.00 TO NODE 147.00 = 1524.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 147.00 TO NODE 147.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 23.80  
RAINFALL INTENSITY(INCH/HR) = 1.66  
TOTAL STREAM AREA(ACRES) = 33.50  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 37.71

\*\*\*\*\*

FLOW PROCESS FROM NODE 148.00 TO NODE 149.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 433.00  
UPSTREAM ELEVATION(FEET) = 1708.40  
DOWNSTREAM ELEVATION(FEET) = 1702.10  
ELEVATION DIFFERENCE(FEET) = 6.30  
TC =  $0.303 * [(433.00^{**3}) / (6.30)]^{**2}$  = 8.010  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.028  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8851  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.01  
TOTAL AREA(ACRES) = 0.75 TOTAL RUNOFF(CFS) = 2.01

\*\*\*\*\*

FLOW PROCESS FROM NODE 149.00 TO NODE 147.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1695.50 DOWNSTREAM(FEET) = 1689.50  
FLOW LENGTH(FEET) = 28.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.5 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.57  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.01  
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 8.04  
LONGEST FLOWPATH FROM NODE 148.00 TO NODE 147.00 = 461.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 147.00 TO NODE 147.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 8.04  
RAINFALL INTENSITY(INCH/HR) = 3.02  
TOTAL STREAM AREA(ACRES) = 0.75  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.01



\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	37.71	23.80	1.663	33.50
2	2.01	8.04	3.021	0.75

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	14.75	8.04	3.021
2	38.81	23.80	1.663

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 38.81 Tc (MIN.) = 23.80  
 TOTAL AREA (ACRES) = 34.2  
 LONGEST FLOWPATH FROM NODE 145.00 TO NODE 147.00 = 1524.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 147.00 TO NODE 150.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1689.50 DOWNSTREAM (FEET) = 1688.50  
 FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 12.35  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 38.81  
 PIPE TRAVEL TIME (MIN.) = 0.06 Tc (MIN.) = 23.86  
 LONGEST FLOWPATH FROM NODE 145.00 TO NODE 150.00 = 1569.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 150.00 TO NODE 150.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 23.86  
 RAINFALL INTENSITY (INCH/HR) = 1.66  
 TOTAL STREAM AREA (ACRES) = 34.25  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 38.81

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 142.00 TO NODE 151.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS COMMERCIAL

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 607.00  
UPSTREAM ELEVATION(FEET) = 1708.40  
DOWNSTREAM ELEVATION(FEET) = 1700.80  
ELEVATION DIFFERENCE(FEET) = 7.60  
TC = 0.303\*[(607.00\*\*3)/(7.60)]\*\*.2 = 9.448  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.765  
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8840  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 2.40  
TOTAL AREA(ACRES) = 0.98 TOTAL RUNOFF(CFS) = 2.40

\*\*\*\*\*  
FLOW PROCESS FROM NODE 151.00 TO NODE 150.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1694.80 DOWNSTREAM(FEET) = 1688.50  
FLOW LENGTH(FEET) = 55.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.2 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.44  
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 2.40  
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 9.53  
LONGEST FLOWPATH FROM NODE 142.00 TO NODE 150.00 = 662.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 150.00 TO NODE 150.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 9.53  
RAINFALL INTENSITY(INCH/HR) = 2.75  
TOTAL STREAM AREA(ACRES) = 0.98  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.40

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	38.81	23.86	1.661	34.25
2	2.40	9.53	2.752	0.98

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
------------------	-----------------	--------------	--------------------------

1	17.89	9.53	2.752
2	40.26	23.86	1.661

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 40.26 Tc (MIN.) = 23.86  
TOTAL AREA (ACRES) = 35.2  
LONGEST FLOWPATH FROM NODE 145.00 TO NODE 150.00 = 1569.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 150.00 TO NODE 152.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1688.50 DOWNSTREAM (FEET) = 1675.50  
FLOW LENGTH (FEET) = 433.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 14.7 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 14.88  
GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 40.26  
PIPE TRAVEL TIME (MIN.) = 0.48 Tc (MIN.) = 24.35  
LONGEST FLOWPATH FROM NODE 145.00 TO NODE 152.00 = 2002.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 152.00 TO NODE 152.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 24.35  
RAINFALL INTENSITY (INCH/HR) = 1.64  
TOTAL STREAM AREA (ACRES) = 35.23  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 40.26

\*\*\*\*\*

FLOW PROCESS FROM NODE 153.00 TO NODE 154.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**0.2}$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 785.00  
UPSTREAM ELEVATION (FEET) = 1704.80  
DOWNSTREAM ELEVATION (FEET) = 1700.00  
ELEVATION DIFFERENCE (FEET) = 4.80  
TC =  $0.533 * [(785.00^{**3}) / (4.80)]^{**0.2}$  = 21.236  
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 1.771  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6725  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 9.34  
TOTAL AREA (ACRES) = 7.84 TOTAL RUNOFF (CFS) = 9.34

\*\*\*\*\*

FLOW PROCESS FROM NODE 154.00 TO NODE 152.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1695.00 DOWNSTREAM(FEET) = 1675.50  
 FLOW LENGTH(FEET) = 28.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 3.6 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 31.23  
 GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 9.34  
 PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 21.25  
 LONGEST FLOWPATH FROM NODE 153.00 TO NODE 152.00 = 813.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 152.00 TO NODE 152.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION(MIN.) = 21.25  
 RAINFALL INTENSITY(INCH/HR) = 1.77  
 TOTAL STREAM AREA(ACRES) = 7.84  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.34

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	40.26	24.35	1.643	35.23
2	9.34	21.25	1.770	7.84

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	44.48	21.25	1.770
2	48.92	24.35	1.643

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 48.92 Tc(MIN.) = 24.35  
 TOTAL AREA(ACRES) = 43.1  
 LONGEST FLOWPATH FROM NODE 145.00 TO NODE 152.00 = 2002.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 152.00 TO NODE 155.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1675.50 DOWNSTREAM(FEET) = 1674.70  
 FLOW LENGTH(FEET) = 23.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 15.7 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 16.55  
 GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 48.92

PIPE TRAVEL TIME (MIN.) = 0.02 Tc (MIN.) = 24.37  
LONGEST FLOWPATH FROM NODE 145.00 TO NODE 155.00 = 2025.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 155.00 TO NODE 155.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 24.37  
RAINFALL INTENSITY (INCH/HR) = 1.64  
TOTAL STREAM AREA (ACRES) = 43.07  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 48.92

\*\*\*\*\*  
FLOW PROCESS FROM NODE 156.00 TO NODE 157.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH (FEET) = 524.00  
UPSTREAM ELEVATION (FEET) = 1705.50  
DOWNSTREAM ELEVATION (FEET) = 1697.00  
ELEVATION DIFFERENCE (FEET) = 8.50  
TC =  $0.533 * [(524.00^{**3}) / (8.50)]^{**2}$  = 14.863  
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.155  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7042  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF (CFS) = 4.96  
TOTAL AREA (ACRES) = 3.27 TOTAL RUNOFF (CFS) = 4.96

\*\*\*\*\*  
FLOW PROCESS FROM NODE 157.00 TO NODE 155.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1691.00 DOWNSTREAM (FEET) = 1674.70  
FLOW LENGTH (FEET) = 51.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 3.2 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 19.66  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 4.96  
PIPE TRAVEL TIME (MIN.) = 0.04 Tc (MIN.) = 14.91  
LONGEST FLOWPATH FROM NODE 156.00 TO NODE 155.00 = 575.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 155.00 TO NODE 155.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION (MIN.) = 14.91  
RAINFALL INTENSITY (INCH/HR) = 2.15  
TOTAL STREAM AREA (ACRES) = 3.27

PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.96

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	48.92	24.37	1.642	43.07
2	4.96	14.91	2.152	3.27

\*\*\*\*\*WARNING\*\*\*\*\*  
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
 \*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	34.89	14.91	2.152
2	52.71	24.37	1.642

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 52.71 Tc (MIN.) = 24.37  
 TOTAL AREA (ACRES) = 46.3  
 LONGEST FLOWPATH FROM NODE 145.00 TO NODE 155.00 = 2025.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 155.00 TO NODE 158.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1674.70 DOWNSTREAM (FEET) = 1652.00  
 FLOW LENGTH (FEET) = 67.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 9.0 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 38.38  
 GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 52.71  
 PIPE TRAVEL TIME (MIN.) = 0.03 Tc (MIN.) = 24.40  
 LONGEST FLOWPATH FROM NODE 145.00 TO NODE 158.00 = 2092.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 158.00 TO NODE 158.00 IS CODE = 13  
 -----

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<

```

+-----+
| Watershed 8 |
|             |
|             |
+-----+

```

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 156.00 TO NODE 180.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM  
 DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
 $TC = K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 737.00  
 UPSTREAM ELEVATION(FEET) = 1705.50  
 DOWNSTREAM ELEVATION(FEET) = 1697.00  
 ELEVATION DIFFERENCE(FEET) = 8.50  
 $TC = 0.533 * [(737.00^{**3}) / (8.50)]^{**2} = 18.239$   
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.925  
 UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6864  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF(CFS) = 9.05  
 TOTAL AREA(ACRES) = 6.85 TOTAL RUNOFF(CFS) = 9.05

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 180.00 TO NODE 181.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====  
 ELEVATION DATA: UPSTREAM(FEET) = 1692.00 DOWNSTREAM(FEET) = 1650.00  
 FLOW LENGTH(FEET) = 175.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 4.6 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 21.27  
 GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 9.05  
 PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 18.38  
 LONGEST FLOWPATH FROM NODE 156.00 TO NODE 181.00 = 912.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 181.00 TO NODE 181.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION(MIN.) = 18.38  
 RAINFALL INTENSITY(INCH/HR) = 1.92  
 TOTAL STREAM AREA(ACRES) = 6.85  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.05

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 182.00 TO NODE 183.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====  
 ASSUMED INITIAL SUBAREA UNIFORM  
 DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
 $TC = K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 1449.00  
 UPSTREAM ELEVATION(FEET) = 1714.00  
 DOWNSTREAM ELEVATION(FEET) = 1704.30  
 ELEVATION DIFFERENCE(FEET) = 9.70  
 $TC = 0.533 * [(1449.00^{**3}) / (9.70)]^{**2} = 26.649$   
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.563  
 UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6506  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF(CFS) = 26.36  
 TOTAL AREA(ACRES) = 25.92 TOTAL RUNOFF(CFS) = 26.36

\*\*\*\*\*

FLOW PROCESS FROM NODE 183.00 TO NODE 181.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1690.00 DOWNSTREAM(FEET) = 1650.00
FLOW LENGTH(FEET) = 316.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 8.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 22.13
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 26.36
PIPE TRAVEL TIME(MIN.) = 0.24 Tc(MIN.) = 26.89
LONGEST FLOWPATH FROM NODE 182.00 TO NODE 181.00 = 1765.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 181.00 TO NODE 181.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 26.89
RAINFALL INTENSITY(INCH/HR) = 1.56
TOTAL STREAM AREA(ACRES) = 25.92
PEAK FLOW RATE(CFS) AT CONFLUENCE = 26.36

\*\* CONFLUENCE DATA \*\*

Table with 5 columns: STREAM NUMBER, RUNOFF (CFS), Tc (MIN.), INTENSITY (INCH/HOUR), AREA (ACRE). Rows for stream 1 and 2.

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

Table with 4 columns: STREAM NUMBER, RUNOFF (CFS), Tc (MIN.), INTENSITY (INCH/HOUR). Rows for stream 1 and 2.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 33.70 Tc(MIN.) = 26.89
TOTAL AREA(ACRES) = 32.8
LONGEST FLOWPATH FROM NODE 182.00 TO NODE 181.00 = 1765.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 181.00 TO NODE 184.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

\*\*\*\*\*



ELEVATION DATA: UPSTREAM(FEET) = 1650.00 DOWNSTREAM(FEET) = 1648.00  
FLOW LENGTH(FEET) = 8.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 7.7 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 30.26  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 33.70  
PIPE TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 26.89  
LONGEST FLOWPATH FROM NODE 182.00 TO NODE 184.00 = 1773.00 FEET.

-----  
| Watershed 14 |  
| |  

\*\*\*\*\*  
FLOW PROCESS FROM NODE 200.00 TO NODE 201.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
TC =  $K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$   
INITIAL SUBAREA FLOW-LENGTH(FEET) = 825.00  
UPSTREAM ELEVATION(FEET) = 1733.90  
DOWNSTREAM ELEVATION(FEET) = 1728.00  
ELEVATION DIFFERENCE(FEET) = 5.90  
TC =  $0.533 * [(825.00^{**3}) / (5.90)]^{**2} = 20.994$   
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.782  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6736  
SOIL CLASSIFICATION IS "C"  
SUBAREA RUNOFF(CFS) = 9.34  
TOTAL AREA(ACRES) = 7.78 TOTAL RUNOFF(CFS) = 9.34

\*\*\*\*\*  
FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1722.00 DOWNSTREAM(FEET) = 1717.50  
FLOW LENGTH(FEET) = 562.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 11.4 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.32  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 9.34  
PIPE TRAVEL TIME(MIN.) = 1.48 Tc(MIN.) = 22.48  
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 202.00 = 1387.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 22.48  
RAINFALL INTENSITY(INCH/HR) = 1.72  
TOTAL STREAM AREA(ACRES) = 7.78  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.34

\*\*\*\*\*

FLOW PROCESS FROM NODE 203.00 TO NODE 204.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM

DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER

TC = K\*[(LENGTH\*\*3)/(ELEVATION CHANGE)]\*\*.2

INITIAL SUBAREA FLOW-LENGTH(FEET) = 791.00

UPSTREAM ELEVATION(FEET) = 1727.70

DOWNSTREAM ELEVATION(FEET) = 1722.10

ELEVATION DIFFERENCE(FEET) = 5.60

TC = 0.533\*[(791.00\*\*3)/(5.60)]\*\*.2 = 20.686

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.797

UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6750

SOIL CLASSIFICATION IS "C"

SUBAREA RUNOFF(CFS) = 13.40

TOTAL AREA(ACRES) = 11.05 TOTAL RUNOFF(CFS) = 13.40

\*\*\*\*\*

FLOW PROCESS FROM NODE 204.00 TO NODE 202.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1717.70 DOWNSTREAM(FEET) = 1717.50

FLOW LENGTH(FEET) = 22.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 24.0 INCH PIPE IS 13.7 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 7.23

GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 13.40

PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 20.74

LONGEST FLOWPATH FROM NODE 203.00 TO NODE 202.00 = 813.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 20.74

RAINFALL INTENSITY(INCH/HR) = 1.79

TOTAL STREAM AREA(ACRES) = 11.05

PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.40

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	9.34	22.48	1.716	7.78
2	13.40	20.74	1.794	11.05

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	22.01	20.74	1.794
2	22.16	22.48	1.716

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 22.01 Tc (MIN.) = 20.74  
 TOTAL AREA (ACRES) = 18.8  
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 202.00 = 1387.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 202.00 TO NODE 205.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1717.50 DOWNSTREAM (FEET) = 1716.80  
 FLOW LENGTH (FEET) = 77.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.01  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 22.01  
 PIPE TRAVEL TIME (MIN.) = 0.18 Tc (MIN.) = 20.92  
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 205.00 = 1464.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 205.00 TO NODE 205.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 20.92  
 RAINFALL INTENSITY (INCH/HR) = 1.79  
 TOTAL STREAM AREA (ACRES) = 18.83  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 22.01

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 206.00 TO NODE 207.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM  
 DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER  
 $TC = K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$   
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 890.00  
 UPSTREAM ELEVATION (FEET) = 1727.00  
 DOWNSTREAM ELEVATION (FEET) = 1722.00  
 ELEVATION DIFFERENCE (FEET) = 5.00  
 $TC = 0.533 * [(890.00 ** 3) / (5.00)] ** .2 = 22.711$   
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 1.707  
 UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .6662  
 SOIL CLASSIFICATION IS "C"  
 SUBAREA RUNOFF (CFS) = 12.56  
 TOTAL AREA (ACRES) = 11.05 TOTAL RUNOFF (CFS) = 12.56

\*\*\*\*\*

FLOW PROCESS FROM NODE 207.00 TO NODE 205.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1717.00 DOWNSTREAM(FEET) = 1716.80
FLOW LENGTH(FEET) = 22.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.12
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.56
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 22.76
LONGEST FLOWPATH FROM NODE 206.00 TO NODE 205.00 = 912.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 205.00 TO NODE 205.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 22.76
RAINFALL INTENSITY(INCH/HR) = 1.70
TOTAL STREAM AREA(ACRES) = 11.05
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.56

\*\* CONFLUENCE DATA \*\*

Table with 5 columns: STREAM NUMBER, RUNOFF (CFS), Tc (MIN.), INTENSITY (INCH/HOUR), AREA (ACRE). Rows 1 and 2.

\*\*\*\*\*WARNING\*\*\*\*\*

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

Table with 4 columns: STREAM NUMBER, RUNOFF (CFS), Tc (MIN.), INTENSITY (INCH/HOUR). Rows 1 and 2.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 33.56 Tc(MIN.) = 20.92
TOTAL AREA(ACRES) = 29.9
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 205.00 = 1464.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 205.00 TO NODE 208.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

```

=====
ELEVATION DATA: UPSTREAM(FEET) = 1716.80  DOWNSTREAM(FEET) = 1716.20
FLOW LENGTH(FEET) = 72.00  MANNING'S N = 0.013
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.68
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 33.56
PIPE TRAVEL TIME(MIN.) = 0.11  Tc(MIN.) = 21.03
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 208.00 = 1536.00 FEET.

*****
FLOW PROCESS FROM NODE 208.00 TO NODE 209.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1716.20  DOWNSTREAM(FEET) = 1716.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 12.00  CHANNEL SLOPE = 0.0167
CHANNEL BASE(FEET) = 4.00  "Z" FACTOR = 1.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) = 2.00
CHANNEL FLOW THRU SUBAREA(CFS) = 33.56
FLOW VELOCITY(FEET/SEC.) = 9.06  FLOW DEPTH(FEET) = 0.78
TRAVEL TIME(MIN.) = 0.02  Tc(MIN.) = 21.05
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 209.00 = 1548.00 FEET.

*****
FLOW PROCESS FROM NODE 209.00 TO NODE 209.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 21.05
RAINFALL INTENSITY(INCH/HR) = 1.78
TOTAL STREAM AREA(ACRES) = 29.88
PEAK FLOW RATE(CFS) AT CONFLUENCE = 33.56

*****
FLOW PROCESS FROM NODE 210.00 TO NODE 209.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS: UNDEVELOPED WITH POOR COVER
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 430.00
UPSTREAM ELEVATION(FEET) = 1778.00
DOWNSTREAM ELEVATION(FEET) = 1747.00
ELEVATION DIFFERENCE(FEET) = 31.00
TC = 0.533*[(430.00**3)/(31.00)]**.2 = 10.191
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.652
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7342
SOIL CLASSIFICATION IS "C"
SUBAREA RUNOFF(CFS) = 3.02
TOTAL AREA(ACRES) = 1.55  TOTAL RUNOFF(CFS) = 3.02

*****
FLOW PROCESS FROM NODE 210.00 TO NODE 209.00 IS CODE = 51
-----

```

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1747.00 DOWNSTREAM(FEET) = 1716.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 1622.00 CHANNEL SLOPE = 0.0191  
CHANNEL BASE(FEET) = 1.50 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.294  
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7137  
SOIL CLASSIFICATION IS "C"  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 17.78  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 8.80  
AVERAGE FLOW DEPTH(FEET) = 0.86 TRAVEL TIME(MIN.) = 3.07  
Tc(MIN.) = 13.26  
SUBAREA AREA(ACRES) = 17.93 SUBAREA RUNOFF(CFS) = 29.36  
TOTAL AREA(ACRES) = 19.5 PEAK FLOW RATE(CFS) = 32.37

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
DEPTH(FEET) = 1.18 FLOW VELOCITY(FEET/SEC.) = 10.27  
LONGEST FLOWPATH FROM NODE 210.00 TO NODE 209.00 = 2052.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 209.00 TO NODE 209.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 13.26  
RAINFALL INTENSITY(INCH/HR) = 2.29  
TOTAL STREAM AREA(ACRES) = 19.48  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 32.37

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	33.56	21.05	1.779	29.88
2	32.37	13.26	2.294	19.48

\*\*\*\*\*WARNING\*\*\*\*\*  
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED  
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA  
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.  
\*\*\*\*\*

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	53.52	13.26	2.294
2	58.67	21.05	1.779

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
PEAK FLOW RATE(CFS) = 58.67 Tc(MIN.) = 21.05  
TOTAL AREA(ACRES) = 49.4  
LONGEST FLOWPATH FROM NODE 210.00 TO NODE 209.00 = 2052.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 209.00 TO NODE 211.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1716.00 DOWNSTREAM(FEET) = 1712.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 241.00 CHANNEL SLOPE = 0.0166  
CHANNEL BASE(FEET) = 4.00 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 58.67  
FLOW VELOCITY(FEET/SEC.) = 10.78 FLOW DEPTH(FEET) = 1.07  
TRAVEL TIME(MIN.) = 0.37 Tc(MIN.) = 21.43  
LONGEST FLOWPATH FROM NODE 210.00 TO NODE 211.00 = 2293.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 211.00 TO NODE 212.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1712.00 DOWNSTREAM(FEET) = 1711.50  
CHANNEL LENGTH THRU SUBAREA(FEET) = 100.00 CHANNEL SLOPE = 0.0050  
CHANNEL BASE(FEET) = 4.00 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 58.67  
FLOW VELOCITY(FEET/SEC.) = 7.03 FLOW DEPTH(FEET) = 1.51  
TRAVEL TIME(MIN.) = 0.24 Tc(MIN.) = 21.66  
LONGEST FLOWPATH FROM NODE 210.00 TO NODE 212.00 = 2393.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 49.4 TC(MIN.) = 21.66  
PEAK FLOW RATE(CFS) = 58.67

=====

END OF RATIONAL METHOD ANALYSIS

***APPENDIX D***

Peak Discharge Water Surface Elevation Calculations (WSPG)



20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1000.000	1645.000	.515	1645.515	19.42	26.64	11.02	1656.53	.00	1.49	2.02	2.500	.000	.00	1	.0
6.000	.2617					.2593	1.56	.51	7.82	.51	.013	.00	.00	PIPE	
1006.000	1646.570	.515	1647.085	19.42	26.63	11.01	1658.10	.18	1.49	2.02	2.500	.000	.00	1	.0
31.302	.2605					.2574	8.06	.69	7.81	.51	.013	.00	.00	PIPE	
1037.302	1654.725	.516	1655.242	19.42	26.51	10.91	1666.15	.18	1.49	2.02	2.500	.000	.00	1	.0
46.475	.2605					.2396	11.13	.69	7.76	.51	.013	.00	.00	PIPE	
1083.778	1666.834	.534	1667.368	19.42	25.27	9.92	1677.29	.16	1.49	2.05	2.500	.000	.00	1	.0
17.297	.2605					.2094	3.62	.70	7.27	.51	.013	.00	.00	PIPE	
1101.075	1671.340	.552	1671.892	19.42	24.10	9.02	1680.91	.15	1.49	2.07	2.500	.000	.00	1	.0
10.338	.2605					.1831	1.89	.70	6.81	.51	.013	.00	.00	PIPE	
1111.412	1674.033	.571	1674.604	19.42	22.98	8.20	1682.80	.14	1.49	2.10	2.500	.000	.00	1	.0
7.225	.2605					.1600	1.16	.71	6.38	.51	.013	.00	.00	PIPE	
1118.637	1675.916	.590	1676.506	19.42	21.91	7.45	1683.96	.13	1.49	2.12	2.500	.000	.00	1	.0
5.443	.2605					.1399	.76	.72	5.98	.51	.013	.00	.00	PIPE	
1124.081	1677.334	.611	1677.945	19.42	20.89	6.78	1684.72	.12	1.49	2.15	2.500	.000	.00	1	.0
4.307	.2605					.1224	.53	.73	5.60	.51	.013	.00	.00	PIPE	
1128.388	1678.456	.632	1679.088	19.42	19.92	6.16	1685.25	.11	1.49	2.17	2.500	.000	.00	1	.0
3.511	.2605					.1070	.38	.74	5.24	.51	.013	.00	.00	PIPE	

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1131.899	1679.371	.653	1680.024	19.42	18.99	5.60	1685.62	.10	1.49	2.20	2.500	.000	.00	1	.0
	2.912	.2605				.0936	.27	.75	4.90	.51	.013	.00	.00	PIPE	
1134.811	1680.130	.676	1680.806	19.42	18.11	5.09	1685.90	.09	1.49	2.22	2.500	.000	.00	1	.0
	2.462	.2605				.0819	.20	.77	4.59	.51	.013	.00	.00	PIPE	
1137.272	1680.771	.699	1681.470	19.42	17.26	4.63	1686.10	.08	1.49	2.24	2.500	.000	.00	1	.0
	2.095	.2605				.0717	.15	.78	4.30	.51	.013	.00	.00	PIPE	
1139.368	1681.317	.724	1682.041	19.42	16.46	4.21	1686.25	.08	1.49	2.27	2.500	.000	.00	1	.0
	1.807	.2605				.0628	.11	.80	4.02	.51	.013	.00	.00	PIPE	
1141.175	1681.788	.749	1682.537	19.42	15.69	3.82	1686.36	.07	1.49	2.29	2.500	.000	.00	1	.0
	1.564	.2605				.0549	.09	.82	3.76	.51	.013	.00	.00	PIPE	
1142.739	1682.195	.775	1682.970	19.42	14.96	3.48	1686.45	.06	1.49	2.31	2.500	.000	.00	1	.0
	1.356	.2605				.0481	.07	.84	3.52	.51	.013	.00	.00	PIPE	
1144.095	1682.548	.803	1683.352	19.42	14.27	3.16	1686.51	.06	1.49	2.33	2.500	.000	.00	1	.0
	1.188	.2605				.0421	.05	.86	3.29	.51	.013	.00	.00	PIPE	
1145.283	1682.858	.831	1683.689	19.42	13.60	2.87	1686.56	.05	1.49	2.36	2.500	.000	.00	1	.0
	1.038	.2605				.0369	.04	.89	3.08	.51	.013	.00	.00	PIPE	
1146.321	1683.128	.860	1683.988	19.42	12.97	2.61	1686.60	.05	1.49	2.38	2.500	.000	.00	1	.0
	.905	.2605				.0323	.03	.91	2.88	.51	.013	.00	.00	PIPE	



20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1151.495	1684.476	1.232	1685.708	19.42	8.05	1.01	1686.72	.02	1.49	2.50	2.500	.000	.00	1	.0
	.177	.2605				.0088	.00	1.25	1.44	.51	.013	.00	.00	PIPE	
1151.672	1684.522	1.279	1685.801	19.42	7.68	.92	1686.72	.02	1.49	2.50	2.500	.000	.00	1	.0
	.131	.2605				.0078	.00	1.30	1.35	.51	.013	.00	.00	PIPE	
1151.804	1684.557	1.329	1685.886	19.42	7.32	.83	1686.72	.02	1.49	2.49	2.500	.000	.00	1	.0
	.093	.2605				.0069	.00	1.35	1.25	.51	.013	.00	.00	PIPE	
1151.897	1684.581	1.381	1685.962	19.42	6.98	.76	1686.72	.02	1.49	2.49	2.500	.000	.00	1	.0
	.054	.2605				.0061	.00	1.40	1.16	.51	.013	.00	.00	PIPE	
1151.951	1684.595	1.436	1686.031	19.42	6.66	.69	1686.72	.01	1.49	2.47	2.500	.000	.00	1	.0
	.019	.2605				.0054	.00	1.45	1.08	.51	.013	.00	.00	PIPE	
1151.970	1684.600	1.495	1686.095	19.42	6.34	.62	1686.72	.00	1.49	2.45	2.500	.000	.00	1	.0
JUNCT STR	.0214					.0034	.02	1.51	1.00		.013	.00	.00	PIPE	
1156.650	1684.700	2.352	1687.052	9.57	3.05	.14	1687.20	.00	1.11	.00	2.000	.000	.00	1	.0
	43.616	.0101				.0018	.08	.00	.00	.91	.013	.00	.00	PIPE	
1200.266	1685.139	2.000	1687.139	9.57	3.05	.14	1687.28	2.00	1.11	.00	2.000	.000	.00	1	.0
	3.044	.0101				.0017	.01	2.00	.00	.91	.013	.00	.00	PIPE	
1203.310	1685.170	1.973	1687.143	9.57	3.05	.14	1687.29	.00	1.11	.46	2.000	.000	.00	1	.0
	17.942	.0100				.0016	.03	1.97	.21	.91	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1221.252	1685.350	1.807	1687.157	9.57	3.20	.16	1687.32	.00	1.11	1.18	2.000	.000	.00	1	.0
10.699	.0100					.0016	.02	1.81	.35	.91	.013	.00	.00	PIPE	
1231.951	1685.458	1.701	1687.159	9.57	3.36	.18	1687.33	.00	1.11	1.43	2.000	.000	.00	1	.0
8.510	.0100					.0018	.01	1.70	.42	.91	.013	.00	.00	PIPE	
1240.461	1685.543	1.613	1687.156	9.57	3.52	.19	1687.35	.00	1.11	1.58	2.000	.000	.00	1	.0
7.123	.0100					.0019	.01	1.61	.47	.91	.013	.00	.00	PIPE	
1247.584	1685.615	1.536	1687.151	9.57	3.70	.21	1687.36	.00	1.11	1.69	2.000	.000	.00	1	.0
6.184	.0100					.0022	.01	1.54	.53	.91	.013	.00	.00	PIPE	
1253.768	1685.677	1.466	1687.143	9.57	3.88	.23	1687.38	.00	1.11	1.77	2.000	.000	.00	1	.0
5.324	.0100					.0024	.01	1.47	.58	.91	.013	.00	.00	PIPE	
1259.092	1685.730	1.402	1687.132	9.57	4.07	.26	1687.39	.00	1.11	1.83	2.000	.000	.00	1	.0
4.536	.0100					.0027	.01	1.40	.63	.91	.013	.00	.00	PIPE	
1263.628	1685.776	1.343	1687.119	9.57	4.26	.28	1687.40	.00	1.11	1.88	2.000	.000	.00	1	.0
.457	.0100					.0029	.00	1.34	.69	.91	.013	.00	.00	PIPE	
1264.085	1685.780	1.343	1687.124	9.57	4.27	.28	1687.41	.00	1.11	1.88	2.000	.000	.00	1	.0
HYDRAULIC JUMP															
1264.085	1685.780	.907	1686.687	9.57	6.91	.74	1687.43	.00	1.11	1.99	2.000	.000	.00	1	.0
216.366	.0100					.0100	2.17	.91	1.46	.91	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1480.451	1687.953	.907	1688.860	9.57	6.91	.74	1689.60	.00	1.11	1.99	2.000	.000	.00	1	.0
60.136	.0100					.0099	.59	.91	1.46	.91	.013	.00	.00	PIPE	
1540.587	1688.557	.915	1689.473	9.57	6.82	.72	1690.20	.00	1.11	1.99	2.000	.000	.00	1	.0
33.136	.0100					.0091	.30	.92	1.43	.91	.013	.00	.00	PIPE	
1573.722	1688.890	.950	1689.840	9.57	6.51	.66	1690.50	.00	1.11	2.00	2.000	.000	.00	1	.0
12.234	.0100					.0080	.10	.95	1.34	.91	.013	.00	.00	PIPE	
1585.956	1689.013	.985	1689.998	9.57	6.20	.60	1690.60	.00	1.11	2.00	2.000	.000	.00	1	.0
5.469	.0100					.0071	.04	.99	1.24	.91	.013	.00	.00	PIPE	
1591.424	1689.068	1.023	1690.091	9.57	5.91	.54	1690.63	.00	1.11	2.00	2.000	.000	.00	1	.0
2.455	.0100					.0062	.02	1.02	1.16	.91	.013	.00	.00	PIPE	
1593.879	1689.093	1.063	1690.156	9.57	5.64	.49	1690.65	.00	1.11	2.00	2.000	.000	.00	1	.0
.721	.0100					.0055	.00	1.06	1.08	.91	.013	.00	.00	PIPE	
1594.600	1689.100	1.106	1690.206	9.57	5.37	.45	1690.65	.00	1.11	1.99	2.000	.000	.00	1	.0
JUNCT STR	.0214					.0031	.01	1.11	1.00		.013	.00	.00	PIPE	
1599.270	1689.200	1.465	1690.665	6.29	2.55	.10	1690.77	.00	.89	1.77	2.000	.000	.00	1	.0
6.643	.0092					.0010	.01	1.47	.38	.74	.013	.00	.00	PIPE	
1605.913	1689.261	1.401	1690.662	6.29	2.67	.11	1690.77	.00	.89	1.83	2.000	.000	.00	1	.0
5.956	.0092					.0012	.01	1.40	.42	.74	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1611.869	1689.316	1.342	1690.658	6.29	2.80	.12	1690.78	.00	.89	1.88	2.000	.000	.00	1	.0
5.418	.0092					.0013	.01	1.34	.45	.74	.013	.00	.00	PIPE	
1617.286	1689.366	1.287	1690.653	6.29	2.94	.13	1690.79	.00	.89	1.92	2.000	.000	.00	1	.0
4.859	.0092					.0015	.01	1.29	.49	.74	.013	.00	.00	PIPE	
1622.145	1689.411	1.236	1690.647	6.29	3.09	.15	1690.79	.00	.89	1.94	2.000	.000	.00	1	.0
4.537	.0092					.0017	.01	1.24	.53	.74	.013	.00	.00	PIPE	
1626.682	1689.452	1.187	1690.639	6.29	3.24	.16	1690.80	.00	.89	1.96	2.000	.000	.00	1	.0
4.060	.0092					.0019	.01	1.19	.57	.74	.013	.00	.00	PIPE	
1630.743	1689.490	1.141	1690.631	6.29	3.39	.18	1690.81	.00	.89	1.98	2.000	.000	.00	1	.0
3.549	.0092					.0021	.01	1.14	.62	.74	.013	.00	.00	PIPE	
1634.292	1689.522	1.098	1690.620	6.29	3.56	.20	1690.82	.00	.89	1.99	2.000	.000	.00	1	.0
3.140	.0092					.0024	.01	1.10	.67	.74	.013	.00	.00	PIPE	
1637.432	1689.551	1.057	1690.608	6.29	3.73	.22	1690.82	.00	.89	2.00	2.000	.000	.00	1	.0
1.050	.0092					.0027	.00	1.06	.72	.74	.013	.00	.00	PIPE	
1638.482	1689.561	1.017	1690.578	6.29	3.92	.24	1690.82	.00	.89	2.00	2.000	.000	.00	1	.0
HYDRAULIC JUMP															
1638.482	1689.561	.741	1690.302	6.29	5.94	.55	1690.85	.00	.89	1.93	2.000	.000	.00	1	.0
19.594	.0092					.0085	.17	.74	1.41	.74	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1658.075	1689.741	.768	1690.509	6.29	5.66	.50	1691.01	.00	.89	1.95	2.000	.000	.00	1 .0
10.226	.0092					.0074	.08	.77	1.32	.74	.013	.00	.00	PIPE
1668.301	1689.835	.795	1690.630	6.29	5.40	.45	1691.08	.00	.89	1.96	2.000	.000	.00	1 .0
4.501	.0092					.0065	.03	.80	1.23	.74	.013	.00	.00	PIPE
1672.803	1689.877	.824	1690.701	6.29	5.15	.41	1691.11	.00	.89	1.97	2.000	.000	.00	1 .0
2.119	.0092					.0057	.01	.82	1.15	.74	.013	.00	.00	PIPE
1674.921	1689.896	.854	1690.751	6.29	4.91	.37	1691.12	.00	.89	1.98	2.000	.000	.00	1 .0
.379	.0092					.0050	.00	.85	1.07	.74	.013	.00	.00	PIPE
1675.300	1689.900	.888	1690.787	6.29	4.67	.34	1691.13	.00	.89	1.99	2.000	.000	.00	1 .0
JUNCT STR	.0214					.0029	.01	.89	1.00		.013	.00	.00	PIPE
1679.970	1690.000	1.169	1691.169	3.38	2.29	.08	1691.25	.00	.70	1.24	1.500	.000	.00	1 .0
5.223	.0100					.0012	.01	1.17	.37	.59	.013	.00	.00	PIPE
1685.193	1690.052	1.115	1691.167	3.38	2.40	.09	1691.26	.00	.70	1.31	1.500	.000	.00	1 .0
4.631	.0100					.0013	.01	1.12	.41	.59	.013	.00	.00	PIPE
1689.824	1690.099	1.066	1691.165	3.38	2.52	.10	1691.26	.00	.70	1.36	1.500	.000	.00	1 .0
4.260	.0100					.0015	.01	1.07	.45	.59	.013	.00	.00	PIPE
1694.083	1690.141	1.020	1691.161	3.38	2.64	.11	1691.27	.00	.70	1.40	1.500	.000	.00	1 .0
3.754	.0100					.0017	.01	1.02	.49	.59	.013	.00	.00	PIPE



20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1697.838	1690.179	.978	1691.157	3.38	2.77	.12	1691.28	.00	.70	1.43	1.500	.000	.00	1	.0
	3.349	.0100				.0019	.01	.98	.53	.59	.013	.00	.00	PIPE	
1701.187	1690.212	.939	1691.151	3.38	2.90	.13	1691.28	.00	.70	1.45	1.500	.000	.00	1	.0
	3.047	.0100				.0021	.01	.94	.57	.59	.013	.00	.00	PIPE	
1704.234	1690.243	.902	1691.145	3.38	3.04	.14	1691.29	.00	.70	1.47	1.500	.000	.00	1	.0
	2.722	.0100				.0024	.01	.90	.62	.59	.013	.00	.00	PIPE	
1706.955	1690.270	.867	1691.137	3.38	3.19	.16	1691.30	.00	.70	1.48	1.500	.000	.00	1	.0
	2.367	.0100				.0027	.01	.87	.67	.59	.013	.00	.00	PIPE	
1709.323	1690.293	.834	1691.128	3.38	3.35	.17	1691.30	.00	.70	1.49	1.500	.000	.00	1	.0
	.088	.0100				.0031	.00	.83	.72	.59	.013	.00	.00	PIPE	
1709.411	1690.294	.802	1691.096	3.38	3.51	.19	1691.29	.00	.70	1.50	1.500	.000	.00	1	.0
HYDRAULIC JUMP															
1709.411	1690.294	.585	1690.879	3.38	5.30	.44	1691.31	.00	.70	1.46	1.500	.000	.00	1	.0
	127.266	.0100				.0100	1.27	.59	1.41	.59	.013	.00	.00	PIPE	
1836.676	1691.567	.585	1692.152	3.38	5.30	.44	1692.59	.00	.70	1.46	1.500	.000	.00	1	.0
	31.354	.0100				.0100	.31	.59	1.41	.59	.013	.00	.00	PIPE	
1868.030	1691.880	.585	1692.465	3.38	5.30	.44	1692.90	.00	.70	1.46	1.500	.000	.00	1	.0
	259.612	.0100				.0100	2.60	.59	1.41	.59	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
2127.642	1694.476	.585	1695.061	3.38	5.30	.44	1695.50	.00	.70	1.46	1.500	.000	.00	1	.0
31.408	.0100					.0097	.31	.59	1.41	.59	.013	.00	.00	PIPE	
2159.050	1694.790	.594	1695.384	3.38	5.19	.42	1695.80	.00	.70	1.47	1.500	.000	.00	1	.0
57.413	.0094					.0094	.54	.59	1.37	.59	.013	.00	.00	PIPE	
2216.463	1695.332	.594	1695.926	3.38	5.19	.42	1696.34	.00	.70	1.47	1.500	.000	.00	1	.0
30.544	.0094					.0091	.28	.59	1.37	.59	.013	.00	.00	PIPE	
2247.007	1695.620	.605	1696.225	3.38	5.06	.40	1696.62	.00	.70	1.47	1.500	.000	.00	1	.0
12.233	.0094					.0083	.10	.61	1.32	.59	.013	.00	.00	PIPE	
2259.240	1695.735	.627	1696.362	3.38	4.82	.36	1696.72	.00	.70	1.48	1.500	.000	.00	1	.0
4.550	.0094					.0073	.03	.63	1.24	.59	.013	.00	.00	PIPE	
2263.790	1695.778	.650	1696.428	3.38	4.60	.33	1696.76	.00	.70	1.49	1.500	.000	.00	1	.0
1.924	.0094					.0064	.01	.65	1.15	.59	.013	.00	.00	PIPE	
2265.713	1695.797	.674	1696.471	3.38	4.39	.30	1696.77	.00	.70	1.49	1.500	.000	.00	1	.0
.377	.0094					.0056	.00	.67	1.08	.59	.013	.00	.00	PIPE	
2266.090	1695.800	.701	1696.501	3.38	4.17	.27	1696.77	.00	.70	1.50	1.500	.000	.00	1	.0
JUNCT STR	.0250					.0029	.01	.70	1.00		.013	.00	.00	PIPE	
2268.090	1695.850	.950	1696.800	1.69	1.43	.03	1696.83	.00	.49	1.45	1.500	.000	.00	1	.0
6.643	.0057					.0005	.00	.95	.28	.47	.013	.00	.00	PIPE	



WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1000.000	1656.000	.707	1656.707	17.16	17.27	4.63	1661.34	.00	1.49	1.91	2.000	.000	.00	1	.0
39.130	.0800					.0803	3.14	.71	4.22	.71	.013	.00	.00	PIPE	
1039.130	1659.130	.706	1659.836	17.16	17.30	4.65	1664.49	.30	1.49	1.91	2.000	.000	.00	1	.0
14.117	.0805					.0805	1.14	1.00	4.23	.71	.013	.00	.00	PIPE	
1053.247	1660.267	.706	1660.973	17.16	17.30	4.65	1665.62	.30	1.49	1.91	2.000	.000	.00	1	.0
80.133	.0805					.0803	6.43	1.00	4.23	.71	.013	.00	.00	PIPE	
1133.380	1666.720	.707	1667.427	17.16	17.26	4.63	1672.05	.00	1.49	1.91	2.000	.000	.00	1	.0
29.310	.0800					.0800	2.34	.71	4.22	.71	.013	.00	.00	PIPE	
1162.690	1669.064	.707	1669.772	17.16	17.26	4.63	1674.40	.00	1.49	1.91	2.000	.000	.00	1	.0
79.495	.0800					.0776	6.17	.71	4.22	.71	.013	.00	.00	PIPE	
1242.185	1675.423	.719	1676.142	17.16	16.88	4.43	1680.57	.00	1.49	1.92	2.000	.000	.00	1	.0
40.060	.0800					.0706	2.83	.72	4.09	.71	.013	.00	.00	PIPE	
1282.245	1678.627	.744	1679.371	17.16	16.10	4.02	1683.39	.00	1.49	1.93	2.000	.000	.00	1	.0
18.696	.0800					.0619	1.16	.74	3.82	.71	.013	.00	.00	PIPE	
1300.941	1680.123	.771	1680.894	17.16	15.35	3.66	1684.55	.00	1.49	1.95	2.000	.000	.00	1	.0
11.848	.0800					.0543	.64	.77	3.57	.71	.013	.00	.00	PIPE	
1312.789	1681.070	.799	1681.869	17.16	14.63	3.33	1685.19	.00	1.49	1.96	2.000	.000	.00	1	.0
8.449	.0800					.0476	.40	.80	3.33	.71	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1321.239	1681.746	.828	1682.574	17.16	13.95	3.02	1685.60	.00	1.49	1.97	2.000	.000	.00	1 .0
6.391	.0800					.0418	.27	.83	3.11	.71	.013	.00	.00	PIPE
1327.630	1682.257	.859	1683.116	17.16	13.30	2.75	1685.86	.00	1.49	1.98	2.000	.000	.00	1 .0
5.062	.0800					.0368	.19	.86	2.90	.71	.013	.00	.00	PIPE
1332.692	1682.662	.890	1683.552	17.16	12.68	2.50	1686.05	.00	1.49	1.99	2.000	.000	.00	1 .0
4.070	.0800					.0323	.13	.89	2.71	.71	.013	.00	.00	PIPE
1336.762	1682.988	.923	1683.911	17.16	12.09	2.27	1686.18	.00	1.49	1.99	2.000	.000	.00	1 .0
3.324	.0800					.0284	.09	.92	2.53	.71	.013	.00	.00	PIPE
1340.086	1683.254	.958	1684.212	17.16	11.53	2.06	1686.28	.00	1.49	2.00	2.000	.000	.00	1 .0
2.758	.0800					.0250	.07	.96	2.35	.71	.013	.00	.00	PIPE
1342.844	1683.474	.994	1684.468	17.16	10.99	1.88	1686.35	.00	1.49	2.00	2.000	.000	.00	1 .0
2.271	.0800					.0220	.05	.99	2.19	.71	.013	.00	.00	PIPE
1345.115	1683.656	1.033	1684.689	17.16	10.48	1.71	1686.40	.00	1.49	2.00	2.000	.000	.00	1 .0
1.900	.0800					.0194	.04	1.03	2.04	.71	.013	.00	.00	PIPE
1347.015	1683.808	1.073	1684.881	17.16	9.99	1.55	1686.43	.00	1.49	1.99	2.000	.000	.00	1 .0
1.575	.0800					.0171	.03	1.07	1.90	.71	.013	.00	.00	PIPE
1348.590	1683.934	1.115	1685.049	17.16	9.53	1.41	1686.46	.00	1.49	1.99	2.000	.000	.00	1 .0
1.298	.0800					.0151	.02	1.12	1.76	.71	.013	.00	.00	PIPE

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1349.888	1684.037	1.159	1685.197	17.16	9.09	1.28	1686.48	.00	1.49	1.97	2.000	.000	.00	1	.0
	1.044	.0800				.0134	.01	1.16	1.64	.71	.013	.00	.00	PIPE	
1350.931	1684.121	1.206	1685.327	17.16	8.66	1.17	1686.49	.00	1.49	1.96	2.000	.000	.00	1	.0
	.821	.0800				.0118	.01	1.21	1.52	.71	.013	.00	.00	PIPE	
1351.752	1684.187	1.256	1685.443	17.16	8.26	1.06	1686.50	.00	1.49	1.93	2.000	.000	.00	1	.0
	.638	.0800				.0105	.01	1.26	1.40	.71	.013	.00	.00	PIPE	
1352.390	1684.238	1.308	1685.546	17.16	7.88	.96	1686.51	.00	1.49	1.90	2.000	.000	.00	1	.0
	.432	.0800				.0093	.00	1.31	1.30	.71	.013	.00	.00	PIPE	
1352.823	1684.272	1.365	1685.637	17.16	7.51	.88	1686.51	.00	1.49	1.86	2.000	.000	.00	1	.0
	.260	.0800				.0083	.00	1.37	1.19	.71	.013	.00	.00	PIPE	
1353.082	1684.293	1.426	1685.719	17.16	7.16	.80	1686.52	.00	1.49	1.81	2.000	.000	.00	1	.0
	.088	.0800				.0074	.00	1.43	1.10	.71	.013	.00	.00	PIPE	
1353.170	1684.300	1.493	1685.793	17.16	6.82	.72	1686.52	.00	1.49	1.74	2.000	.000	.00	1	.0
JUNCT STR	.0214					.0039	.02	1.49	1.00		.013	.00	.00	PIPE	
1357.840	1684.400	2.564	1686.964	6.15	1.96	.06	1687.02	.00	.88	.00	2.000	.000	.00	1	.0
	29.620	.0000				.0007	.02	2.56	.00	.00	.013	.00	.00	PIPE	
1387.460	1684.400	2.586	1686.986	6.15	1.96	.06	1687.05	.00	.88	.00	2.000	.000	.00	1	.0
	48.031	.0129				.0007	.04	2.59	.00	.67	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1435.491 1685.022 2.000 1687.022 6.15 1.96 .06 1687.08 .00 .88 .00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
14.598 .0129 | | | | | .0007 .01 2.00 .00 .67 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1450.090 1685.211 1.814 1687.025 6.15 2.05 .07 1687.09 .00 .88 1.16 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.265 .0129 | | | | | .0007 .01 1.81 .23 .67 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1458.355 1685.318 1.706 1687.024 6.15 2.15 .07 1687.10 .00 .88 1.42 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.612 .0129 | | | | | .0007 .00 1.71 .27 .67 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1464.967 1685.403 1.618 1687.021 6.15 2.26 .08 1687.10 .00 .88 1.57 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.769 .0129 | | | | | .0008 .00 1.62 .30 .67 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1470.736 1685.478 1.540 1687.018 6.15 2.37 .09 1687.11 .00 .88 1.68 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.082 .0129 | | | | | .0009 .00 1.54 .34 .67 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1475.818 1685.544 1.470 1687.014 6.15 2.48 .10 1687.11 .00 .88 1.77 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.551 .0129 | | | | | .0010 .00 1.47 .37 .67 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1480.369 1685.603 1.406 1687.009 6.15 2.61 .11 1687.11 .00 .88 1.83 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.341 .0129 | | | | | .0011 .00 1.41 .40 .67 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1481.710 1685.620 1.387 1687.007 6.15 2.65 .11 1687.12 .00 .88 1.84 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.320 .0100 | | | | | .0011 .01 1.39 .42 .71 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1487.030 1685.673 1.329 1687.002 6.15 2.77 .12 1687.12 .00 .88 1.89 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.942 .0100 | | | | | .0013 .01 1.33 .45 .71 .013 .00 .00 PIPE
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20-750 Meridian Upper Plateau

100 Year Storm

Line 5

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1953.724	1690.342	.732	1691.074	6.15	5.90	.54	1691.61	.03	.88	1.93	2.000	.000	.00	1	.0
13.713	.0100					.0084	.12	.77	1.41	.71	.013	.00	.00	PIPE	
1967.437	1690.480	.759	1691.239	6.15	5.62	.49	1691.73	.03	.88	1.94	2.000	.000	.00	1	.0
6.658	.0100					.0074	.05	.79	1.32	.71	.013	.00	.00	PIPE	
1974.095	1690.547	.786	1691.333	6.15	5.36	.45	1691.78	.03	.88	1.95	2.000	.000	.00	1	.0
3.252	.0100					.0065	.02	.82	1.23	.71	.013	.00	.00	PIPE	
1977.348	1690.579	.815	1691.394	6.15	5.11	.41	1691.80	.03	.88	1.97	2.000	.000	.00	1	.0
1.580	.0100					.0057	.01	.84	1.15	.71	.013	.00	.00	PIPE	
1978.928	1690.595	.845	1691.440	6.15	4.87	.37	1691.81	.02	.88	1.98	2.000	.000	.00	1	.0
.472	.0100					.0050	.00	.87	1.07	.71	.013	.00	.00	PIPE	
1979.400	1690.600	.877	1691.477	6.15	4.64	.33	1691.81	.00	.88	1.98	2.000	.000	.00	1	.0
JUNCT STR	.0214					.0027	.01	.88	1.00		.013	.00	.00	PIPE	
1984.070	1690.700	1.232	1691.932	2.67	1.72	.05	1691.98	.00	.62	1.15	1.500	.000	.00	1	.0
6.212	.0098					.0007	.00	1.23	.26	.52	.013	.00	.00	PIPE	
1990.282	1690.761	1.171	1691.932	2.67	1.80	.05	1691.98	.00	.62	1.24	1.500	.000	.00	1	.0
5.408	.0098					.0007	.00	1.17	.29	.52	.013	.00	.00	PIPE	
1995.690	1690.814	1.117	1691.931	2.67	1.89	.06	1691.99	.00	.62	1.31	1.500	.000	.00	1	.0
4.846	.0098					.0008	.00	1.12	.32	.52	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
2000.536	1690.861	1.068	1691.929	2.67	1.98	.06	1691.99	.00	.62	1.36	1.500	.000	.00	1	.0
4.387	.0098					.0009	.00	1.07	.35	.52	.013	.00	.00	PIPE	
2004.923	1690.904	1.023	1691.927	2.67	2.08	.07	1691.99	.00	.62	1.40	1.500	.000	.00	1	.0
4.146	.0098					.0010	.00	1.02	.38	.52	.013	.00	.00	PIPE	
2009.070	1690.945	.980	1691.925	2.67	2.18	.07	1692.00	.00	.62	1.43	1.500	.000	.00	1	.0
3.668	.0098					.0012	.00	.98	.42	.52	.013	.00	.00	PIPE	
2012.737	1690.981	.941	1691.922	2.67	2.29	.08	1692.00	.00	.62	1.45	1.500	.000	.00	1	.0
3.528	.0098					.0013	.00	.94	.45	.52	.013	.00	.00	PIPE	
2016.265	1691.015	.903	1691.918	2.67	2.40	.09	1692.01	.00	.62	1.47	1.500	.000	.00	1	.0
3.141	.0098					.0015	.00	.90	.49	.52	.013	.00	.00	PIPE	
2019.406	1691.046	.868	1691.914	2.67	2.52	.10	1692.01	.00	.62	1.48	1.500	.000	.00	1	.0
2.861	.0098					.0017	.00	.87	.52	.52	.013	.00	.00	PIPE	
2022.267	1691.074	.835	1691.909	2.67	2.64	.11	1692.02	.00	.62	1.49	1.500	.000	.00	1	.0
2.564	.0098					.0019	.00	.84	.56	.52	.013	.00	.00	PIPE	
2024.831	1691.099	.804	1691.903	2.67	2.77	.12	1692.02	.00	.62	1.50	1.500	.000	.00	1	.0
2.377	.0098					.0022	.01	.80	.61	.52	.013	.00	.00	PIPE	
2027.208	1691.123	.774	1691.897	2.67	2.90	.13	1692.03	.00	.62	1.50	1.500	.000	.00	1	.0
2.174	.0098					.0025	.01	.77	.65	.52	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2029.382 1691.144 .745 1691.889 2.67 3.04 .14 1692.03 .00 .62 1.50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.740 .0098 .0028 .00 .75 .70 .52 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2030.122 1691.151 .718 1691.869 2.67 3.19 .16 1692.03 .00 .62 1.50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2030.122 1691.151 .518 1691.670 2.67 4.93 .38 1692.05 .00 .62 1.43 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
421.878 .0098 .0098 4.13 .52 1.41 .52 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2451.999 1695.286 .518 1695.804 2.67 4.93 .38 1696.18 .00 .62 1.43 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
26.915 .0098 .0101 .27 .52 1.41 .52 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2478.915 1695.549 .509 1696.058 2.67 5.04 .40 1696.45 .00 .62 1.42 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
16.226 .0098 .0112 .18 .51 1.46 .52 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2495.141 1695.708 .492 1696.200 2.67 5.29 .43 1696.64 .00 .62 1.41 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.905 .0098 .0128 .11 .49 1.56 .52 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2504.046 1695.796 .475 1696.271 2.67 5.55 .48 1696.75 .00 .62 1.40 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.652 .0098 .0146 .10 .48 1.67 .52 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2510.698 1695.861 .459 1696.320 2.67 5.82 .53 1696.85 .00 .62 1.38 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.344 .0098 .0166 .09 .46 1.78 .52 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2516.042 1695.913 .443 1696.356 2.67 6.10 .58 1696.93 .00 .62 1.37 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.653 .0098 .0190 .09 .44 1.90 .52 .013 .00 .00 PIPE

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WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2520.695 |1695.959|.428 |1696.387| 2.67 | 6.40 | .64 |1697.02|.00 |.62 | 1.35 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.195 |.0098|
|.0217|.09|.43|2.03|.52|.013|.00|.00|PIPE
2524.890 |1696.000|.415 |1696.415| 2.67 | 6.71 | .70 |1697.11|.60 |.62 | 1.34 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR|.2041|.0187|.05|1.02|2.17|.013|.00|.00|PIPE
2527.340 |1696.500|.331 |1696.831| 1.34 | 4.63 | .33 |1697.16|.00 |.43 | 1.24 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
14.194|.0147|.0135|.19|.33|1.69|.33|.013|.00|.00|PIPE
2541.534 |1696.708|.341 |1697.050| 1.34 | 4.43 | .31 |1697.35|.00 |.43 | 1.26 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.899|.0147|.0118|.07|.34|1.59|.33|.013|.00|.00|PIPE
2547.433 |1696.795|.352 |1697.147| 1.34 | 4.23 | .28 |1697.42|.00 |.43 | 1.27 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.052|.0147|.0103|.03|.35|1.49|.33|.013|.00|.00|PIPE
2550.485 |1696.840|.364 |1697.204| 1.34 | 4.03 | .25 |1697.46|.00 |.43 | 1.29 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.765|.0147|.0090|.02|.36|1.40|.33|.013|.00|.00|PIPE
2552.250 |1696.866|.377 |1697.243| 1.34 | 3.84 | .23 |1697.47|.00 |.43 | 1.30 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.161|.0147|.0079|.01|.38|1.31|.33|.013|.00|.00|PIPE
2553.411 |1696.883|.390 |1697.273| 1.34 | 3.66 | .21 |1697.48|.00 |.43 | 1.32 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.768|.0147|.0069|.01|.39|1.23|.33|.013|.00|.00|PIPE
2554.179 |1696.894|.403 |1697.297| 1.34 | 3.49 | .19 |1697.49|.00 |.43 | 1.33 | 1.500 |.000|.00 | 1 |.0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.375|.0147|.0061|.00|.40|1.15|.33|.013|.00|.00|PIPE

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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:49: 6

20-750 Meridian Upper Plateau

100 Year Storm

Line 5

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
2554.554	1696.900	.417	1697.317	1.34	3.33	.17	1697.49	.00	.43	1.34	1.500	.000	.00	1 .0
	.036	.0147				.0053	.00	.42	1.07	.33	.013	.00	.00	PIPE
2554.590	1696.900	.433	1697.333	1.34	3.17	.16	1697.49	.00	.43	1.36	1.500	.000	.00	1 .0

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:49:25

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1000.000	1652.000	.828	1652.828	52.71	33.21	17.13	1669.95	.00	2.36	2.68	3.000	.000	.00	1	.0
12.172	.3361					.2114	2.57	.83	7.61	.75	.013	.00	.00	PIPE	
1012.172	1656.091	.856	1656.947	52.71	31.66	15.56	1672.51	.00	2.36	2.71	3.000	.000	.00	1	.0
9.158	.3361					.1850	1.69	.86	7.12	.75	.013	.00	.00	PIPE	
1021.330	1659.170	.886	1660.056	52.71	30.18	14.15	1674.20	.00	2.36	2.74	3.000	.000	.00	1	.0
7.203	.3361					.1619	1.17	.89	6.66	.75	.013	.00	.00	PIPE	
1028.533	1661.591	.917	1662.508	52.71	28.78	12.86	1675.37	.00	2.36	2.76	3.000	.000	.00	1	.0
5.850	.3361					.1418	.83	.92	6.23	.75	.013	.00	.00	PIPE	
1034.383	1663.557	.949	1664.507	52.71	27.44	11.69	1676.20	.00	2.36	2.79	3.000	.000	.00	1	.0
4.854	.3361					.1242	.60	.95	5.83	.75	.013	.00	.00	PIPE	
1039.237	1665.189	.983	1666.172	52.71	26.16	10.63	1676.80	.00	2.36	2.82	3.000	.000	.00	1	.0
4.100	.3361					.1088	.45	.98	5.45	.75	.013	.00	.00	PIPE	
1043.337	1666.567	1.017	1667.584	52.71	24.94	9.66	1677.25	.00	2.36	2.84	3.000	.000	.00	1	.0
3.497	.3361					.0953	.33	1.02	5.10	.75	.013	.00	.00	PIPE	
1046.834	1667.743	1.053	1668.796	52.71	23.78	8.78	1677.58	.00	2.36	2.86	3.000	.000	.00	1	.0
3.010	.3361					.0835	.25	1.05	4.76	.75	.013	.00	.00	PIPE	
1049.844	1668.755	1.091	1669.846	52.71	22.68	7.99	1677.83	.00	2.36	2.89	3.000	.000	.00	1	.0
2.613	.3361					.0732	.19	1.09	4.45	.75	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1052.457	1669.633	1.130	1670.763	52.71	21.62	7.26	1678.02	.00	2.36	2.91	3.000	.000	.00	1 .0
	2.276	.3361				.0642	.15	1.13	4.16	.75	.013	.00	.00	PIPE
1054.734	1670.398	1.171	1671.569	52.71	20.62	6.60	1678.17	.00	2.36	2.93	3.000	.000	.00	1 .0
	1.991	.3361				.0564	.11	1.17	3.89	.75	.013	.00	.00	PIPE
1056.724	1671.068	1.214	1672.282	52.71	19.66	6.00	1678.28	.00	2.36	2.95	3.000	.000	.00	1 .0
	1.749	.3361				.0495	.09	1.21	3.63	.75	.013	.00	.00	PIPE
1058.473	1671.656	1.258	1672.914	52.71	18.74	5.45	1678.37	.00	2.36	2.96	3.000	.000	.00	1 .0
	1.537	.3361				.0435	.07	1.26	3.39	.75	.013	.00	.00	PIPE
1060.010	1672.172	1.304	1673.476	52.71	17.87	4.96	1678.43	.00	2.36	2.97	3.000	.000	.00	1 .0
	1.352	.3361				.0382	.05	1.30	3.16	.75	.013	.00	.00	PIPE
1061.362	1672.626	1.352	1673.979	52.71	17.04	4.51	1678.49	.00	2.36	2.99	3.000	.000	.00	1 .0
	1.186	.3361				.0336	.04	1.35	2.95	.75	.013	.00	.00	PIPE
1062.547	1673.025	1.403	1674.428	52.71	16.24	4.10	1678.53	.00	2.36	2.99	3.000	.000	.00	1 .0
	1.042	.3361				.0295	.03	1.40	2.75	.75	.013	.00	.00	PIPE
1063.589	1673.375	1.456	1674.831	52.71	15.49	3.73	1678.56	.00	2.36	3.00	3.000	.000	.00	1 .0
	.915	.3361				.0260	.02	1.46	2.56	.75	.013	.00	.00	PIPE
1064.504	1673.683	1.511	1675.194	52.71	14.77	3.39	1678.58	.00	2.36	3.00	3.000	.000	.00	1 .0
	.798	.3361				.0229	.02	1.51	2.39	.75	.013	.00	.00	PIPE

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1065.302	1673.951	1.569	1675.520	52.71	14.08	3.08	1678.60	.00	2.36	3.00	3.000	.000	.00	1	.0
	.693	.3361				.0202	.01	1.57	2.22	.75	.013	.00	.00	PIPE	
1065.994	1674.184	1.630	1675.814	52.71	13.43	2.80	1678.61	.00	2.36	2.99	3.000	.000	.00	1	.0
	.595	.3361				.0178	.01	1.63	2.06	.75	.013	.00	.00	PIPE	
1066.589	1674.384	1.695	1676.079	52.71	12.80	2.54	1678.62	.00	2.36	2.97	3.000	.000	.00	1	.0
	.513	.3361				.0157	.01	1.70	1.92	.75	.013	.00	.00	PIPE	
1067.102	1674.556	1.762	1676.318	52.71	12.20	2.31	1678.63	.00	2.36	2.95	3.000	.000	.00	1	.0
	.428	.3361				.0139	.01	1.76	1.78	.75	.013	.00	.00	PIPE	
1067.530	1674.700	1.835	1676.535	52.71	11.64	2.10	1678.64	.00	2.36	2.92	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0161	.08	1.83	1.65		.013	.00	.00	PIPE	
1072.200	1674.800	1.555	1676.355	48.92	13.23	2.72	1679.07	.00	2.28	3.00	3.000	.000	.00	1	.0
	1.533	.0528				.0187	.03	1.55	2.10	1.17	.013	.00	.00	PIPE	
1073.733	1674.881	1.570	1676.451	48.92	13.06	2.65	1679.10	.00	2.28	3.00	3.000	.000	.00	1	.0
	5.036	.0528				.0173	.09	1.57	2.06	1.17	.013	.00	.00	PIPE	
1078.769	1675.147	1.632	1676.779	48.92	12.45	2.41	1679.18	.00	2.28	2.99	3.000	.000	.00	1	.0
	4.126	.0528				.0153	.06	1.63	1.91	1.17	.013	.00	.00	PIPE	
1082.896	1675.365	1.696	1677.061	48.92	11.87	2.19	1679.25	.00	2.28	2.97	3.000	.000	.00	1	.0
	3.331	.0528				.0135	.04	1.70	1.78	1.17	.013	.00	.00	PIPE	



20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
1086.226	1675.541	1.764	1677.305	48.92	11.32	1.99	1679.29	.00	2.28	2.95	3.000	.000	.00	1	.0
	2.663	.0528				.0119	.03	1.76	1.65	1.17	.013	.00	.00	PIPE	
1088.889	1675.681	1.836	1677.517	48.92	10.79	1.81	1679.32	.00	2.28	2.92	3.000	.000	.00	1	.0
	2.093	.0528				.0106	.02	1.84	1.53	1.17	.013	.00	.00	PIPE	
1090.982	1675.792	1.912	1677.704	48.92	10.29	1.64	1679.35	.00	2.28	2.88	3.000	.000	.00	1	.0
	1.576	.0528				.0094	.01	1.91	1.41	1.17	.013	.00	.00	PIPE	
1092.558	1675.875	1.993	1677.868	48.92	9.81	1.49	1679.36	.00	2.28	2.83	3.000	.000	.00	1	.0
	1.098	.0528				.0084	.01	1.99	1.30	1.17	.013	.00	.00	PIPE	
1093.656	1675.933	2.080	1678.013	48.92	9.35	1.36	1679.37	.00	2.28	2.77	3.000	.000	.00	1	.0
	.650	.0528				.0074	.00	2.08	1.20	1.17	.013	.00	.00	PIPE	
1094.306	1675.967	2.174	1678.141	48.92	8.92	1.23	1679.38	.00	2.28	2.68	3.000	.000	.00	1	.0
	.224	.0528				.0067	.00	2.17	1.10	1.17	.013	.00	.00	PIPE	
1094.530	1675.979	2.277	1678.256	48.92	8.50	1.12	1679.38	.00	2.28	2.57	3.000	.000	.00	1	.0
JUNCT STR	-.0805					.0050	.02	2.28	1.00		.013	.00	.00	PIPE	
1099.240	1675.600	3.582	1679.182	40.26	5.70	.50	1679.69	.00	2.07	.00	3.000	.000	.00	1	.0
	10.177	.0298				.0036	.04	3.58	.00	1.22	.013	.00	.00	PIPE	
1109.417	1675.904	3.315	1679.219	40.26	5.70	.50	1679.72	.00	2.07	.00	3.000	.000	.00	1	.0

HYDRAULIC JUMP

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:49:25

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1109.417	1675.904	1.224	1677.128	40.26	14.85	3.42	1680.55	.00	2.07	2.95	3.000	.000	.00	1	.0
148.208	.0298					.0298	4.42	1.22	2.73	1.22	.013	.00	.00	PIPE	
1257.625	1680.326	1.224	1681.550	40.26	14.85	3.42	1684.97	.00	2.07	2.95	3.000	.000	.00	1	.0
146.437	.0298					.0285	4.17	1.22	2.73	1.22	.013	.00	.00	PIPE	
1404.062	1684.696	1.255	1685.951	40.26	14.35	3.20	1689.15	.00	2.07	2.96	3.000	.000	.00	1	.0
56.456	.0298					.0255	1.44	1.26	2.60	1.22	.013	.00	.00	PIPE	
1460.518	1686.381	1.302	1687.683	40.26	13.68	2.91	1690.59	.00	2.07	2.97	3.000	.000	.00	1	.0
29.184	.0298					.0224	.65	1.30	2.42	1.22	.013	.00	.00	PIPE	
1489.702	1687.252	1.350	1688.602	40.26	13.05	2.64	1691.24	.00	2.07	2.98	3.000	.000	.00	1	.0
18.786	.0298					.0197	.37	1.35	2.26	1.22	.013	.00	.00	PIPE	
1508.489	1687.813	1.400	1689.213	40.26	12.44	2.40	1691.62	.00	2.07	2.99	3.000	.000	.00	1	.0
13.231	.0298					.0173	.23	1.40	2.11	1.22	.013	.00	.00	PIPE	
1521.719	1688.207	1.453	1689.660	40.26	11.86	2.18	1691.84	.00	2.07	3.00	3.000	.000	.00	1	.0
9.811	.0298					.0153	.15	1.45	1.96	1.22	.013	.00	.00	PIPE	
1531.530	1688.500	1.509	1690.009	40.26	11.31	1.99	1691.99	.00	2.07	3.00	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0156	.07	1.51	1.83		.013	.00	.00	PIPE	
1536.200	1688.600	1.407	1690.007	38.81	11.92	2.21	1692.21	.00	2.03	2.99	3.000	.000	.00	1	.0
16.683	.0200					.0165	.27	1.41	2.01	1.34	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1552.883	1688.933	1.427	1690.360	38.81	11.70	2.13	1692.49	.00	2.03	3.00	3.000	.000	.00	1	.0
28.367	.0200					.0151	.43	1.43	1.96	1.34	.013	.00	.00	PIPE	
1581.250	1689.500	1.482	1690.982	38.81	11.16	1.93	1692.91	.00	2.03	3.00	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0151	.07	1.48	1.83		.013	.00	.00	PIPE	
1585.920	1689.600	1.405	1691.005	37.71	11.60	2.09	1693.09	.00	2.00	2.99	3.000	.000	.00	1	.0
.973	.0194					.0160	.02	1.41	1.96	1.33	.013	.00	.00	PIPE	
1586.893	1689.619	1.406	1691.025	37.71	11.59	2.08	1693.11	.00	2.00	2.99	3.000	.000	.00	1	.0
30.885	.0194					.0150	.46	1.41	1.96	1.33	.013	.00	.00	PIPE	
1617.778	1690.218	1.459	1691.677	37.71	11.05	1.90	1693.57	.00	2.00	3.00	3.000	.000	.00	1	.0
18.714	.0194					.0132	.25	1.46	1.82	1.33	.013	.00	.00	PIPE	
1636.492	1690.581	1.515	1692.096	37.71	10.53	1.72	1693.82	.00	2.00	3.00	3.000	.000	.00	1	.0
12.669	.0194					.0116	.15	1.52	1.70	1.33	.013	.00	.00	PIPE	
1649.161	1690.827	1.573	1692.400	37.71	10.04	1.57	1693.97	.00	2.00	3.00	3.000	.000	.00	1	.0
8.883	.0194					.0102	.09	1.57	1.58	1.33	.013	.00	.00	PIPE	
1658.044	1690.999	1.634	1692.634	37.71	9.58	1.42	1694.06	.00	2.00	2.99	3.000	.000	.00	1	.0
6.217	.0194					.0090	.06	1.63	1.47	1.33	.013	.00	.00	PIPE	
1664.260	1691.120	1.699	1692.819	37.71	9.13	1.29	1694.11	.00	2.00	2.97	3.000	.000	.00	1	.0
4.350	.0194					.0080	.03	1.70	1.37	1.33	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1668.610	1691.204	1.767	1692.972	37.71	8.71	1.18	1694.15	.00	2.00	2.95	3.000	.000	.00	1	.0
2.834	.0194					.0071	.02	1.77	1.27	1.33	.013	.00	.00	PIPE	
1671.444	1691.260	1.839	1693.099	37.71	8.30	1.07	1694.17	.00	2.00	2.92	3.000	.000	.00	1	.0
1.616	.0194					.0063	.01	1.84	1.17	1.33	.013	.00	.00	PIPE	
1673.061	1691.291	1.915	1693.206	37.71	7.91	.97	1694.18	.00	2.00	2.88	3.000	.000	.00	1	.0
.469	.0194					.0055	.00	1.92	1.08	1.33	.013	.00	.00	PIPE	
1673.530	1691.300	1.998	1693.298	37.71	7.54	.88	1694.18	.00	2.00	2.83	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0033	.02	2.00	1.00		.013	.00	.00	PIPE	
1678.210	1691.400	2.929	1694.329	3.82	2.16	.07	1694.40	.00	.75	.00	1.500	.000	.00	1	.0
77.059	.0199					.0013	.10	2.93	.00	.52	.013	.00	.00	PIPE	
1755.269	1692.931	1.500	1694.431	3.82	2.16	.07	1694.50	.00	.75	.00	1.500	.000	.00	1	.0
7.061	.0199					.0012	.01	1.50	.00	.52	.013	.00	.00	PIPE	
1762.330	1693.071	1.360	1694.432	3.82	2.27	.08	1694.51	.00	.75	.87	1.500	.000	.00	1	.0
3.857	.0199					.0012	.00	1.36	.29	.52	.013	.00	.00	PIPE	
1766.187	1693.148	1.280	1694.428	3.82	2.38	.09	1694.52	.00	.75	1.06	1.500	.000	.00	1	.0
3.135	.0199					.0013	.00	1.28	.34	.52	.013	.00	.00	PIPE	
1769.322	1693.210	1.213	1694.423	3.82	2.49	.10	1694.52	.00	.75	1.18	1.500	.000	.00	1	.0
2.621	.0199					.0014	.00	1.21	.39	.52	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1771.943	1693.262	1.155	1694.418	3.82	2.62	.11	1694.52	.00	.75	1.26	1.500	.000	.00	1	.0
	2.317	.0199				.0016	.00	1.16	.43	.52	.013	.00	.00	PIPE	
1774.260	1693.308	1.102	1694.411	3.82	2.74	.12	1694.53	.00	.75	1.32	1.500	.000	.00	1	.0
	2.006	.0199				.0018	.00	1.10	.47	.52	.013	.00	.00	PIPE	
1776.266	1693.348	1.054	1694.402	3.82	2.88	.13	1694.53	.00	.75	1.37	1.500	.000	.00	1	.0
	.390	.0199				.0020	.00	1.05	.52	.52	.013	.00	.00	PIPE	
1776.656	1693.356	1.010	1694.366	3.82	3.02	.14	1694.51	.00	.75	1.41	1.500	.000	.00	1	.0
HYDRAULIC JUMP															
1776.656	1693.356	.520	1693.876	3.82	7.03	.77	1694.64	.00	.75	1.43	1.500	.000	.00	1	.0
	220.902	.0199				.0199	4.39	.52	2.01	.52	.013	.00	.00	PIPE	
1997.558	1697.745	.520	1698.265	3.82	7.03	.77	1699.03	.00	.75	1.43	1.500	.000	.00	1	.0
	45.979	.0199				.0198	.91	.52	2.01	.52	.013	.00	.00	PIPE	
2043.537	1698.659	.520	1699.179	3.82	7.01	.76	1699.94	.00	.75	1.43	1.500	.000	.00	1	.0
	36.961	.0199				.0185	.68	.52	2.00	.52	.013	.00	.00	PIPE	
2080.498	1699.393	.539	1699.932	3.82	6.68	.69	1700.63	.00	.75	1.44	1.500	.000	.00	1	.0
	12.094	.0199				.0162	.20	.54	1.87	.52	.013	.00	.00	PIPE	
2092.592	1699.633	.558	1700.191	3.82	6.37	.63	1700.82	.00	.75	1.45	1.500	.000	.00	1	.0
	6.613	.0199				.0142	.09	.56	1.75	.52	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
2099.206	1699.765	.578	1700.343	3.82	6.08	.57	1700.92	.00	.75	1.46	1.500	.000	.00	1	.0
4.210	.0199					.0125	.05	.58	1.63	.52	.013	.00	.00	PIPE	
2103.415	1699.848	.599	1700.447	3.82	5.79	.52	1700.97	.00	.75	1.47	1.500	.000	.00	1	.0
2.846	.0199					.0110	.03	.60	1.52	.52	.013	.00	.00	PIPE	
2106.261	1699.905	.621	1700.526	3.82	5.52	.47	1701.00	.00	.75	1.48	1.500	.000	.00	1	.0
1.958	.0199					.0096	.02	.62	1.42	.52	.013	.00	.00	PIPE	
2108.219	1699.944	.644	1700.588	3.82	5.27	.43	1701.02	.00	.75	1.48	1.500	.000	.00	1	.0
1.327	.0199					.0085	.01	.64	1.33	.52	.013	.00	.00	PIPE	
2109.545	1699.970	.668	1700.638	3.82	5.02	.39	1701.03	.00	.75	1.49	1.500	.000	.00	1	.0
.931	.0199					.0074	.01	.67	1.24	.52	.013	.00	.00	PIPE	
2110.476	1699.989	.692	1700.681	3.82	4.79	.36	1701.04	.00	.75	1.50	1.500	.000	.00	1	.0
.476	.0199					.0065	.00	.69	1.15	.52	.013	.00	.00	PIPE	
2110.952	1699.998	.718	1700.716	3.82	4.56	.32	1701.04	.00	.75	1.50	1.500	.000	.00	1	.0
.098	.0199					.0057	.00	.72	1.08	.52	.013	.00	.00	PIPE	
2111.050	1700.000	.747	1700.747	3.82	4.34	.29	1701.04	.00	.75	1.50	1.500	.000	.00	1	.0
JUNCT STR	.0272					.0030	.01	.75	1.00		.013	.00	.00	PIPE	
2114.720	1700.100	.982	1701.082	1.91	1.56	.04	1701.12	.00	.52	1.43	1.500	.000	.00	1	.0
1.694	.0218					.0006	.00	.98	.30	.36	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2116.414 1700.137 .942 1701.079 1.91 1.63 .04 1701.12 .00 .52 1.45 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.599 .0218 .0007 .00 .94 .32 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2118.013 1700.172 .904 1701.076 1.91 1.71 .05 1701.12 .00 .52 1.47 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.444 .0218 .0008 .00 .90 .35 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2119.457 1700.203 .869 1701.073 1.91 1.80 .05 1701.12 .00 .52 1.48 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.334 .0218 .0009 .00 .87 .37 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2120.791 1700.233 .836 1701.069 1.91 1.89 .06 1701.12 .00 .52 1.49 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.269 .0218 .0010 .00 .84 .40 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2122.060 1700.260 .804 1701.064 1.91 1.98 .06 1701.13 .00 .52 1.50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.154 .0218 .0011 .00 .80 .43 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2123.214 1700.286 .774 1701.060 1.91 2.07 .07 1701.13 .00 .52 1.50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.036 .0218 .0013 .00 .77 .47 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2124.249 1700.308 .746 1701.054 1.91 2.18 .07 1701.13 .00 .52 1.50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.963 .0218 .0014 .00 .75 .50 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2125.212 1700.329 .719 1701.048 1.91 2.28 .08 1701.13 .00 .52 1.50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.547 .0218 .0016 .00 .72 .54 .36 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2125.759 1700.341 .693 1701.034 1.91 2.39 .09 1701.12 .00 .52 1.50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

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HYDRAULIC JUMP

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 7

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
2125.759	1700.341	.368	1700.709	1.91	5.68	.50	1701.21	.00	.52	1.29	1.500	.000	.00	1	.0
	3.372	.0218				.0177	.06	.37	1.96	.36	.013	.00	.00	PIPE	
2129.131	1700.415	.381	1700.796	1.91	5.40	.45	1701.25	.00	.52	1.31	1.500	.000	.00	1	.0
	4.379	.0218				.0154	.07	.38	1.83	.36	.013	.00	.00	PIPE	
2133.510	1700.510	.394	1700.905	1.91	5.15	.41	1701.32	.00	.52	1.32	1.500	.000	.00	1	.0
	2.800	.0218				.0135	.04	.39	1.71	.36	.013	.00	.00	PIPE	
2136.311	1700.572	.408	1700.980	1.91	4.91	.37	1701.35	.00	.52	1.34	1.500	.000	.00	1	.0
	1.991	.0218				.0118	.02	.41	1.60	.36	.013	.00	.00	PIPE	
2138.302	1700.615	.422	1701.037	1.91	4.68	.34	1701.38	.00	.52	1.35	1.500	.000	.00	1	.0
	1.381	.0218				.0103	.01	.42	1.50	.36	.013	.00	.00	PIPE	
2139.683	1700.645	.437	1701.082	1.91	4.46	.31	1701.39	.00	.52	1.36	1.500	.000	.00	1	.0
	1.023	.0218				.0091	.01	.44	1.40	.36	.013	.00	.00	PIPE	
2140.706	1700.668	.452	1701.120	1.91	4.25	.28	1701.40	.00	.52	1.38	1.500	.000	.00	1	.0
	.685	.0218				.0079	.01	.45	1.31	.36	.013	.00	.00	PIPE	
2141.391	1700.683	.468	1701.151	1.91	4.06	.26	1701.41	.00	.52	1.39	1.500	.000	.00	1	.0
	.484	.0218				.0069	.00	.47	1.23	.36	.013	.00	.00	PIPE	
2141.874	1700.693	.484	1701.177	1.91	3.87	.23	1701.41	.00	.52	1.40	1.500	.000	.00	1	.0
	.260	.0218				.0061	.00	.48	1.15	.36	.013	.00	.00	PIPE	



20-750 Meridian Upper Plateau

100 Year Storm

Line 7

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT|or I.D. | ZL | Prs/Pip
-|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|-
L/Elem |Ch Slope | | | | | SF Ave | HF |SE Dpth|Froude N|Norm Dp | "N" | X-Fall| ZR |Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
| | | | | | | | | | | | | | | | |
2142.134 1700.699 .501 1701.200 1.91 3.69 .21 1701.41 .00 .52 1.41 1.500 .000 .00 1 .0
-|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|-
.046 .0218 | | | | | .0053 .00 .50 1.07 .36 .013 .00 .00 PIPE
| | | | | | | | | | | | | | | | |
2142.180 1700.700 .521 1701.220 1.91 3.51 .19 1701.41 .00 .52 1.43 1.500 .000 .00 1 .0
-|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|- -|-

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WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 8

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1000.000	1648.000	1.062	1649.062	33.70	15.04	3.51	1652.58	.00	1.89	2.87	3.000	.000	.00	1	.0
	.376	.3663				.0344	.01	1.06	3.00	.59	.013	.00	.00	PIPE	
1000.376	1648.138	1.078	1649.216	33.70	14.74	3.37	1652.59	.00	1.89	2.88	3.000	.000	.00	1	.0
	.799	.3663				.0313	.03	1.08	2.91	.59	.013	.00	.00	PIPE	
1001.175	1648.430	1.117	1649.547	33.70	14.05	3.07	1652.61	.00	1.89	2.90	3.000	.000	.00	1	.0
	.705	.3663				.0275	.02	1.12	2.72	.59	.013	.00	.00	PIPE	
1001.880	1648.688	1.157	1649.845	33.70	13.40	2.79	1652.63	.00	1.89	2.92	3.000	.000	.00	1	.0
	.618	.3663				.0241	.01	1.16	2.54	.59	.013	.00	.00	PIPE	
1002.497	1648.915	1.199	1650.114	33.70	12.77	2.53	1652.65	.00	1.89	2.94	3.000	.000	.00	1	.0
	.543	.3663				.0211	.01	1.20	2.38	.59	.013	.00	.00	PIPE	
1003.040	1649.113	1.242	1650.355	33.70	12.18	2.30	1652.66	.00	1.89	2.96	3.000	.000	.00	1	.0
	.470	.3663				.0186	.01	1.24	2.22	.59	.013	.00	.00	PIPE	
1003.510	1649.286	1.288	1650.574	33.70	11.61	2.09	1652.67	.00	1.89	2.97	3.000	.000	.00	1	.0
	.407	.3663				.0163	.01	1.29	2.07	.59	.013	.00	.00	PIPE	
1003.917	1649.435	1.336	1650.771	33.70	11.07	1.90	1652.67	.00	1.89	2.98	3.000	.000	.00	1	.0
	.352	.3663				.0143	.01	1.34	1.93	.59	.013	.00	.00	PIPE	
1004.269	1649.564	1.385	1650.949	33.70	10.56	1.73	1652.68	.00	1.89	2.99	3.000	.000	.00	1	.0
	.298	.3663				.0126	.00	1.39	1.80	.59	.013	.00	.00	PIPE	

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 8

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1004.567	1649.673	1.437	1651.110	33.70	10.07	1.57	1652.68	.00	1.89	3.00	3.000	.000	.00	1	.0
	.248					.0111	.00		1.44	1.68	.59	.013	.00		PIPE
1004.815	1649.764	1.492	1651.256	33.70	9.60	1.43	1652.69	.00	1.89	3.00	3.000	.000	.00	1	.0
	.205					.0098	.00		1.49	1.56	.59	.013	.00		PIPE
1005.020	1649.839	1.549	1651.388	33.70	9.15	1.30	1652.69	.00	1.89	3.00	3.000	.000	.00	1	.0
	.163					.0086	.00		1.55	1.46	.59	.013	.00		PIPE
1005.182	1649.898	1.609	1651.507	33.70	8.72	1.18	1652.69	.00	1.89	2.99	3.000	.000	.00	1	.0
	.124					.0076	.00		1.61	1.35	.59	.013	.00		PIPE
1005.306	1649.944	1.672	1651.616	33.70	8.32	1.07	1652.69	.00	1.89	2.98	3.000	.000	.00	1	.0
	.085					.0067	.00		1.67	1.26	.59	.013	.00		PIPE
1005.392	1649.975	1.739	1651.714	33.70	7.93	.98	1652.69	.00	1.89	2.96	3.000	.000	.00	1	.0
	.052					.0059	.00		1.74	1.17	.59	.013	.00		PIPE
1005.444	1649.994	1.809	1651.803	33.70	7.56	.89	1652.69	.00	1.89	2.94	3.000	.000	.00	1	.0
	.016					.0052	.00		1.81	1.08	.59	.013	.00		PIPE
1005.460	1650.000	1.885	1651.885	33.70	7.21	.81	1652.69	.00	1.89	2.90	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0032	.01		1.89	1.00		.013	.00		PIPE
1010.140	1650.100	3.042	1653.142	8.43	2.68	.11	1653.25	.00	1.03	.00	2.000	.000	.00	1	.0
	1.880	.2544				.0014	.00		3.04	.00	.37	.013	.00		PIPE

20-750 Meridian Upper Plateau

100 Year Storm

Line 8

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1012.020	1650.578	2.566	1653.144	8.43	2.68	.11	1653.26	.00	1.03	.00	2.000	.000	.00	1	.0
HYDRAULIC JUMP															
1012.020	1650.578	.368	1650.946	8.43	21.25	7.01	1657.96	.00	1.03	1.55	2.000	.000	.00	1	.0
6.690	.2544					.2543	1.70	.37	7.40	.37	.013	.00	.00	PIPE	
1018.710	1652.280	.368	1652.648	8.43	21.24	7.00	1659.65	.48	1.03	1.55	2.000	.000	.00	1	.0
30.428	.2540					.2540	7.73	.85	7.40	.37	.013	.00	.00	PIPE	
1049.138	1660.007	.368	1660.375	8.43	21.24	7.00	1667.38	.48	1.03	1.55	2.000	.000	.00	1	.0
40.252	.2540					.2529	10.18	.85	7.40	.37	.013	.00	.00	PIPE	
1089.390	1670.230	.369	1670.599	8.43	21.17	6.96	1677.56	.00	1.03	1.55	2.000	.000	.00	1	.0
34.687	.2544					.2407	8.35	.37	7.36	.37	.013	.00	.00	PIPE	
1124.078	1679.053	.377	1679.430	8.43	20.50	6.52	1685.95	.00	1.03	1.56	2.000	.000	.00	1	.0
14.756	.2544					.2150	3.17	.38	7.05	.37	.013	.00	.00	PIPE	
1138.833	1682.806	.389	1683.195	8.43	19.54	5.93	1689.13	.00	1.03	1.58	2.000	.000	.00	1	.0
7.893	.2544					.1878	1.48	.39	6.60	.37	.013	.00	.00	PIPE	
1146.726	1684.813	.403	1685.217	8.43	18.63	5.39	1690.61	.00	1.03	1.60	2.000	.000	.00	1	.0
5.290	.2544					.1642	.87	.40	6.18	.37	.013	.00	.00	PIPE	
1152.016	1686.159	.416	1686.575	8.43	17.77	4.90	1691.48	.00	1.03	1.62	2.000	.000	.00	1	.0
3.883	.2544					.1435	.56	.42	5.79	.37	.013	.00	.00	PIPE	

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 8

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1155.899	1687.147	.431	1687.578	8.43	16.94	4.46	1692.03	.00	1.03	1.64	2.000	.000	.00	1	.0
	3.033	.2544				.1254	.38	.43	5.43	.37	.013	.00	.00	PIPE	
1158.933	1687.918	.445	1688.363	8.43	16.15	4.05	1692.41	.00	1.03	1.66	2.000	.000	.00	1	.0
	2.440	.2544				.1096	.27	.45	5.08	.37	.013	.00	.00	PIPE	
1161.372	1688.539	.460	1688.999	8.43	15.40	3.68	1692.68	.00	1.03	1.68	2.000	.000	.00	1	.0
	2.010	.2544				.0958	.19	.46	4.76	.37	.013	.00	.00	PIPE	
1163.383	1689.050	.476	1689.526	8.43	14.68	3.35	1692.87	.00	1.03	1.70	2.000	.000	.00	1	.0
	1.685	.2544				.0838	.14	.48	4.46	.37	.013	.00	.00	PIPE	
1165.068	1689.479	.493	1689.972	8.43	14.00	3.04	1693.02	.00	1.03	1.72	2.000	.000	.00	1	.0
	1.440	.2544				.0733	.11	.49	4.17	.37	.013	.00	.00	PIPE	
1166.507	1689.845	.509	1690.354	8.43	13.35	2.77	1693.12	.00	1.03	1.74	2.000	.000	.00	1	.0
	1.227	.2544				.0641	.08	.51	3.91	.37	.013	.00	.00	PIPE	
1167.734	1690.157	.527	1690.684	8.43	12.73	2.52	1693.20	.00	1.03	1.76	2.000	.000	.00	1	.0
	1.062	.2544				.0561	.06	.53	3.66	.37	.013	.00	.00	PIPE	
1168.797	1690.427	.545	1690.972	8.43	12.13	2.29	1693.26	.00	1.03	1.78	2.000	.000	.00	1	.0
	.920	.2544				.0490	.05	.55	3.42	.37	.013	.00	.00	PIPE	
1169.717	1690.661	.564	1691.225	8.43	11.57	2.08	1693.30	.00	1.03	1.80	2.000	.000	.00	1	.0
	.799	.2544				.0429	.03	.56	3.20	.37	.013	.00	.00	PIPE	

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 8

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1170.516 | 1690.865 | .584 | 1691.449 | 8.43 | 11.03 | 1.89 | 1693.34 | .00 | 1.03 | 1.82 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.700 | .2544 | | | | | .0376 | .03 | .58 | 3.00 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1171.216 | 1691.043 | .604 | 1691.647 | 8.43 | 10.52 | 1.72 | 1693.36 | .00 | 1.03 | 1.84 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.610 | .2544 | | | | | .0329 | .02 | .60 | 2.81 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1171.826 | 1691.198 | .625 | 1691.823 | 8.43 | 10.03 | 1.56 | 1693.38 | .00 | 1.03 | 1.85 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.527 | .2544 | | | | | .0288 | .02 | .63 | 2.62 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1172.354 | 1691.332 | .648 | 1691.980 | 8.43 | 9.56 | 1.42 | 1693.40 | .00 | 1.03 | 1.87 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.467 | .2544 | | | | | .0252 | .01 | .65 | 2.46 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1172.821 | 1691.451 | .670 | 1692.121 | 8.43 | 9.12 | 1.29 | 1693.41 | .00 | 1.03 | 1.89 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.402 | .2544 | | | | | .0221 | .01 | .67 | 2.30 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1173.223 | 1691.553 | .694 | 1692.247 | 8.43 | 8.69 | 1.17 | 1693.42 | .00 | 1.03 | 1.90 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.348 | .2544 | | | | | .0194 | .01 | .69 | 2.15 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1173.570 | 1691.641 | .719 | 1692.361 | 8.43 | 8.29 | 1.07 | 1693.43 | .00 | 1.03 | 1.92 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.299 | .2544 | | | | | .0170 | .01 | .72 | 2.01 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1173.869 | 1691.718 | .745 | 1692.463 | 8.43 | 7.90 | .97 | 1693.43 | .00 | 1.03 | 1.93 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.255 | .2544 | | | | | .0149 | .00 | .75 | 1.88 | .37 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1174.125 | 1691.782 | .772 | 1692.555 | 8.43 | 7.53 | .88 | 1693.44 | .00 | 1.03 | 1.95 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.220 | .2544 | | | | | .0131 | .00 | .77 | 1.75 | .37 | .013 | .00 | .00 | PIPE
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20-750 Meridian Upper Plateau

100 Year Storm

Line 8

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1174.345	1691.839	.799	1692.638	8.43	7.18	.80	1693.44	.00	1.03	1.96	2.000	.000	.00	1	.0
	.176	.2544				.0115	.00	.80	1.64	.37	.013	.00	.00	PIPE	
1174.521	1691.883	.829	1692.713	8.43	6.85	.73	1693.44	.00	1.03	1.97	2.000	.000	.00	1	.0
	.148	.2544				.0101	.00	.83	1.53	.37	.013	.00	.00	PIPE	
1174.670	1691.921	.859	1692.780	8.43	6.53	.66	1693.44	.00	1.03	1.98	2.000	.000	.00	1	.0
	.115	.2544				.0089	.00	.86	1.43	.37	.013	.00	.00	PIPE	
1174.785	1691.950	.891	1692.841	8.43	6.23	.60	1693.44	.00	1.03	1.99	2.000	.000	.00	1	.0
	.088	.2544				.0078	.00	.89	1.33	.37	.013	.00	.00	PIPE	
1174.873	1691.973	.924	1692.897	8.43	5.94	.55	1693.44	.00	1.03	1.99	2.000	.000	.00	1	.0
	.060	.2544				.0068	.00	.92	1.24	.37	.013	.00	.00	PIPE	
1174.932	1691.988	.959	1692.947	8.43	5.66	.50	1693.44	.00	1.03	2.00	2.000	.000	.00	1	.0
	.037	.2544				.0060	.00	.96	1.16	.37	.013	.00	.00	PIPE	
1174.970	1691.997	.995	1692.992	8.43	5.40	.45	1693.44	.00	1.03	2.00	2.000	.000	.00	1	.0
	.010	.2544				.0053	.00	1.00	1.08	.37	.013	.00	.00	PIPE	
1174.980	1692.000	1.035	1693.035	8.43	5.14	.41	1693.44	.00	1.03	2.00	2.000	.000	.00	1	.0

20-750 Meridian Upper Plateau

100 Year Storm

Line 8A

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1002.530 | 1650.100 | .477 | 1650.577 | 25.27 | 34.97 | 18.99 | 1669.57 | .00 | 1.62 | 2.19 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
15.440 | .5570 | | | | | .4504 | 6.95 | .48 | 10.74 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1017.970 | 1658.700 | .492 | 1659.192 | 25.27 | 33.42 | 17.35 | 1676.54 | 1.28 | 1.62 | 2.22 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.280 | .5570 | | | | | .4011 | 2.92 | 1.78 | 10.10 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1025.250 | 1662.755 | .503 | 1663.258 | 25.27 | 32.26 | 16.16 | 1679.42 | 1.21 | 1.62 | 2.24 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.249 | .5570 | | | | | .3566 | 2.59 | 1.71 | 9.62 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1032.499 | 1666.792 | .520 | 1667.312 | 25.27 | 30.76 | 14.69 | 1682.01 | 1.11 | 1.62 | 2.27 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.371 | .5570 | | | | | .3116 | 1.67 | 1.63 | 9.01 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1037.870 | 1669.784 | .538 | 1670.322 | 25.27 | 29.33 | 13.36 | 1683.68 | 1.03 | 1.62 | 2.30 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.202 | .5570 | | | | | .2723 | 1.14 | 1.56 | 8.45 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1042.072 | 1672.124 | .556 | 1672.680 | 25.27 | 27.97 | 12.14 | 1684.82 | .94 | 1.62 | 2.33 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.400 | .5570 | | | | | .2378 | .81 | 1.50 | 7.92 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1045.472 | 1674.018 | .575 | 1674.593 | 25.27 | 26.66 | 11.04 | 1685.63 | .87 | 1.62 | 2.36 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.820 | .5570 | | | | | .2078 | .59 | 1.44 | 7.42 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1048.292 | 1675.588 | .594 | 1676.182 | 25.27 | 25.42 | 10.04 | 1686.22 | .80 | 1.62 | 2.39 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.377 | .5570 | | | | | .1815 | .43 | 1.39 | 6.95 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1050.668 | 1676.912 | .614 | 1677.526 | 25.27 | 24.24 | 9.12 | 1686.65 | .74 | 1.62 | 2.42 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.029 | .5570 | | | | | .1586 | .32 | 1.35 | 6.51 | .46 | .013 | .00 | .00 | PIPE
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20-750 Meridian Upper Plateau

100 Year Storm

Line 8A

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1052.698	1678.042	.635	1678.677	25.27	23.11	8.29	1686.97	.68	1.62	2.45	3.000	.000	.00	1	.0
	1.750	.5570				.1386	.24	1.31	6.10	.46	.013	.00	.00	PIPE	
1054.447	1679.017	.657	1679.674	25.27	22.04	7.54	1687.21	.62	1.62	2.48	3.000	.000	.00	1	.0
	1.523	.5570				.1212	.18	1.28	5.71	.46	.013	.00	.00	PIPE	
1055.970	1679.865	.679	1680.544	25.27	21.01	6.85	1687.40	.57	1.62	2.51	3.000	.000	.00	1	.0
	1.328	.5570				.1060	.14	1.25	5.35	.46	.013	.00	.00	PIPE	
1057.298	1680.605	.703	1681.308	25.27	20.03	6.23	1687.54	.53	1.62	2.54	3.000	.000	.00	1	.0
	1.168	.5570				.0927	.11	1.23	5.01	.46	.013	.00	.00	PIPE	
1058.467	1681.255	.727	1681.983	25.27	19.10	5.67	1687.65	.49	1.62	2.57	3.000	.000	.00	1	.0
	1.030	.5570				.0810	.08	1.21	4.69	.46	.013	.00	.00	PIPE	
1059.497	1681.829	.752	1682.581	25.27	18.21	5.15	1687.73	.45	1.62	2.60	3.000	.000	.00	1	.0
	.910	.5570				.0709	.06	1.20	4.39	.46	.013	.00	.00	PIPE	
1060.406	1682.336	.778	1683.114	25.27	17.36	4.68	1687.80	.41	1.62	2.63	3.000	.000	.00	1	.0
	.807	.5570				.0620	.05	1.19	4.11	.46	.013	.00	.00	PIPE	
1061.214	1682.785	.804	1683.589	25.27	16.56	4.26	1687.85	.38	1.62	2.66	3.000	.000	.00	1	.0
	.714	.5570				.0542	.04	1.18	3.85	.46	.013	.00	.00	PIPE	
1061.928	1683.183	.832	1684.015	25.27	15.79	3.87	1687.88	.35	1.62	2.69	3.000	.000	.00	1	.0
	.633	.5570				.0474	.03	1.18	3.60	.46	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time:10: 0:15

20-750 Meridian Upper Plateau

100 Year Storm

Line 8A

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1062.561 | 1683.536 | .861 | 1684.397 | 25.27 | 15.05 | 3.52 | 1687.91 | .32 | 1.62 | 2.71 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.562 | .5570 | | | | | .0415 | .02 | 1.18 | 3.37 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1063.123 | 1683.849 | .891 | 1684.740 | 25.27 | 14.35 | 3.20 | 1687.94 | .29 | 1.62 | 2.74 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.499 | .5570 | | | | | .0364 | .02 | 1.18 | 3.16 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1063.622 | 1684.127 | .922 | 1685.049 | 25.27 | 13.68 | 2.91 | 1687.96 | .27 | 1.62 | 2.77 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.440 | .5570 | | | | | .0318 | .01 | 1.19 | 2.95 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1064.063 | 1684.372 | .955 | 1685.327 | 25.27 | 13.05 | 2.64 | 1687.97 | .25 | 1.62 | 2.80 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.390 | .5570 | | | | | .0279 | .01 | 1.20 | 2.76 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1064.453 | 1684.589 | .989 | 1685.578 | 25.27 | 12.44 | 2.40 | 1687.98 | .23 | 1.62 | 2.82 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.344 | .5570 | | | | | .0244 | .01 | 1.21 | 2.58 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1064.797 | 1684.781 | 1.024 | 1685.805 | 25.27 | 11.86 | 2.18 | 1687.99 | .21 | 1.62 | 2.85 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.303 | .5570 | | | | | .0214 | .01 | 1.23 | 2.42 | .46 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1065.100 | 1684.950 | 1.060 | 1686.010 | 25.27 | 11.31 | 1.99 | 1688.00 | .00 | 1.62 | 2.87 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
21.555 | .0200 | | | | | .0200 | .43 | 1.06 | 2.26 | 1.06 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1086.655 | 1685.381 | 1.060 | 1686.441 | 25.27 | 11.31 | 1.99 | 1688.43 | .00 | 1.62 | 2.87 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
115.721 | .0200 | | | | | .0192 | 2.22 | 1.06 | 2.26 | 1.06 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1202.376 | 1687.696 | 1.085 | 1688.781 | 25.27 | 10.95 | 1.86 | 1690.64 | .00 | 1.62 | 2.88 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
46.230 | .0200 | | | | | .0172 | .79 | 1.09 | 2.16 | 1.06 | .013 | .00 | .00 | PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 8A

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1248.606	1688.621	1.124	1689.745	25.27	10.44	1.69	1691.44	.00	1.62	2.90	3.000	.000	.00	1	.0
	22.914	.0200				.0151	.35	1.12	2.02	1.06	.013	.00	.00	PIPE	
1271.521	1689.079	1.165	1690.244	25.27	9.96	1.54	1691.78	.00	1.62	2.92	3.000	.000	.00	1	.0
	14.456	.0200				.0132	.19	1.17	1.88	1.06	.013	.00	.00	PIPE	
1285.976	1689.368	1.207	1690.575	25.27	9.49	1.40	1691.98	.00	1.62	2.94	3.000	.000	.00	1	.0
	9.915	.0200				.0116	.12	1.21	1.76	1.06	.013	.00	.00	PIPE	
1295.891	1689.567	1.251	1690.818	25.27	9.05	1.27	1692.09	.00	1.62	2.96	3.000	.000	.00	1	.0
	7.102	.0200				.0102	.07	1.25	1.64	1.06	.013	.00	.00	PIPE	
1302.993	1689.709	1.297	1691.006	25.27	8.63	1.16	1692.16	.00	1.62	2.97	3.000	.000	.00	1	.0
	5.173	.0200				.0090	.05	1.30	1.53	1.06	.013	.00	.00	PIPE	
1308.167	1689.812	1.345	1691.157	25.27	8.23	1.05	1692.21	.00	1.62	2.98	3.000	.000	.00	1	.0
	3.758	.0200				.0079	.03	1.35	1.43	1.06	.013	.00	.00	PIPE	
1311.925	1689.887	1.395	1691.283	25.27	7.85	.96	1692.24	.00	1.62	2.99	3.000	.000	.00	1	.0
	2.668	.0200				.0069	.02	1.40	1.33	1.06	.013	.00	.00	PIPE	
1314.593	1689.941	1.447	1691.388	25.27	7.48	.87	1692.26	.00	1.62	3.00	3.000	.000	.00	1	.0
	1.725	.0200				.0061	.01	1.45	1.24	1.06	.013	.00	.00	PIPE	
1316.319	1689.975	1.502	1691.477	25.27	7.13	.79	1692.27	.00	1.62	3.00	3.000	.000	.00	1	.0
	.944	.0200				.0054	.01	1.50	1.16	1.06	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 8A

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El.| Elev | Depth | Width | Dia.-FT|or I.D.| ZL |Prs/Pip
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-
L/Elem |Ch Slope | | | | | | | | | | | | | | | |
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
| | | | | | | | | | | | | | | | |
1317.263 1689.994 1.560 1691.554 25.27 6.80 .72 1692.27 .00 1.62 3.00 3.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-
.287 .0200 | | | | | | | | | | | | | | | |
|.0047 .00 1.56 1.08 1.06 .013 .00 .00 PIPE
| | | | | | | | | | | | | | | | |
1317.550 1690.000 1.622 1691.622 25.27 6.48 .65 1692.27 .00 1.62 2.99 3.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-

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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:51:19

20-750 Meridian Upper Plateau

100 Year Storm

Line 14

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	1711.500	1.222	1712.722	58.67	9.19	1.31	1714.03	.00	1.63	6.44	1.500	4.000	1.00	0 .0
	6.789	.0050				.0081	.06	1.22	1.63	1.39	.013	.00	1.00	TRAP
1006.789	1711.534	1.210	1712.744	58.67	9.31	1.35	1714.09	.00	1.63	6.42	1.500	4.000	1.00	0 .0
	23.015	.0050				.0088	.20	1.21	1.66	1.39	.013	.00	1.00	TRAP
1029.805	1711.649	1.164	1712.813	58.67	9.77	1.48	1714.29	.00	1.63	6.33	1.500	4.000	1.00	0 .0
	20.176	.0050				.0101	.20	1.16	1.77	1.39	.013	.00	1.00	TRAP
1049.981	1711.750	1.119	1712.869	58.67	10.24	1.63	1714.50	.00	1.63	6.24	1.500	4.000	1.00	0 .0
	18.138	.0050				.0116	.21	1.12	1.88	1.39	.013	.00	1.00	TRAP
1068.119	1711.841	1.076	1712.917	58.67	10.74	1.79	1714.71	.00	1.63	6.15	1.500	4.000	1.00	0 .0
	16.572	.0050				.0133	.22	1.08	2.01	1.39	.013	.00	1.00	TRAP
1084.691	1711.923	1.034	1712.958	58.67	11.27	1.97	1714.93	.00	1.63	6.07	1.500	4.000	1.00	0 .0
	15.309	.0050				.0152	.23	1.03	2.14	1.39	.013	.00	1.00	TRAP
1100.000	1712.000	.994	1712.994	58.67	11.82	2.17	1715.16	.00	1.63	5.99	1.500	4.000	1.00	0 .0
	114.286	.0166				.0153	1.75	.99	2.29	.99	.013	.00	1.00	TRAP
1214.286	1713.897	1.031	1714.928	58.67	11.31	1.99	1716.92	.00	1.63	6.06	1.500	4.000	1.00	0 .0
	44.413	.0166				.0135	.60	1.03	2.16	.99	.013	.00	1.00	TRAP
1258.698	1714.634	1.072	1715.707	58.67	10.79	1.81	1717.51	.00	1.63	6.14	1.500	4.000	1.00	0 .0
	25.026	.0166				.0117	.29	1.07	2.02	.99	.013	.00	1.00	TRAP

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:51:19

20-750 Meridian Upper Plateau

100 Year Storm

Line 14

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1283.724 | 1715.050 | 1.115 | 1716.165 | 58.67 | 10.29 | 1.64 | 1717.81 | .00 | 1.63 | 6.23 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
16.550 | .0166 | | | | | .0103 | .17 | 1.12 | 1.89 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1300.275 | 1715.324 | 1.160 | 1716.484 | 58.67 | 9.81 | 1.49 | 1717.98 | .00 | 1.63 | 6.32 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
11.762 | .0166 | | | | | .0090 | .11 | 1.16 | 1.78 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1312.037 | 1715.520 | 1.205 | 1716.725 | 58.67 | 9.35 | 1.36 | 1718.08 | .00 | 1.63 | 6.41 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.662 | .0166 | | | | | .0078 | .07 | 1.21 | 1.67 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1320.699 | 1715.663 | 1.253 | 1716.916 | 58.67 | 8.92 | 1.23 | 1718.15 | .00 | 1.63 | 6.51 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.475 | .0166 | | | | | .0068 | .04 | 1.25 | 1.56 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1327.174 | 1715.771 | 1.302 | 1717.073 | 58.67 | 8.50 | 1.12 | 1718.19 | .00 | 1.63 | 6.60 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.837 | .0166 | | | | | .0060 | .03 | 1.30 | 1.47 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1332.011 | 1715.851 | 1.352 | 1717.204 | 58.67 | 8.10 | 1.02 | 1718.22 | .00 | 1.63 | 6.70 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.557 | .0166 | | | | | .0052 | .02 | 1.35 | 1.37 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1335.568 | 1715.910 | 1.405 | 1717.315 | 58.67 | 7.73 | .93 | 1718.24 | .00 | 1.63 | 6.81 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.520 | .0166 | | | | | .0046 | .01 | 1.40 | 1.29 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1338.088 | 1715.952 | 1.459 | 1717.411 | 58.67 | 7.37 | .84 | 1718.25 | .00 | 1.63 | 6.92 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.659 | .0166 | | | | | .0040 | .01 | 1.46 | 1.21 | .99 | .013 | .00 | 1.00 | TRAP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1339.748 | 1715.980 | 1.514 | 1717.494 | 58.67 | 7.03 | .77 | 1718.26 | .00 | 1.63 | 7.03 | 1.500 | 4.000 | 1.00 | 0 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.928 | .0166 | | | | | .0035 | .00 | 1.51 | 1.14 | .99 | .013 | .00 | 1.00 | TRAP
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20-750 Meridian Upper Plateau

100 Year Storm

Line 14

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
1340.675	1715.995	1.572	1717.567	58.67	6.70	.70	1718.26	.00	1.63	7.14	1.500	4.000	1.00	0 .0
	.295					.0031	.00	1.57	1.07	.99	.013	.00	1.00	TRAP
1340.970	1716.000	1.632	1717.632	58.67	6.38	.63	1718.26	1.19	1.63	7.26	1.500	4.000	1.00	0 .0
JUNCT STR	.0150					.0017	.02	2.83	1.00		.013	.00	1.00	TRAP
1354.330	1716.200	1.959	1718.159	33.56	2.87	.13	1718.29	.00	1.17	7.92	1.500	4.000	1.00	0 .0
TRANS STR	.5000							1.96	.42		.013	.00	1.00	TRAP
1354.350	1716.210	1.881	1718.091	33.56	7.19	.80	1718.89	.00	1.88	2.90	3.000	.000	.00	1 .0
	1.116					.0046	.01	1.88	1.00	.00	.013	.00	.00	PIPE
1355.466	1716.207	1.975	1718.182	33.56	6.80	.72	1718.90	.00	1.88	2.85	3.000	.000	.00	1 .0
	3.510					.0040	.01	1.97	.91	.00	.013	.00	.00	PIPE
1358.975	1716.196	2.074	1718.270	33.56	6.44	.64	1718.91	.00	1.88	2.77	3.000	.000	.00	1 .0
	6.079					.0035	.02	2.07	.83	.00	.013	.00	.00	PIPE
1365.055	1716.179	2.177	1718.356	33.56	6.11	.58	1718.94	.00	1.88	2.68	3.000	.000	.00	1 .0
	8.795					.0031	.03	2.18	.75	.00	.013	.00	.00	PIPE
1373.850	1716.153	2.286	1718.439	33.56	5.81	.52	1718.96	.00	1.88	2.55	3.000	.000	.00	1 .0
	11.625					.0028	.03	2.29	.68	.00	.013	.00	.00	PIPE
1385.475	1716.119	2.401	1718.519	33.56	5.53	.48	1718.99	.00	1.88	2.40	3.000	.000	.00	1 .0
	14.533					.0025	.04	2.40	.61	.00	.013	.00	.00	PIPE

20-750 Meridian Upper Plateau

100 Year Storm

Line 14

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1400.008 | 1716.076 | 2.521 | 1718.597 | 33.56 | 5.29 | .44 | 1719.03 | .00 | 1.88 | 2.20 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
17.481 | -.0029 | | | | | .0023 | .04 | 2.52 | .55 | .00 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1417.488 | 1716.025 | 2.647 | 1718.672 | 33.56 | 5.08 | .40 | 1719.07 | .00 | 1.88 | 1.93 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.522 | -.0029 | | | | | .0022 | .02 | 2.65 | .48 | .00 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1426.010 | 1716.000 | 2.704 | 1718.703 | 33.56 | 5.00 | .39 | 1719.09 | .00 | 1.88 | 1.79 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR | .1923 | | | | | .0018 | .01 | 2.70 | .46 | | | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1430.690 | 1716.900 | 2.223 | 1719.124 | 22.01 | 3.92 | .24 | 1719.36 | .00 | 1.51 | 2.63 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
11.670 | .0078 | | | | | .0014 | .02 | 2.22 | .47 | 1.27 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1442.360 | 1716.991 | 2.125 | 1719.116 | 22.01 | 4.11 | .26 | 1719.38 | .00 | 1.51 | 2.73 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
10.258 | .0078 | | | | | .0016 | .02 | 2.13 | .52 | 1.27 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1452.618 | 1717.071 | 2.035 | 1719.107 | 22.01 | 4.31 | .29 | 1719.40 | .00 | 1.51 | 2.80 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
9.161 | .0078 | | | | | .0018 | .02 | 2.04 | .56 | 1.27 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1461.779 | 1717.143 | 1.951 | 1719.094 | 22.01 | 4.52 | .32 | 1719.41 | .00 | 1.51 | 2.86 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.984 | .0078 | | | | | .0020 | .02 | 1.95 | .61 | 1.27 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1469.763 | 1717.205 | 1.873 | 1719.078 | 22.01 | 4.74 | .35 | 1719.43 | .00 | 1.51 | 2.91 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.062 | .0078 | | | | | .0023 | .02 | 1.87 | .66 | 1.27 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1476.825 | 1717.260 | 1.799 | 1719.060 | 22.01 | 4.97 | .38 | 1719.44 | .00 | 1.51 | 2.94 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.031 | .0078 | | | | | .0026 | .02 | 1.80 | .71 | 1.27 | .013 | .00 | .00 | PIPE
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WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 14

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS)| (FPS)| Grd.El.| Elev  |Depth  |Width  |Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1482.856 1717.308 1.729 1719.037 22.01 5.22 .42 1719.46 .00 1.51 2.96 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
2.887 .0078 | .0029 .01 1.73 .77 1.27 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1485.743 1717.330 1.663 1718.993 22.01 5.47 .46 1719.46 .00 1.51 2.98 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
HYDRAULIC JUMP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1485.743 1717.330 1.349 1718.679 22.01 7.14 .79 1719.47 .00 1.51 2.98 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
4.350 .0078 | .0063 .03 1.35 1.24 1.27 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1490.093 1717.364 1.349 1718.713 22.01 7.14 .79 1719.50 .00 1.51 2.98 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
11.481 .0078 | .0059 .07 1.35 1.24 1.27 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1501.573 1717.454 1.399 1718.853 22.01 6.81 .72 1719.57 .00 1.51 2.99 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
4.726 .0078 | .0052 .02 1.40 1.15 1.27 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1506.299 1717.491 1.452 1718.943 22.01 6.49 .65 1719.60 .00 1.51 3.00 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1.181 .0078 | .0046 .01 1.45 1.08 1.27 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1507.480 1717.500 1.509 1719.009 22.01 6.18 .59 1719.60 .00 1.51 3.00 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
JUNCT STR .0214 | .0030 .01 1.51 1.00 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1512.150 1717.600 2.426 1720.026 9.34 2.97 .14 1720.16 .00 1.09 .00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
69.500 .0078 | .0017 .12 2.43 .00 .96 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1581.650 1718.145 2.000 1720.145 9.34 2.97 .14 1720.28 .00 1.09 .00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
27.357 .0078 | .0016 .04 2.00 .00 .96 .013 .00 .00 PIPE
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WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 14

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
1609.007 1718.359 1.814 1720.173 9.34 3.12 .15 1720.32 .00 1.09 1.16 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
14.756 .0078 .0015 .02 1.81 .34 .96 .013 .00 .00 PIPE
1623.762 1718.475 1.706 1720.181 9.34 3.27 .17 1720.35 .00 1.09 1.42 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
11.574 .0078 .0017 .02 1.71 .41 .96 .013 .00 .00 PIPE
1635.336 1718.566 1.618 1720.184 9.34 3.43 .18 1720.37 .00 1.09 1.57 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
9.954 .0078 .0018 .02 1.62 .46 .96 .013 .00 .00 PIPE
1645.291 1718.644 1.540 1720.184 9.34 3.60 .20 1720.38 .00 1.09 1.68 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.606 .0078 .0020 .02 1.54 .51 .96 .013 .00 .00 PIPE
1653.896 1718.711 1.470 1720.181 9.34 3.77 .22 1720.40 .00 1.09 1.77 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.532 .0078 .0023 .02 1.47 .56 .96 .013 .00 .00 PIPE
1661.428 1718.770 1.406 1720.176 9.34 3.96 .24 1720.42 .00 1.09 1.83 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.747 .0078 .0025 .02 1.41 .61 .96 .013 .00 .00 PIPE
1668.175 1718.823 1.346 1720.169 9.34 4.15 .27 1720.44 .00 1.09 1.88 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.680 .0078 .0029 .02 1.35 .67 .96 .013 .00 .00 PIPE
1673.855 1718.868 1.291 1720.159 9.34 4.35 .29 1720.45 .00 1.09 1.91 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.895 .0078 .0032 .02 1.29 .72 .96 .013 .00 .00 PIPE
1678.751 1718.906 1.239 1720.145 9.34 4.57 .32 1720.47 .00 1.09 1.94 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.192 .0078 .0036 .00 1.24 .78 .96 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1000.000	1632.500	1.422	1633.922	51.42	15.57	3.77	1637.69	.00	2.33	3.00	3.000	.000	.00	1	.0
154.250	.0285					.0285	4.40	1.42	2.61	1.42	.013	.00	.00	PIPE	
1154.250	1636.900	1.423	1638.323	51.42	15.57	3.77	1642.09	.38	2.33	3.00	3.000	.000	.00	1	.0
46.140	.0286					.0285	1.31	1.80	2.61	1.42	.013	.00	.00	PIPE	
1200.390	1638.220	1.423	1639.643	51.42	15.56	3.76	1643.40	.00	2.33	3.00	3.000	.000	.00	1	.0
JUNCT STR	.0200					.0287	.14	1.42	2.61		.013	.00	.00	PIPE	
1205.400	1638.320	1.416	1639.736	51.42	15.66	3.81	1643.55	.00	2.33	3.00	3.000	.000	.00	1	.0
112.240	.0290					.0290	3.25	1.42	2.64	1.42	.013	.00	.00	PIPE	
1317.640	1641.571	1.416	1642.987	51.42	15.66	3.81	1646.80	.00	2.33	3.00	3.000	.000	.00	1	.0
162.210	.0290					.0291	4.71	1.42	2.64	1.42	.013	.00	.00	PIPE	
1479.850	1646.270	1.414	1647.684	51.42	15.70	3.83	1651.51	.38	2.33	3.00	3.000	.000	.00	1	.0
94.250	.0292					.0291	2.74	1.80	2.65	1.41	.013	.00	.00	PIPE	
1574.100	1649.020	1.415	1650.435	51.42	15.68	3.82	1654.25	.00	2.33	3.00	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0293	.14	1.42	2.64		.013	.00	.00	PIPE	
1578.770	1649.120	1.409	1650.529	51.42	15.76	3.86	1654.39	.00	2.33	2.99	3.000	.000	.00	1	.0
96.820	.0300					.0288	2.79	1.41	2.66	1.40	.013	.00	.00	PIPE	
1675.590	1652.020	1.428	1653.448	51.42	15.50	3.73	1657.18	.02	2.33	3.00	3.000	.000	.00	1	.0
273.916	.0282					.0282	7.72	1.44	2.60	1.43	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1949.506	1659.737	1.428	1661.164	51.42	15.50	3.73	1664.90	.02	2.33	3.00	3.000	.000	.00	1	.0
169.074	.0282					.0273	4.62	1.44	2.60	1.43	.013	.00	.00	PIPE	
2118.580	1664.500	1.454	1665.954	51.42	15.14	3.56	1669.51	.00	2.33	3.00	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0272	.13	1.45	2.51		.013	.00	.00	PIPE	
2123.260	1664.600	1.423	1666.023	50.91	15.42	3.69	1669.71	.00	2.32	3.00	3.000	.000	.00	1	.0
77.300	.0279					.0279	2.16	1.42	2.59	1.42	.013	.00	.00	PIPE	
2200.560	1666.760	1.423	1668.183	50.91	15.41	3.69	1671.87	.00	2.32	3.00	3.000	.000	.00	1	.0
269.863	.0279					.0279	7.53	1.42	2.58	1.42	.013	.00	.00	PIPE	
2470.423	1674.287	1.423	1675.711	50.91	15.41	3.69	1679.40	.00	2.32	3.00	3.000	.000	.00	1	.0
149.587	.0279					.0279	4.18	1.42	2.58	1.42	.013	.00	.00	PIPE	
2620.010	1678.460	1.423	1679.883	50.91	15.41	3.69	1683.57	.37	2.32	3.00	3.000	.000	.00	1	.0
71.800	.0279					.0280	2.01	1.79	2.59	1.42	.013	.00	.00	PIPE	
2691.810	1680.460	1.421	1681.881	50.91	15.44	3.70	1685.58	.00	2.32	3.00	3.000	.000	.00	1	.0
3.565	.0281					.0281	.10	1.42	2.59	1.42	.013	.00	.00	PIPE	
2695.375	1680.560	1.421	1681.981	50.91	15.44	3.70	1685.68	.00	2.32	3.00	3.000	.000	.00	1	.0
168.265	.0281					.0278	4.68	1.42	2.59	1.42	.013	.00	.00	PIPE	
2863.640	1685.280	1.428	1686.708	50.91	15.35	3.66	1690.36	.03	2.32	3.00	3.000	.000	.00	1	.0
109.134	.0278					.0273	2.98	1.46	2.57	1.42	.013	.00	.00	PIPE	



WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS)| (FPS)| Grd.El.| Elev  |Depth  |Width  |Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
L/Elem  |Ch Slope|      |      |      |      | SF Ave | HF   |SE Dpth|Froude N|Norm Dp | "N"  | X-Fall| ZR  |Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3200.771 | 1694.653 | 1.944 | 1696.597 | 50.91 | 10.50 | 1.71 | 1698.31 | .00 | 2.32 | 2.87 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
4.134 | .0273 | | | | | .0097 | .04 | 1.94 | 1.42 | 1.43 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3204.905 | 1694.766 | 2.027 | 1696.793 | 50.91 | 10.01 | 1.56 | 1698.35 | .00 | 2.32 | 2.81 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
2.763 | .0273 | | | | | .0086 | .02 | 2.03 | 1.31 | 1.43 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3207.669 | 1694.841 | 2.117 | 1696.958 | 50.91 | 9.55 | 1.42 | 1698.37 | .00 | 2.32 | 2.73 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
1.618 | .0273 | | | | | .0077 | .01 | 2.12 | 1.20 | 1.43 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3209.287 | 1694.885 | 2.214 | 1697.099 | 50.91 | 9.10 | 1.29 | 1698.39 | .00 | 2.32 | 2.64 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
.543 | .0273 | | | | | .0069 | .00 | 2.21 | 1.10 | 1.43 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3209.830 | 1694.900 | 2.321 | 1697.221 | 50.91 | 8.68 | 1.17 | 1698.39 | .00 | 2.32 | 2.51 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
JUNCT STR | .0214 | | | | | .0050 | .02 | 2.32 | 1.00 | | | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3214.510 | 1695.000 | 3.494 | 1698.494 | 13.18 | 4.20 | .27 | 1698.77 | .00 | 1.31 | .00 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
30.080 | .0199 | | | | | .0034 | .10 | 3.49 | .00 | .90 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3244.590 | 1695.600 | 2.996 | 1698.596 | 13.18 | 4.20 | .27 | 1698.87 | .00 | 1.31 | .00 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
JUNCT STR | .0214 | | | | | .0029 | .01 | 3.00 | .00 | | | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3249.260 | 1695.700 | 3.054 | 1698.754 | 11.20 | 3.57 | .20 | 1698.95 | .00 | 1.20 | .00 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
53.988 | .0220 | | | | | .0024 | .13 | 3.05 | .00 | .80 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
3303.248 | 1696.886 | 2.000 | 1698.886 | 11.20 | 3.57 | .20 | 1699.08 | .00 | 1.20 | .00 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----
8.383 | .0220 | | | | | .0023 | .02 | 2.00 | .00 | .80 | .013 | .00 | .00 | PIPE

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WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS)| (FPS)| Head   | Grd.El.| Elev   |Depth  | Width  |Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3311.631 1697.071 1.814 1698.885 11.20 3.74 .22 1699.10 .00 1.20 1.16 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.419 .0220 | .0022 .01 1.81 .41 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3314.050 1697.124 1.756 1698.880 11.20 3.83 .23 1699.11 .00 1.20 1.31 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3314.050 1697.124 .796 1697.920 11.20 9.61 1.43 1699.35 .00 1.20 1.96 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
367.290 .0220 | .0220 8.07 .80 2.19 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3681.340 1705.196 .796 1705.992 11.20 9.61 1.43 1707.43 .00 1.20 1.96 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
75.681 .0220 | .0207 1.56 .80 2.19 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3757.021 1706.859 .824 1707.684 11.20 9.17 1.31 1708.99 .00 1.20 1.97 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
23.266 .0220 | .0182 .42 .82 2.05 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3780.288 1707.371 .854 1708.225 11.20 8.74 1.19 1709.41 .00 1.20 1.98 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
12.611 .0220 | .0160 .20 .85 1.92 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3792.898 1707.648 .886 1708.534 11.20 8.34 1.08 1709.61 .00 1.20 1.99 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.189 .0220 | .0140 .11 .89 1.79 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3801.088 1707.828 .919 1708.747 11.20 7.95 .98 1709.73 .00 1.20 1.99 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.722 .0220 | .0123 .07 .92 1.67 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3806.810 1707.954 .953 1708.907 11.20 7.58 .89 1709.80 .00 1.20 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.051 .0220 | .0108 .04 .95 1.55 .80 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
3810.861 1708.043 .989 1709.032 11.20 7.23 .81 1709.84 .00 1.20 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.874 .0220 | | | | | .0096 .03 .99 1.45 .80 .013 .00 .00 PIPE
3813.734 1708.106 1.027 1709.133 11.20 6.89 .74 1709.87 .00 1.20 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.992 .0220 | | | | | .0084 .02 1.03 1.35 .80 .013 .00 .00 PIPE
3815.726 1708.150 1.067 1709.217 11.20 6.57 .67 1709.89 .00 1.20 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.368 .0220 | | | | | .0074 .01 1.07 1.25 .80 .013 .00 .00 PIPE
3817.095 1708.180 1.108 1709.288 11.20 6.26 .61 1709.90 .00 1.20 1.99 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.738 .0220 | | | | | .0066 .00 1.11 1.16 .80 .013 .00 .00 PIPE
3817.833 1708.196 1.152 1709.348 11.20 5.97 .55 1709.90 .00 1.20 1.98 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.188 .0220 | | | | | .0058 .00 1.15 1.08 .80 .013 .00 .00 PIPE
3818.020 1708.200 1.200 1709.400 11.20 5.69 .50 1709.90 .00 1.20 1.96 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR .0214 | | | | | .0041 .02 1.20 1.00 .014 .00 .00 PIPE
3822.700 1708.300 1.562 1709.862 8.85 3.36 .18 1710.04 .00 1.06 1.65 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.691 .0100 | | | | | .0018 .01 1.56 .47 .87 .013 .00 .00 PIPE
3829.391 1708.367 1.489 1709.856 8.85 3.53 .19 1710.05 .00 1.06 1.74 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.694 .0100 | | | | | .0020 .01 1.49 .52 .87 .013 .00 .00 PIPE
3835.085 1708.424 1.424 1709.848 8.85 3.70 .21 1710.06 .00 1.06 1.81 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.105 .0100 | | | | | .0022 .01 1.42 .57 .87 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
3840.191 1708.475 1.363 1709.838 8.85 3.88 .23 1710.07 .00 1.06 1.86 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.342 .0100 | | | | | .0025 .01 1.36 .62 .87 .013 .00 .00 PIPE
3844.533 1708.518 1.307 1709.826 8.85 4.07 .26 1710.08 .00 1.06 1.90 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.640 .0100 | | | | | .0028 .00 1.31 .67 .87 .013 .00 .00 PIPE
3846.173 1708.535 1.254 1709.789 8.85 4.27 .28 1710.07 .00 1.06 1.93 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP
3846.173 1708.535 .869 1709.404 8.85 6.76 .71 1710.11 .00 1.06 1.98 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
302.146 .0100 | | | | | .0100 3.02 .87 1.47 .87 .013 .00 .00 PIPE
4148.319 1711.557 .869 1712.426 8.85 6.76 .71 1713.14 .00 1.06 1.98 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
56.561 .0100 | | | | | .0103 .58 .87 1.47 .87 .013 .00 .00 PIPE
4204.879 1712.123 .855 1712.978 8.85 6.90 .74 1713.72 .00 1.06 1.98 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
33.554 .0100 | | | | | .0113 .38 .86 1.51 .87 .013 .00 .00 PIPE
4238.433 1712.459 .825 1713.284 8.85 7.24 .81 1714.10 .00 1.06 1.97 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
18.137 .0100 | | | | | .0129 .23 .83 1.62 .87 .013 .00 .00 PIPE
4256.570 1712.640 .796 1713.436 8.85 7.59 .90 1714.33 .06 1.06 1.96 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
29.223 .0137 | | | | | .0137 .40 .85 1.73 .80 .013 .00 .00 PIPE
4285.793 1713.041 .796 1713.837 8.85 7.59 .90 1714.73 .06 1.06 1.96 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
64.266 .0137 | | | | | .0133 .85 .85 1.73 .80 .013 .00 .00 PIPE

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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:12: 5

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
4350.059	1713.923	.810	1714.733	8.85	7.41	.85	1715.58	.06	1.06	1.96	2.000	.000	.00	1	.0
28.235	.0137					.0120	.34	.87	1.67	.80	.013	.00	.00	PIPE	
4378.294	1714.310	.840	1715.150	8.85	7.06	.77	1715.93	.05	1.06	1.97	2.000	.000	.00	1	.0
12.533	.0137					.0106	.13	.89	1.56	.80	.013	.00	.00	PIPE	
4390.827	1714.482	.871	1715.353	8.85	6.73	.70	1716.06	.05	1.06	1.98	2.000	.000	.00	1	.0
7.113	.0137					.0093	.07	.92	1.46	.80	.013	.00	.00	PIPE	
4397.939	1714.580	.904	1715.484	8.85	6.42	.64	1716.12	.00	1.06	1.99	2.000	.000	.00	1	.0
4.581	.0102					.0085	.04	.90	1.36	.86	.013	.00	.00	PIPE	
4402.521	1714.627	.913	1715.540	8.85	6.33	.62	1716.16	.00	1.06	1.99	2.000	.000	.00	1	.0
9.520	.0102					.0079	.07	.91	1.33	.86	.013	.00	.00	PIPE	
4412.041	1714.724	.947	1715.671	8.85	6.04	.57	1716.24	.00	1.06	2.00	2.000	.000	.00	1	.0
4.652	.0102					.0069	.03	.95	1.24	.86	.013	.00	.00	PIPE	
4416.692	1714.772	.983	1715.755	8.85	5.75	.51	1716.27	.00	1.06	2.00	2.000	.000	.00	1	.0
2.113	.0102					.0061	.01	.98	1.16	.86	.013	.00	.00	PIPE	
4418.805	1714.793	1.021	1715.815	8.85	5.49	.47	1716.28	.00	1.06	2.00	2.000	.000	.00	1	.0
.645	.0102					.0054	.00	1.02	1.08	.86	.013	.00	.00	PIPE	
4419.450	1714.800	1.061	1715.861	8.85	5.23	.42	1716.29	.00	1.06	2.00	2.000	.000	.00	1	.0
JUNCT STR	.0214					.0028	.01	1.06	1.00		.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4424.120 1714.900 1.468 1716.368 4.81 1.95 .06 1716.43 .00 .77 1.77 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
17.638 .0039 | .0006 .01 1.47 .29 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4441.758 1714.969 1.404 1716.373 4.81 2.04 .06 1716.44 .00 .77 1.83 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
16.229 .0039 | .0007 .01 1.40 .32 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4457.986 1715.033 1.345 1716.378 4.81 2.14 .07 1716.45 .00 .77 1.88 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
15.188 .0039 | .0008 .01 1.35 .34 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4473.174 1715.092 1.290 1716.382 4.81 2.24 .08 1716.46 .00 .77 1.91 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
14.452 .0039 | .0009 .01 1.29 .37 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4487.625 1715.149 1.238 1716.387 4.81 2.35 .09 1716.47 .00 .77 1.94 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
13.708 .0039 | .0010 .01 1.24 .40 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4501.334 1715.203 1.189 1716.392 4.81 2.47 .09 1716.49 .00 .77 1.96 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
12.952 .0039 | .0011 .01 1.19 .44 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4514.286 1715.253 1.143 1716.396 4.81 2.59 .10 1716.50 .00 .77 1.98 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
12.177 .0039 | .0012 .02 1.14 .47 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4526.463 1715.301 1.100 1716.401 4.81 2.72 .11 1716.52 .00 .77 1.99 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
12.163 .0039 | .0014 .02 1.10 .51 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4538.625 1715.349 1.058 1716.407 4.81 2.85 .13 1716.53 .00 .77 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
11.363 .0039 | .0016 .02 1.06 .55 .80 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS)| (FPS)| Grd.El.| Elev  |Depth  |Width  |Dia.-FT|or I.D.| ZL  |Prs/Pip
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
L/Elem |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
4549.988 1715.393 1.019 1716.412 4.81 2.99 .14 1716.55 .00 .77 2.00 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
10.979 .0039 .0018 .02 1.02 .59 .80 .013 .00 .00 PIPE
4560.967 1715.436 .982 1716.418 4.81 3.13 .15 1716.57 .00 .77 2.00 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
11.147 .0039 .0021 .02 .98 .63 .80 .013 .00 .00 PIPE
4572.115 1715.480 .946 1716.426 4.81 3.29 .17 1716.59 .00 .77 2.00 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
10.897 .0039 .0023 .03 .95 .68 .80 .013 .00 .00 PIPE
4583.012 1715.522 .912 1716.434 4.81 3.45 .18 1716.62 .00 .77 1.99 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
11.544 .0039 .0027 .03 .91 .73 .80 .013 .00 .00 PIPE
4594.556 1715.568 .879 1716.447 4.81 3.62 .20 1716.65 .00 .77 1.99 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
11.973 .0039 .0030 .04 .88 .78 .80 .013 .00 .00 PIPE
4606.529 1715.614 .848 1716.463 4.81 3.79 .22 1716.69 .00 .77 1.98 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
16.155 .0039 .0034 .06 .85 .83 .80 .013 .00 .00 PIPE
4622.684 1715.678 .818 1716.496 4.81 3.98 .25 1716.74 .00 .77 1.97 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
15.781 .0039 .0038 .06 .82 .89 .80 .013 .00 .00 PIPE
4638.465 1715.739 .804 1716.543 4.81 4.07 .26 1716.80 .00 .77 1.96 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|
869.680 .0039 .0039 3.39 .80 .93 .80 .013 .00 .00 PIPE
5508.145 1719.145 .804 1719.948 4.81 4.07 .26 1720.21 .00 .77 1.96 2.000 .000 .00 1 .0
|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|-|

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HYDRAULIC JUMP

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5508.145 1719.145 .730 1719.875 4.81 4.64 .33 1720.21 .00 .77 1.93 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.747 .0039 | .0056 .01 .73 1.11 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5509.892 1719.152 .730 1719.882 4.81 4.64 .33 1720.22 .00 .77 1.93 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.582 .0039 | .0060 .02 .73 1.11 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5513.473 1719.166 .704 1719.870 4.81 4.86 .37 1720.24 .00 .77 1.91 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.386 .0039 | .0068 .03 .70 1.19 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5517.859 1719.183 .680 1719.863 4.81 5.10 .40 1720.27 .00 .77 1.89 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.500 .0039 | .0078 .04 .68 1.27 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5522.359 1719.200 .657 1719.858 4.81 5.35 .44 1720.30 .00 .77 1.88 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.515 .0039 | .0089 .04 .66 1.36 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5526.874 1719.218 .635 1719.853 4.81 5.61 .49 1720.34 .00 .77 1.86 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.315 .0039 | .0101 .04 .64 1.46 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5531.189 1719.235 .613 1719.848 4.81 5.88 .54 1720.39 .00 .77 1.84 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.275 .0039 | .0116 .05 .61 1.56 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5535.465 1719.252 .592 1719.844 4.81 6.17 .59 1720.44 .00 .77 1.83 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.205 .0039 | .0132 .06 .59 1.66 .80 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5539.669 1719.268 .572 1719.840 4.81 6.47 .65 1720.49 .00 .77 1.81 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.114 .0039 | .0151 .06 .57 1.78 .80 .013 .00 .00 PIPE
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WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
5543.783	1719.284	.553	1719.837	4.81	6.79	.72	1720.55	.00	.77	1.79	2.000	.000	.00	1	.0
	4.007	.0039				.0173	.07	.55	1.90	.80	.013	.00	.00	PIPE	
5547.790	1719.300	.535	1719.835	4.81	7.12	.79	1720.62	.00	.77	1.77	2.000	.000	.00	1	.0
JUNCT STR	.0478					.0550	.11	.54	2.03		.013	.00	.00	PIPE	
5549.879	1719.400	.279	1719.679	2.40	10.59	1.74	1721.42	.00	.59	1.17	1.500	.000	.00	1	.0
	11.116	.0968				.0872	.97	.28	4.24	.28	.013	.00	.00	PIPE	
5560.996	1720.476	.285	1720.761	2.40	10.23	1.62	1722.39	.00	.59	1.18	1.500	.000	.00	1	.0
	7.172	.0968				.0776	.56	.29	4.04	.28	.013	.00	.00	PIPE	
5568.168	1721.171	.295	1721.466	2.40	9.75	1.48	1722.94	.00	.59	1.19	1.500	.000	.00	1	.0
	4.290	.0968				.0679	.29	.30	3.78	.28	.013	.00	.00	PIPE	
5572.458	1721.586	.305	1721.891	2.40	9.30	1.34	1723.23	.00	.59	1.21	1.500	.000	.00	1	.0
	2.984	.0968				.0593	.18	.31	3.54	.28	.013	.00	.00	PIPE	
5575.442	1721.875	.315	1722.190	2.40	8.87	1.22	1723.41	.00	.59	1.22	1.500	.000	.00	1	.0
	2.220	.0968				.0518	.12	.32	3.32	.28	.013	.00	.00	PIPE	
5577.663	1722.090	.326	1722.416	2.40	8.45	1.11	1723.53	.00	.59	1.24	1.500	.000	.00	1	.0
	1.744	.0968				.0453	.08	.33	3.11	.28	.013	.00	.00	PIPE	
5579.406	1722.259	.337	1722.596	2.40	8.06	1.01	1723.60	.00	.59	1.25	1.500	.000	.00	1	.0
	1.393	.0968				.0396	.06	.34	2.91	.28	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-1

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5580.799 1722.394 .349 1722.743 2.40 7.68 .92 1723.66 .00 .59 1.27 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.148 .0968 | | | | | | | | | | | | | | | |
|.0346 .04 .35 2.73 .28 .013 .00 .00 PIPE
5581.947 1722.505 .361 1722.866 2.40 7.33 .83 1723.70 .00 .59 1.28 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.958 .0968 | | | | | | | | | | | | | | | |
|.0303 .03 .36 2.56 .28 .013 .00 .00 PIPE
5582.906 1722.598 .373 1722.971 2.40 6.99 .76 1723.73 .00 .59 1.30 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.794 .0968 | | | | | | | | | | | | | | | |
|.0265 .02 .37 2.39 .28 .013 .00 .00 PIPE
5583.700 1722.675 .386 1723.061 2.40 6.66 .69 1723.75 .00 .59 1.31 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.674 .0968 | | | | | | | | | | | | | | | |
|.0232 .02 .39 2.24 .28 .013 .00 .00 PIPE
5584.374 1722.740 .399 1723.139 2.40 6.35 .63 1723.77 .00 .59 1.33 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.561 .0968 | | | | | | | | | | | | | | | |
|.0203 .01 .40 2.10 .28 .013 .00 .00 PIPE
5584.935 1722.794 .413 1723.207 2.40 6.06 .57 1723.78 .00 .59 1.34 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.478 .0968 | | | | | | | | | | | | | | | |
|.0177 .01 .41 1.96 .28 .013 .00 .00 PIPE
5585.413 1722.840 .427 1723.267 2.40 5.77 .52 1723.79 .00 .59 1.35 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.394 .0968 | | | | | | | | | | | | | | | |
|.0155 .01 .43 1.84 .28 .013 .00 .00 PIPE
5585.807 1722.879 .442 1723.321 2.40 5.51 .47 1723.79 .00 .59 1.37 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.322 .0968 | | | | | | | | | | | | | | | |
|.0136 .00 .44 1.72 .28 .013 .00 .00 PIPE
5586.128 1722.910 .458 1723.368 2.40 5.25 .43 1723.80 .00 .59 1.38 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.270 .0968 | | | | | | | | | | | | | | | |
|.0119 .00 .46 1.61 .28 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	1632.500	1.644	1634.144	57.61	14.52	3.28	1637.42	.00	2.46	2.99	3.000	.000	.00	1 .0
146.090	.0220					.0221	3.22	1.64	2.22	1.64	.013	.00	.00	PIPE
1146.090	1635.710	1.641	1637.351	57.61	14.56	3.29	1640.64	.33	2.46	2.99	3.000	.000	.00	1 .0
46.150	.0221					.0222	1.02	1.97	2.23	1.64	.013	.00	.00	PIPE
1192.240	1636.730	1.640	1638.370	57.61	14.57	3.30	1641.67	.00	2.46	2.99	3.000	.000	.00	1 .0
118.335	.0220					.0225	2.67	1.64	2.23	1.64	.013	.00	.00	PIPE
1310.575	1639.331	1.624	1640.955	57.61	14.75	3.38	1644.33	.00	2.46	2.99	3.000	.000	.00	1 .0
113.797	.0220					.0244	2.78	1.62	2.27	1.64	.013	.00	.00	PIPE
1424.372	1641.832	1.563	1643.395	57.61	15.47	3.71	1647.11	.00	2.46	3.00	3.000	.000	.00	1 .0
54.948	.0220					.0277	1.52	1.56	2.44	1.64	.013	.00	.00	PIPE
1479.320	1643.040	1.506	1644.546	57.61	16.22	4.09	1648.63	.41	2.46	3.00	3.000	.000	.00	1 .0
21.533	.0272					.0298	.64	1.91	2.63	1.54	.013	.00	.00	PIPE
1500.853	1643.625	1.496	1645.121	57.61	16.35	4.15	1649.27	.42	2.46	3.00	3.000	.000	.00	1 .0
72.717	.0272					.0321	2.34	1.91	2.66	1.54	.013	.00	.00	PIPE
1573.570	1645.600	1.442	1647.042	57.61	17.15	4.57	1651.61	.00	2.46	3.00	3.000	.000	.00	1 .0
JUNCT STR	.0214					.0345	.16	1.44	2.85		.013	.00	.00	PIPE
1578.240	1645.700	1.433	1647.133	57.61	17.28	4.64	1651.77	.00	2.46	3.00	3.000	.000	.00	1 .0
106.660	.0350					.0347	3.71	1.43	2.89	1.43	.013	.00	.00	PIPE

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1684.900	1649.430	1.436	1650.866	57.61	17.23	4.61	1655.48	.00	2.46	3.00	3.000	.000	.00	1	.0
149.504	.0350					.0327	4.89	1.44	2.88	1.43	.013	.00	.00	PIPE	
1834.404	1654.661	1.486	1656.147	57.61	16.48	4.22	1660.37	.00	2.46	3.00	3.000	.000	.00	1	.0
53.674	.0350					.0289	1.55	1.49	2.69	1.43	.013	.00	.00	PIPE	
1888.078	1656.539	1.543	1658.082	57.61	15.72	3.84	1661.92	.00	2.46	3.00	3.000	.000	.00	1	.0
30.312	.0350					.0255	.77	1.54	2.51	1.43	.013	.00	.00	PIPE	
1918.390	1657.600	1.603	1659.203	57.61	14.99	3.49	1662.69	.00	2.46	2.99	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0292	.14	1.62	2.33		.013	.00	.00	PIPE	
1923.070	1657.700	1.364	1659.063	52.56	16.82	4.39	1663.45	.02	2.36	2.99	3.000	.000	.00	1	.0
32.865	.0346					.0346	1.14	1.38	2.90	1.36	.013	.00	.00	PIPE	
1955.934	1658.836	1.364	1660.199	52.56	16.82	4.39	1664.59	.02	2.36	2.99	3.000	.000	.00	1	.0
166.816	.0346					.0329	5.49	1.38	2.90	1.36	.013	.00	.00	PIPE	
2122.750	1664.600	1.404	1666.004	52.56	16.20	4.07	1670.08	.00	2.36	2.99	3.000	.000	.00	1	.0
JUNCT STR	.0214					.0331	.15	1.42	2.74		.013	.00	.00	PIPE	
2127.420	1664.700	1.341	1666.041	51.36	16.79	4.38	1670.42	.02	2.33	2.98	3.000	.000	.00	1	.0
20.570	.0350					.0350	.72	1.36	2.92	1.34	.013	.00	.00	PIPE	
2147.990	1665.420	1.341	1666.761	51.36	16.79	4.38	1671.14	.02	2.33	2.98	3.000	.000	.00	1	.0
64.041	.0350					.0350	2.24	1.36	2.92	1.34	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
2212.030	1667.660	1.341	1669.001	51.36	16.80	4.38	1673.38	.00	2.33	2.98	3.000	.000	.00	1	.0
123.785	.0350					.0350	4.33	1.34	2.92	1.34	.013	.00	.00	PIPE	
2335.815	1671.993	1.341	1673.333	51.36	16.80	4.38	1677.71	.00	2.33	2.98	3.000	.000	.00	1	.0
164.184	.0350					.0341	5.59	1.34	2.92	1.34	.013	.00	.00	PIPE	
2499.998	1677.739	1.362	1679.101	51.36	16.46	4.20	1683.31	.00	2.33	2.99	3.000	.000	.00	1	.0
85.425	.0350					.0311	2.66	1.36	2.84	1.34	.013	.00	.00	PIPE	
2585.423	1680.729	1.413	1682.142	51.36	15.69	3.82	1685.96	.00	2.33	2.99	3.000	.000	.00	1	.0
38.607	.0350					.0274	1.06	1.41	2.64	1.34	.013	.00	.00	PIPE	
2624.030	1682.080	1.466	1683.546	51.36	14.96	3.47	1687.02	.35	2.33	3.00	3.000	.000	.00	1	.0
3.195	.0331					.0255	.08	1.81	2.46	1.36	.013	.00	.00	PIPE	
2627.226	1682.186	1.471	1683.657	51.36	14.89	3.44	1687.10	.34	2.33	3.00	3.000	.000	.00	1	.0
27.711	.0331					.0238	.66	1.82	2.45	1.36	.013	.00	.00	PIPE	
2654.937	1683.102	1.527	1684.630	51.36	14.20	3.13	1687.76	.31	2.33	3.00	3.000	.000	.00	1	.0
18.630	.0331					.0210	.39	1.84	2.28	1.36	.013	.00	.00	PIPE	
2673.567	1683.719	1.586	1685.305	51.36	13.54	2.85	1688.15	.28	2.33	3.00	3.000	.000	.00	1	.0
13.485	.0331					.0185	.25	1.87	2.12	1.36	.013	.00	.00	PIPE	
2687.053	1684.165	1.648	1685.813	51.36	12.91	2.59	1688.40	.26	2.33	2.99	3.000	.000	.00	1	.0
10.127	.0331					.0163	.17	1.91	1.97	1.36	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:54:46

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
2697.180	1684.500	1.714	1686.214	51.36	12.31	2.35	1688.57	.00	2.33	2.97	3.000	.000	.00	1 .0
JUNCT STR	.0214					.0157	.07	1.71	1.83		.013	.00	.00	PIPE
2701.850	1684.600	1.676	1686.276	50.72	12.49	2.42	1688.70	.00	2.32	2.98	3.000	.000	.00	1 .0
161.650	.0160					.0160	2.58	1.68	1.88	1.68	.013	.00	.00	PIPE
2863.500	1687.190	1.678	1688.868	50.72	12.47	2.41	1691.28	.02	2.32	2.98	3.000	.000	.00	1 .0
51.071	.0160					.0160	.81	1.70	1.88	1.68	.013	.00	.00	PIPE
2914.571	1688.005	1.678	1689.683	50.72	12.47	2.41	1692.10	.02	2.32	2.98	3.000	.000	.00	1 .0
165.779	.0160					.0157	2.60	1.70	1.88	1.68	.013	.00	.00	PIPE
3080.350	1690.650	1.698	1692.348	50.72	12.29	2.35	1694.69	.00	2.32	2.97	3.000	.000	.00	1 .0
43.270	.0163					.0151	.65	1.70	1.84	1.67	.013	.00	.00	PIPE
3123.620	1691.353	1.720	1693.073	50.72	12.09	2.27	1695.34	.00	2.32	2.97	3.000	.000	.00	1 .0
57.343	.0163					.0139	.80	1.72	1.79	1.67	.013	.00	.00	PIPE
3180.963	1692.285	1.790	1694.075	50.72	11.53	2.06	1696.14	.00	2.32	2.94	3.000	.000	.00	1 .0
28.841	.0163					.0123	.35	1.79	1.66	1.67	.013	.00	.00	PIPE
3209.803	1692.754	1.863	1694.617	50.72	10.99	1.88	1696.49	.00	2.32	2.91	3.000	.000	.00	1 .0
17.232	.0163					.0109	.19	1.86	1.54	1.67	.013	.00	.00	PIPE
3227.036	1693.034	1.941	1694.975	50.72	10.48	1.71	1696.68	.00	2.32	2.87	3.000	.000	.00	1 .0
10.933	.0163					.0097	.11	1.94	1.42	1.67	.013	.00	.00	PIPE

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem |Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
3237.969 1693.212 2.024 1695.236 50.72 9.99 1.55 1696.79 .00 2.32 2.81 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.789 .0163 | | | | | .0086 .06 2.02 1.31 1.67 .013 .00 .00 PIPE
3244.758 1693.323 2.113 1695.436 50.72 9.53 1.41 1696.85 .00 2.32 2.74 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.632 .0163 | | | | | .0077 .03 2.11 1.20 1.67 .013 .00 .00 PIPE
3248.390 1693.382 2.210 1695.592 50.72 9.08 1.28 1696.87 .00 2.32 2.64 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.130 .0163 | | | | | .0069 .01 2.21 1.10 1.67 .013 .00 .00 PIPE
3249.520 1693.400 2.317 1695.717 50.72 8.66 1.16 1696.88 .00 2.32 2.52 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR .0214 | | | | | .0054 .03 2.32 1.00 .013 .00 .00 PIPE
3254.200 1693.500 2.809 1696.309 46.91 6.82 .72 1697.03 .00 2.23 1.46 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
39.050 .0026 | | | | | .0043 .17 2.81 .55 3.00 .013 .00 .00 PIPE
3293.250 1693.600 2.902 1696.502 46.91 6.70 .70 1697.20 .00 2.23 1.07 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR .0214 | | | | | .0028 .01 2.90 .46 .013 .00 .00 PIPE
3297.920 1693.700 3.868 1697.568 23.18 3.28 .17 1697.73 .00 1.55 .00 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
293.915 .0042 | | | | | .0012 .35 3.87 .00 1.57 .013 .00 .00 PIPE
3591.835 1694.922 3.000 1697.922 23.18 3.28 .17 1698.09 .00 1.55 .00 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
86.012 .0042 | | | | | .0011 .10 3.00 .00 1.57 .013 .00 .00 PIPE
3677.847 1695.280 2.721 1698.001 23.18 3.44 .18 1698.19 .00 1.55 1.74 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
46.518 .0042 | | | | | .0011 .05 2.72 .31 1.57 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3965.695 | 1696.478 | 1.717 | 1698.195 | 23.18 | 5.54 | .48 | 1698.67 | .00 | 1.55 | 2.97 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
20.206 | .0042 | | | | | .0033 | .07 | 1.72 | .82 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3985.902 | 1696.562 | 1.652 | 1698.214 | 23.18 | 5.81 | .52 | 1698.74 | .00 | 1.55 | 2.98 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
22.855 | .0042 | | | | | .0037 | .09 | 1.65 | .89 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4008.757 | 1696.657 | 1.590 | 1698.247 | 23.18 | 6.09 | .58 | 1698.82 | .00 | 1.55 | 2.99 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
14.708 | .0042 | | | | | .0041 | .06 | 1.59 | .95 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4023.465 | 1696.718 | 1.568 | 1698.286 | 23.18 | 6.20 | .60 | 1698.88 | .00 | 1.55 | 3.00 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1414.135 | .0042 | | | | | .0041 | 5.85 | 1.57 | .98 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5437.600 | 1702.600 | 1.568 | 1704.168 | 23.18 | 6.20 | .60 | 1704.77 | .00 | 1.55 | 3.00 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR | .0214 | | | | | .0028 | .01 | 1.57 | .98 | | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5442.270 | 1702.700 | 1.958 | 1704.658 | 19.02 | 3.89 | .24 | 1704.89 | .00 | 1.40 | 2.86 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
10.660 | .0067 | | | | | .0015 | .02 | 1.96 | .52 | 1.22 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5452.930 | 1702.771 | 1.879 | 1704.650 | 19.02 | 4.08 | .26 | 1704.91 | .00 | 1.40 | 2.90 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
9.818 | .0067 | | | | | .0017 | .02 | 1.88 | .57 | 1.22 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5462.748 | 1702.837 | 1.804 | 1704.641 | 19.02 | 4.28 | .28 | 1704.93 | .00 | 1.40 | 2.94 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.680 | .0067 | | | | | .0019 | .02 | 1.80 | .61 | 1.22 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5471.427 | 1702.895 | 1.734 | 1704.629 | 19.02 | 4.49 | .31 | 1704.94 | .00 | 1.40 | 2.96 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.649 | .0067 | | | | | .0022 | .02 | 1.73 | .66 | 1.22 | .013 | .00 | .00 | PIPE
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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
5479.076 | 1702.946 | 1.668 | 1704.614 | 19.02 | 4.71 | .34 | 1704.96 | .00 | 1.40 | 2.98 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.720 | .0067 | | | | | .0024 | .02 | 1.67 | .71 | 1.22 | .013 | .00 | .00 | PIPE
5485.796 | 1702.991 | 1.605 | 1704.596 | 19.02 | 4.94 | .38 | 1704.98 | .00 | 1.40 | 2.99 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.636 | .0067 | | | | | .0028 | .02 | 1.61 | .77 | 1.22 | .013 | .00 | .00 | PIPE
5491.433 | 1703.029 | 1.545 | 1704.574 | 19.02 | 5.18 | .42 | 1704.99 | .00 | 1.40 | 3.00 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.268 | .0067 | | | | | .0031 | .01 | 1.55 | .83 | 1.22 | .013 | .00 | .00 | PIPE
5495.700 | 1703.057 | 1.488 | 1704.546 | 19.02 | 5.44 | .46 | 1705.00 | .00 | 1.40 | 3.00 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP
5495.700 | 1703.057 | 1.298 | 1704.355 | 19.02 | 6.49 | .65 | 1705.01 | .00 | 1.40 | 2.97 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.710 | .0067 | | | | | .0051 | .02 | 1.30 | 1.15 | 1.22 | .013 | .00 | .00 | PIPE
5500.410 | 1703.089 | 1.346 | 1704.435 | 19.02 | 6.19 | .59 | 1705.03 | .00 | 1.40 | 2.98 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.670 | .0067 | | | | | .0044 | .01 | 1.35 | 1.07 | 1.22 | .013 | .00 | .00 | PIPE
5502.080 | 1703.100 | 1.397 | 1704.497 | 19.02 | 5.89 | .54 | 1705.04 | .07 | 1.40 | 2.99 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR .0000 | | | | | | .0028 | .01 | 1.47 | 1.00 | | | .013 | .00 | .00 | PIPE
5504.560 | 1703.100 | 1.787 | 1704.887 | 16.63 | 3.79 | .22 | 1705.11 | .00 | 1.30 | 2.94 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.540 | .0066 | | | | | .0015 | .01 | 1.79 | .55 | 1.14 | .013 | .00 | .00 | PIPE
5509.100 | 1703.130 | 1.753 | 1704.883 | 16.63 | 3.88 | .23 | 1705.12 | .02 | 1.30 | 2.96 | 3.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
10.783 | .0057 | | | | | .0016 | .02 | 1.78 | .57 | 1.19 | .013 | .00 | .00 | PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
5519.883 1703.192 1.685 1704.877 16.63 4.07 .26 1705.13 .03 1.30 2.98 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.153 .0057 | .0018 .01 1.71 .61 1.19 .013 .00 .00 PIPE
5527.036 1703.233 1.621 1704.854 16.63 4.27 .28 1705.14 .03 1.30 2.99 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP
5527.036 1703.233 1.020 1704.253 16.63 7.84 .95 1705.21 .09 1.30 2.84 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
7.090 .0057 | .0104 .07 1.11 1.60 1.19 .013 .00 .00 PIPE
5534.126 1703.273 1.002 1704.276 16.63 8.03 1.00 1705.28 .09 1.30 2.83 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
11.547 .0057 | .0115 .13 1.10 1.66 1.19 .013 .00 .00 PIPE
5545.673 1703.340 .968 1704.308 16.63 8.43 1.10 1705.41 .10 1.30 2.81 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
10.547 .0057 | .0131 .14 1.07 1.77 1.19 .013 .00 .00 PIPE
5556.220 1703.400 .935 1704.336 16.63 8.84 1.21 1705.55 .00 1.30 2.78 3.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR .0214 | .0171 .08 .94 1.89 .013 .00 .00 PIPE
5560.891 1703.500 .941 1704.441 14.47 9.95 1.54 1705.98 .00 1.37 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
13.372 .0240 | .0194 .26 .94 2.06 .90 .013 .00 .00 PIPE
5574.262 1703.821 .961 1704.782 14.47 9.69 1.46 1706.24 .00 1.37 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
15.108 .0240 | .0176 .27 .96 1.98 .90 .013 .00 .00 PIPE
5589.370 1704.184 .997 1705.181 14.47 9.24 1.33 1706.51 .00 1.37 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
9.708 .0240 | .0155 .15 1.00 1.84 .90 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
5599.079	1704.417	1.035	1705.452	14.47	8.81	1.21	1706.66	.00	1.37	2.00	2.000	.000	.00	1	.0
	6.726	.0240				.0137	.09	1.04	1.71	.90	.013	.00	.00	PIPE	
5605.805	1704.579	1.075	1705.654	14.47	8.40	1.10	1706.75	.00	1.37	1.99	2.000	.000	.00	1	.0
	4.737	.0240				.0121	.06	1.08	1.59	.90	.013	.00	.00	PIPE	
5610.542	1704.692	1.118	1705.810	14.47	8.01	1.00	1706.81	.00	1.37	1.99	2.000	.000	.00	1	.0
	3.484	.0240				.0107	.04	1.12	1.48	.90	.013	.00	.00	PIPE	
5614.026	1704.776	1.162	1705.938	14.47	7.64	.91	1706.84	.00	1.37	1.97	2.000	.000	.00	1	.0
	2.420	.0240				.0094	.02	1.16	1.37	.90	.013	.00	.00	PIPE	
5616.446	1704.834	1.209	1706.043	14.47	7.28	.82	1706.87	.00	1.37	1.96	2.000	.000	.00	1	.0
	1.584	.0240				.0083	.01	1.21	1.27	.90	.013	.00	.00	PIPE	
5618.030	1704.872	1.259	1706.131	14.47	6.94	.75	1706.88	.00	1.37	1.93	2.000	.000	.00	1	.0
	.905	.0240				.0074	.01	1.26	1.18	.90	.013	.00	.00	PIPE	
5618.935	1704.894	1.312	1706.206	14.47	6.62	.68	1706.89	.00	1.37	1.90	2.000	.000	.00	1	.0
	.255	.0240				.0066	.00	1.31	1.09	.90	.013	.00	.00	PIPE	
5619.190	1704.900	1.371	1706.271	14.47	6.31	.62	1706.89	.00	1.37	1.86	2.000	.000	.00	1	.0
JUNCT STR	.0214					.0033	.02	1.37	1.00		.013	.00	.00	PIPE	
5623.870	1705.000	1.960	1706.960	4.47	1.43	.03	1706.99	.00	.74	.56	2.000	.000	.00	1	.0
	9.463	.0167				.0003	.00	1.96	.11	.53	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:54:46

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El. | Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem |Ch Slope |
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5633.333 | 1705.158 | 1.802 | 1706.960 | 4.47 | 1.50 | .03 | 1707.00 | .00 | .74 | 1.19 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.211 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5639.544 | 1705.262 | 1.697 | 1706.959 | 4.47 | 1.57 | .04 | 1707.00 | .00 | .74 | 1.43 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.159 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5644.703 | 1705.348 | 1.609 | 1706.957 | 4.47 | 1.65 | .04 | 1707.00 | .00 | .74 | 1.59 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.410 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5649.113 | 1705.422 | 1.533 | 1706.955 | 4.47 | 1.73 | .05 | 1707.00 | .00 | .74 | 1.69 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.027 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5653.141 | 1705.489 | 1.463 | 1706.952 | 4.47 | 1.81 | .05 | 1707.00 | .00 | .74 | 1.77 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.580 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5656.720 | 1705.549 | 1.400 | 1706.949 | 4.47 | 1.90 | .06 | 1707.01 | .00 | .74 | 1.83 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.313 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5660.034 | 1705.604 | 1.341 | 1706.945 | 4.47 | 2.00 | .06 | 1707.01 | .00 | .74 | 1.88 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.044 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5663.078 | 1705.655 | 1.286 | 1706.941 | 4.47 | 2.09 | .07 | 1707.01 | .00 | .74 | 1.92 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.833 | .0167 | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5665.911 | 1705.702 | 1.234 | 1706.936 | 4.47 | 2.20 | .07 | 1707.01 | .00 | .74 | 1.94 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.555 | .0167 | | | | | | | | | | | | | | |
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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
5668.466	1705.745	1.186	1706.931	4.47	2.30	.08	1707.01	.00	.74	1.97	2.000	.000	.00	1	.0
	2.399	.0167				.0010	.00	1.19	.41	.53	.013	.00	.00	PIPE	
5670.865	1705.785	1.140	1706.925	4.47	2.41	.09	1707.02	.00	.74	1.98	2.000	.000	.00	1	.0
	2.173	.0167				.0011	.00	1.14	.44	.53	.013	.00	.00	PIPE	
5673.039	1705.821	1.097	1706.918	4.47	2.53	.10	1707.02	.00	.74	1.99	2.000	.000	.00	1	.0
	2.006	.0167				.0012	.00	1.10	.47	.53	.013	.00	.00	PIPE	
5675.044	1705.855	1.056	1706.911	4.47	2.66	.11	1707.02	.00	.74	2.00	2.000	.000	.00	1	.0
	1.897	.0167				.0014	.00	1.06	.51	.53	.013	.00	.00	PIPE	
5676.941	1705.886	1.016	1706.902	4.47	2.79	.12	1707.02	.00	.74	2.00	2.000	.000	.00	1	.0
	.031	.0167				.0016	.00	1.02	.55	.53	.013	.00	.00	PIPE	
5676.972	1705.887	.979	1706.866	4.47	2.92	.13	1707.00	.00	.74	2.00	2.000	.000	.00	1	.0
HYDRAULIC JUMP															
5676.972	1705.887	.529	1706.415	4.47	6.73	.70	1707.12	.00	.74	1.76	2.000	.000	.00	1	.0
	423.312	.0167				.0167	7.07	.53	1.93	.53	.013	.00	.00	PIPE	
6100.284	1712.956	.529	1713.484	4.47	6.73	.70	1714.19	.00	.74	1.76	2.000	.000	.00	1	.0
	44.565	.0167				.0158	.71	.53	1.93	.53	.013	.00	.00	PIPE	
6144.849	1713.700	.543	1714.243	4.47	6.47	.65	1714.89	.00	.74	1.78	2.000	.000	.00	1	.0
	14.970	.0167				.0140	.21	.54	1.83	.53	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:54:46

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
6159.819	1713.950	.562	1714.512	4.47	6.17	.59	1715.10	.00	.74	1.80	2.000	.000	.00	1	.0
	7.837	.0167				.0123	.10	.56	1.71	.53	.013	.00	.00	PIPE	
6167.656	1714.081	.581	1714.662	4.47	5.88	.54	1715.20	.00	.74	1.82	2.000	.000	.00	1	.0
	4.672	.0167				.0107	.05	.58	1.60	.53	.013	.00	.00	PIPE	
6172.328	1714.159	.602	1714.761	4.47	5.61	.49	1715.25	.00	.74	1.83	2.000	.000	.00	1	.0
	3.210	.0167				.0094	.03	.60	1.50	.53	.013	.00	.00	PIPE	
6175.538	1714.213	.623	1714.836	4.47	5.35	.44	1715.28	.00	.74	1.85	2.000	.000	.00	1	.0
	2.171	.0167				.0082	.02	.62	1.40	.53	.013	.00	.00	PIPE	
6177.709	1714.249	.645	1714.894	4.47	5.10	.40	1715.30	.00	.74	1.87	2.000	.000	.00	1	.0
	1.445	.0167				.0072	.01	.65	1.31	.53	.013	.00	.00	PIPE	
6179.155	1714.273	.668	1714.941	4.47	4.86	.37	1715.31	.00	.74	1.89	2.000	.000	.00	1	.0
	.999	.0167				.0063	.01	.67	1.23	.53	.013	.00	.00	PIPE	
6180.154	1714.290	.691	1714.981	4.47	4.64	.33	1715.31	.00	.74	1.90	2.000	.000	.00	1	.0
	.478	.0167				.0055	.00	.69	1.15	.53	.013	.00	.00	PIPE	
6180.632	1714.298	.716	1715.014	4.47	4.42	.30	1715.32	.00	.74	1.92	2.000	.000	.00	1	.0
	.138	.0167				.0048	.00	.72	1.07	.53	.013	.00	.00	PIPE	
6180.770	1714.300	.743	1715.043	4.47	4.21	.27	1715.32	.00	.74	1.93	2.000	.000	.00	1	.0
JUNCT STR	.0214					.0024	.01	.74	1.00		.013	.00	.00	PIPE	



20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El. | Elev  | Depth  | Width  | Dia.-FT|or I.D. | ZL  | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
6200.259 1714.650 .715 1715.365 1.93 1.91 .06 1715.42 .00 .48 1.92 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.224 .0168 | .0010 .00 .72 .46 .35 .013 .00 .00 PIPE
6201.483 1714.670 .690 1715.360 1.93 2.01 .06 1715.42 .00 .48 1.90 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.135 .0168 | .0012 .00 .69 .50 .35 .013 .00 .00 PIPE
6202.618 1714.689 .666 1715.355 1.93 2.10 .07 1715.42 .00 .48 1.89 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.771 .0168 | .0014 .00 .67 .53 .35 .013 .00 .00 PIPE
6203.388 1714.702 .644 1715.346 1.93 2.21 .08 1715.42 .00 .48 1.87 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP
6203.388 1714.702 .348 1715.050 1.93 5.28 .43 1715.48 .00 .48 1.52 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
59.278 .0168 | .0168 1.00 .35 1.89 .35 .013 .00 .00 PIPE
6262.666 1715.700 .348 1716.047 1.93 5.28 .43 1716.48 .00 .48 1.52 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
13.824 .0168 | .0169 .23 .35 1.89 .35 .013 .00 .00 PIPE
6276.490 1715.932 .346 1716.278 1.93 5.29 .43 1716.71 .00 .48 1.51 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
24.811 .0168 | .0181 .45 .35 1.90 .35 .013 .00 .00 PIPE
6301.301 1716.350 .335 1716.685 1.93 5.55 .48 1717.16 .00 .48 1.49 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
9.346 .0168 | .0208 .19 .34 2.03 .35 .013 .00 .00 PIPE
6310.647 1716.507 .324 1716.831 1.93 5.82 .53 1717.36 .00 .48 1.47 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.118 .0168 | .0238 .15 .32 2.16 .35 .013 .00 .00 PIPE

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
6316.765	1716.610	.314	1716.924	1.93	6.11	.58	1717.50	.00	.48	1.46	2.000	.000	.00	1	.0
4.586	.0168					.0273	.13	.31	2.31	.35	.013	.00	.00	PIPE	
6321.351	1716.687	.304	1716.991	1.93	6.40	.64	1717.63	.00	.48	1.44	2.000	.000	.00	1	.0
3.727	.0168					.0312	.12	.30	2.46	.35	.013	.00	.00	PIPE	
6325.077	1716.750	.294	1717.044	1.93	6.72	.70	1717.74	.00	.48	1.42	2.000	.000	.00	1	.0
3.174	.0168					.0357	.11	.29	2.63	.35	.013	.00	.00	PIPE	
6328.251	1716.803	.284	1717.087	1.93	7.04	.77	1717.86	.00	.48	1.40	2.000	.000	.00	1	.0
2.824	.0168					.0409	.12	.28	2.80	.35	.013	.00	.00	PIPE	
6331.075	1716.851	.275	1717.126	1.93	7.39	.85	1717.97	.00	.48	1.38	2.000	.000	.00	1	.0
2.521	.0168					.0469	.12	.28	2.99	.35	.013	.00	.00	PIPE	
6333.596	1716.893	.266	1717.159	1.93	7.75	.93	1718.09	.00	.48	1.36	2.000	.000	.00	1	.0
2.309	.0168					.0537	.12	.27	3.19	.35	.013	.00	.00	PIPE	
6335.904	1716.932	.258	1717.190	1.93	8.13	1.03	1718.22	.00	.48	1.34	2.000	.000	.00	1	.0
2.091	.0168					.0616	.13	.26	3.40	.35	.013	.00	.00	PIPE	
6337.996	1716.967	.249	1717.216	1.93	8.52	1.13	1718.34	.00	.48	1.32	2.000	.000	.00	1	.0
1.964	.0168					.0705	.14	.25	3.63	.35	.013	.00	.00	PIPE	
6339.960	1717.000	.242	1717.242	1.93	8.94	1.24	1718.48	.00	.48	1.30	2.000	.000	.00	1	.0
JUNCT STR	.0481					.0726	.15	.24	3.87		.013	.00	.00	PIPE	

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head   | Grd.El. | Elev   | Depth  | Width  | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
L/Elem  | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
6342.040 | 1717.100 | .268 | 1717.368 | 1.93 | 9.03 | 1.27 | 1718.63 | .00 | .52 | 1.15 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
32.129 | .0698 | | | | | .0698 | 2.24 | .27 | 3.69 | .27 | .013 | .00 | .00 | PIPE
6374.169 | 1719.343 | .268 | 1719.610 | 1.93 | 9.03 | 1.27 | 1720.88 | .00 | .52 | 1.15 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
24.021 | .0698 | | | | | .0665 | 1.60 | .27 | 3.69 | .27 | .013 | .00 | .00 | PIPE
6398.190 | 1721.019 | .274 | 1721.293 | 1.93 | 8.72 | 1.18 | 1722.47 | .00 | .52 | 1.16 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
9.263 | .0698 | | | | | .0592 | .55 | .27 | 3.52 | .27 | .013 | .00 | .00 | PIPE
6407.453 | 1721.666 | .283 | 1721.949 | 1.93 | 8.31 | 1.07 | 1723.02 | .00 | .52 | 1.17 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
4.841 | .0698 | | | | | .0517 | .25 | .28 | 3.29 | .27 | .013 | .00 | .00 | PIPE
6412.294 | 1722.004 | .293 | 1722.297 | 1.93 | 7.93 | .98 | 1723.27 | .00 | .52 | 1.19 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
3.200 | .0698 | | | | | .0452 | .14 | .29 | 3.09 | .27 | .013 | .00 | .00 | PIPE
6415.494 | 1722.227 | .303 | 1722.530 | 1.93 | 7.56 | .89 | 1723.42 | .00 | .52 | 1.20 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
2.330 | .0698 | | | | | .0395 | .09 | .30 | 2.89 | .27 | .013 | .00 | .00 | PIPE
6417.824 | 1722.390 | .313 | 1722.703 | 1.93 | 7.20 | .81 | 1723.51 | .00 | .52 | 1.22 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
1.765 | .0698 | | | | | .0345 | .06 | .31 | 2.71 | .27 | .013 | .00 | .00 | PIPE
6419.589 | 1722.513 | .324 | 1722.837 | 1.93 | 6.87 | .73 | 1723.57 | .00 | .52 | 1.23 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
1.404 | .0698 | | | | | .0302 | .04 | .32 | 2.54 | .27 | .013 | .00 | .00 | PIPE
6420.993 | 1722.611 | .335 | 1722.946 | 1.93 | 6.55 | .67 | 1723.61 | .00 | .52 | 1.25 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|-----|
1.141 | .0698 | | | | | .0264 | .03 | .34 | 2.38 | .27 | .013 | .00 | .00 | PIPE
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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS) | (FPS) | Head  | Grd.El.| Elev  | Depth  | Width  | Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope|          |          |          |          | SF Ave| HF     |SE Dpth|Froude N|Norm Dp | "N"    | X-Fall| ZR  |Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6422.134 | 1722.691 | .346 | 1723.037 | 1.93 | 6.24 | .61 | 1723.64 | .00 | .52 | 1.26 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.921 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6423.055 | 1722.755 | .358 | 1723.113 | 1.93 | 5.95 | .55 | 1723.66 | .00 | .52 | 1.28 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.766 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6423.821 | 1722.809 | .370 | 1723.179 | 1.93 | 5.68 | .50 | 1723.68 | .00 | .52 | 1.29 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.623 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6424.444 | 1722.852 | .383 | 1723.235 | 1.93 | 5.41 | .45 | 1723.69 | .00 | .52 | 1.31 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.521 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6424.965 | 1722.889 | .396 | 1723.285 | 1.93 | 5.16 | .41 | 1723.70 | .00 | .52 | 1.32 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.419 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6425.384 | 1722.918 | .410 | 1723.328 | 1.93 | 4.92 | .38 | 1723.70 | .00 | .52 | 1.34 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.348 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6425.732 | 1722.942 | .424 | 1723.366 | 1.93 | 4.69 | .34 | 1723.71 | .00 | .52 | 1.35 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.271 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6426.003 | 1722.961 | .439 | 1723.400 | 1.93 | 4.47 | .31 | 1723.71 | .00 | .52 | 1.37 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.218 | .0698 | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6426.221 | 1722.976 | .454 | 1723.430 | 1.93 | 4.27 | .28 | 1723.71 | .00 | .52 | 1.38 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.156 | .0698 | | | | | | | | | | | | | | | |
*****

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-2

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Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
6426.377	1722.987	.470	1723.457	1.93	4.07	.26	1723.71	.00	.52	1.39	1.500	.000	.00	1 .0
	.101					.0069	.00	.47	1.23	.27	.013	.00	.00	PIPE
6426.479	1722.994	.487	1723.481	1.93	3.88	.23	1723.71	.00	.52	1.40	1.500	.000	.00	1 .0
	.066					.0061	.00	.49	1.15	.27	.013	.00	.00	PIPE
6426.545	1722.999	.504	1723.503	1.93	3.70	.21	1723.72	.00	.52	1.42	1.500	.000	.00	1 .0
	.015					.0053	.00	.50	1.07	.27	.013	.00	.00	PIPE
6426.560	1723.000	.523	1723.523	1.93	3.52	.19	1723.72	.00	.52	1.43	1.500	.000	.00	1 .0

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20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1096.677 1645.174 1.618 1646.792 4.55 1.67 .04 1646.84 .00 .75 1.57 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.671 .0445 | | | | | .0004 .00 1.62 .22 .42 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1098.348 1645.248 1.540 1646.789 4.55 1.75 .05 1646.84 .00 .75 1.68 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.481 .0445 | | | | | .0005 .00 1.54 .25 .42 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1099.830 1645.314 1.470 1646.784 4.55 1.84 .05 1646.84 .00 .75 1.77 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.336 .0445 | | | | | .0005 .00 1.47 .27 .42 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1101.166 1645.374 1.406 1646.780 4.55 1.93 .06 1646.84 .00 .75 1.83 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.235 .0445 | | | | | .0006 .00 1.41 .30 .42 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1102.401 1645.429 1.346 1646.775 4.55 2.02 .06 1646.84 .00 .75 1.88 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.110 .0445 | | | | | .0007 .00 1.35 .33 .42 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1103.511 1645.478 1.291 1646.769 4.55 2.12 .07 1646.84 .00 .75 1.91 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.827 .0445 | | | | | .0008 .00 1.29 .35 .42 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1104.338 1645.515 1.239 1646.754 4.55 2.23 .08 1646.83 .00 .75 1.94 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP | | | | | | | | | | | | | | | |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1104.338 1645.515 .417 1645.932 4.55 9.58 1.43 1647.36 .00 .75 1.63 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
30.212 .0445 | | | | | .0440 1.33 .42 3.12 .42 .013 .00 .00 PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1134.550 1646.860 .419 1647.280 4.55 9.51 1.40 1648.68 .08 .75 1.63 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
12.240 .0446 | | | | | .0430 .53 .50 3.09 .42 .013 .00 .00 PIPE

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WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1146.790 | 1647.406 | .421 | 1647.827 | 4.55 | 9.43 | 1.38 | 1649.21 | .08 | .75 | 1.63 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
23.109 | .0446 | | | | | .0398 | .92 | .50 | 3.05 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1169.899 | 1648.438 | .436 | 1648.874 | 4.55 | 8.99 | 1.25 | 1650.13 | .07 | .75 | 1.65 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
10.141 | .0446 | | | | | .0349 | .35 | .51 | 2.86 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1180.040 | 1648.890 | .451 | 1649.341 | 4.55 | 8.57 | 1.14 | 1650.48 | .00 | .75 | 1.67 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
5.163 | .0447 | | | | | .0309 | .16 | .45 | 2.68 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1185.203 | 1649.121 | .463 | 1649.584 | 4.55 | 8.25 | 1.06 | 1650.64 | .00 | .75 | 1.69 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.639 | .0447 | | | | | .0274 | .13 | .46 | 2.55 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1189.842 | 1649.328 | .479 | 1649.807 | 4.55 | 7.87 | .96 | 1650.77 | .00 | .75 | 1.71 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
3.446 | .0447 | | | | | .0240 | .08 | .48 | 2.38 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1193.288 | 1649.482 | .495 | 1649.977 | 4.55 | 7.50 | .87 | 1650.85 | .00 | .75 | 1.73 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.632 | .0447 | | | | | .0209 | .06 | .50 | 2.23 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1195.920 | 1649.600 | .512 | 1650.112 | 4.55 | 7.16 | .79 | 1650.91 | .00 | .75 | 1.75 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.058 | .0447 | | | | | .0183 | .04 | .51 | 2.09 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1197.978 | 1649.692 | .530 | 1650.222 | 4.55 | 6.82 | .72 | 1650.94 | .00 | .75 | 1.77 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.664 | .0447 | | | | | .0160 | .03 | .53 | 1.96 | .42 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1199.642 | 1649.766 | .548 | 1650.314 | 4.55 | 6.50 | .66 | 1650.97 | .00 | .75 | 1.78 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.328 | .0447 | | | | | .0140 | .02 | .55 | 1.83 | .42 | .013 | .00 | .00 | PIPE

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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:45:28

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1200.971	1649.825	.567	1650.392	4.55	6.20	.60	1650.99	.00	.75	1.80	2.000	.000	.00	1	.0
1.058	.0447					.0123	.01	.57	1.71	.42	.013	.00	.00	PIPE	
1202.029	1649.872	.587	1650.460	4.55	5.91	.54	1651.00	.00	.75	1.82	2.000	.000	.00	1	.0
.865	.0447					.0107	.01	.59	1.60	.42	.013	.00	.00	PIPE	
1202.894	1649.911	.607	1650.518	4.55	5.64	.49	1651.01	.00	.75	1.84	2.000	.000	.00	1	.0
.648	.0447					.0094	.01	.61	1.50	.42	.013	.00	.00	PIPE	
1203.542	1649.940	.629	1650.569	4.55	5.38	.45	1651.02	.00	.75	1.86	2.000	.000	.00	1	.0
.516	.0447					.0082	.00	.63	1.40	.42	.013	.00	.00	PIPE	
1204.058	1649.963	.651	1650.614	4.55	5.13	.41	1651.02	.00	.75	1.87	2.000	.000	.00	1	.0
.376	.0447					.0072	.00	.65	1.31	.42	.013	.00	.00	PIPE	
1204.433	1649.980	.674	1650.654	4.55	4.89	.37	1651.02	.00	.75	1.89	2.000	.000	.00	1	.0
.253	.0447					.0063	.00	.67	1.23	.42	.013	.00	.00	PIPE	
1204.687	1649.991	.698	1650.689	4.55	4.66	.34	1651.03	.00	.75	1.91	2.000	.000	.00	1	.0
.144	.0447					.0055	.00	.70	1.15	.42	.013	.00	.00	PIPE	
1204.831	1649.998	.723	1650.721	4.55	4.44	.31	1651.03	.00	.75	1.92	2.000	.000	.00	1	.0
.049	.0447					.0048	.00	.72	1.07	.42	.013	.00	.00	PIPE	
1204.880	1650.000	.750	1650.750	4.55	4.23	.28	1651.03	.00	.75	1.94	2.000	.000	.00	1	.0





20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3B

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1002.200 | 1644.600 | 3.040 | 1647.640 | 8.94 | 2.85 | .13 | 1647.77 | .00 | 1.07 | .00 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
24.784 | .0435 | | | | | .0015 | .04 | 3.04 | .00 | .59 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1026.984 | 1645.679 | 2.000 | 1647.679 | 8.94 | 2.85 | .13 | 1647.80 | .00 | 1.07 | .00 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.096 | .0435 | | | | | .0014 | .01 | 2.00 | .00 | .59 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1031.080 | 1645.857 | 1.814 | 1647.671 | 8.94 | 2.98 | .14 | 1647.81 | .00 | 1.07 | 1.16 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1.377 | .0435 | | | | | .0014 | .00 | 1.81 | .33 | .59 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1032.457 | 1645.917 | 1.748 | 1647.665 | 8.94 | 3.07 | .15 | 1647.81 | .00 | 1.07 | 1.33 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
HYDRAULIC JUMP
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1032.457 | 1645.917 | .621 | 1646.538 | 8.94 | 10.75 | 1.79 | 1648.33 | .00 | 1.07 | 1.85 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.478 | .0435 | | | | | .0355 | .23 | .62 | 2.83 | .59 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1038.934 | 1646.199 | .621 | 1646.820 | 8.94 | 10.73 | 1.79 | 1648.61 | .00 | 1.07 | 1.85 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
13.634 | .0435 | | | | | .0332 | .45 | .62 | 2.82 | .59 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1052.568 | 1646.792 | .643 | 1647.435 | 8.94 | 10.23 | 1.63 | 1649.06 | .00 | 1.07 | 1.87 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
8.651 | .0435 | | | | | .0291 | .25 | .64 | 2.64 | .59 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1061.219 | 1647.169 | .666 | 1647.835 | 8.94 | 9.76 | 1.48 | 1649.31 | .00 | 1.07 | 1.89 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
6.124 | .0435 | | | | | .0255 | .16 | .67 | 2.47 | .59 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1067.343 | 1647.435 | .690 | 1648.125 | 8.94 | 9.30 | 1.34 | 1649.47 | .00 | 1.07 | 1.90 | 2.000 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
4.635 | .0435 | | | | | .0224 | .10 | .69 | 2.31 | .59 | .013 | .00 | .00 | PIPE
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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:46:49

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3B

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super |Critical|Flow Top|Height/|Base Wt| |No Wth
Station | Elev  | (FT)  | Elev  | (CFS)| (FPS)| Grd.El.| Elev  |Depth  |Width  |Dia.-FT|or I.D.| ZL  |Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem  |Ch Slope| | | | | SF Ave | HF |SE Dpth|Froude N|Norm Dp | "N" | X-Fall| ZR  |Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1086.891 1648.286 .988 1649.274 8.94 5.78 .52 1649.79 .00 1.07 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|.243|.0435| | | | |.0061|.00|.99|1.16|.59|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1087.134 1648.297 1.026 1649.323 8.94 5.51 .47 1649.79 .00 1.07 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|.076|.0435| | | | |.0054|.00|1.03|1.08|.59|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1087.210 1648.300 1.067 1649.367 8.94 5.24 .43 1649.79 .55 1.07 2.00 2.000 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|JUNCT STR|.0411| | | | |.0042|.01|1.62|1.00|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1089.640 1648.400 1.498 1649.898 6.11 3.46 .19 1650.08 .00 .96 .10 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|19.810|.0050| | | | |.0031|.06|1.50|.14|1.03|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1109.450 1648.500 1.458 1649.958 6.11 3.49 .19 1650.15 1.50 .96 .50 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|JUNCT STR|.0230| | | | |.0019|.00|1.50|.33|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1111.630 1648.550 1.694 1650.244 3.06 1.73 .05 1650.29 .00 .67 .00 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|2.696|.0729| | | | |.0008|.00|1.69|.00|.33|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1114.326 1648.747 1.500 1650.247 3.06 1.73 .05 1650.29 .00 .67 .00 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|1.862|.0729| | | | |.0008|.00|1.50|.00|.33|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1116.188 1648.882 1.360 1650.242 3.06 1.82 .05 1650.29 .00 .67 .87 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|1.038|.0729| | | | |.0008|.00|1.36|.23|.33|.013|.00|.00|PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1117.227 1648.958 1.280 1650.238 3.06 1.90 .06 1650.29 .00 .67 1.06 1.500 .000 .00 1 .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|.852|.0729| | | | |.0008|.00|1.28|.27|.33|.013|.00|.00|PIPE

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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:46:49

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3B

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1118.079 | 1649.020 | 1.213 | 1650.233 | 3.06 | 2.00 | .06 | 1650.30 | .00 | .67 | 1.18 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|.720|.0729| | | | | | | | | | | | | | | |
|.0009|.00|1.21|.31|.33|.013|.00|.00|PIPE
1118.798 | 1649.073 | 1.155 | 1650.228 | 3.06 | 2.10 | .07 | 1650.30 | .00 | .67 | 1.26 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|.643|.0729| | | | | | | | | | | | | | | |
|.0010|.00|1.16|.34|.33|.013|.00|.00|PIPE
1119.441 | 1649.120 | 1.102 | 1650.222 | 3.06 | 2.20 | .07 | 1650.30 | .00 | .67 | 1.32 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|.029|.0729| | | | | | | | | | | | | | | |
|.0011|.00|1.10|.38|.33|.013|.00|.00|PIPE
1119.470 | 1649.122 | 1.102 | 1650.224 | 3.06 | 2.20 | .08 | 1650.30 | .00 | .67 | 1.32 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|HYDRAULIC JUMP| | | | | | | | | | | | | | | |
1119.470 | 1649.122 | .367 | 1649.489 | 3.06 | 9.13 | 1.29 | 1650.78 | .00 | .67 | 1.29 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|.935|.0729| | | | | | | | | | | | | | | |
|.0460|.04|.37|3.15|.33|.013|.00|.00|PIPE
1120.405 | 1649.190 | .380 | 1649.570 | 3.06 | 8.69 | 1.17 | 1650.74 | .00 | .67 | 1.30 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|2.853|.0729| | | | | | | | | | | | | | | |
|.0401|.11|.38|2.95|.33|.013|.00|.00|PIPE
1123.258 | 1649.398 | .393 | 1649.791 | 3.06 | 8.28 | 1.06 | 1650.86 | .00 | .67 | 1.32 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|2.217|.0729| | | | | | | | | | | | | | | |
|.0351|.08|.39|2.76|.33|.013|.00|.00|PIPE
1125.475 | 1649.559 | .406 | 1649.965 | 3.06 | 7.90 | .97 | 1650.93 | .00 | .67 | 1.33 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|1.731|.0729| | | | | | | | | | | | | | | |
|.0307|.05|.41|2.58|.33|.013|.00|.00|PIPE
1127.205 | 1649.685 | .421 | 1650.106 | 3.06 | 7.53 | .88 | 1650.99 | .00 | .67 | 1.35 | 1.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
|1.435|.0729| | | | | | | | | | | | | | | |
|.0269|.04|.42|2.42|.33|.013|.00|.00|PIPE

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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 9:46:49

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3B

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1128.640	1649.790	.435	1650.225	3.06	7.18	.80	1651.03	.00	.67	1.36	1.500	.000	.00	1	.0
	1.169	.0729				.0235	.03	.44	2.26	.33	.013	.00	.00	PIPE	
1129.809	1649.875	.450	1650.325	3.06	6.84	.73	1651.05	.00	.67	1.37	1.500	.000	.00	1	.0
	.958	.0729				.0206	.02	.45	2.12	.33	.013	.00	.00	PIPE	
1130.768	1649.945	.466	1650.411	3.06	6.53	.66	1651.07	.00	.67	1.39	1.500	.000	.00	1	.0
	.786	.0729				.0180	.01	.47	1.98	.33	.013	.00	.00	PIPE	
1131.554	1650.002	.483	1650.485	3.06	6.22	.60	1651.09	.00	.67	1.40	1.500	.000	.00	1	.0
	.660	.0729				.0158	.01	.48	1.85	.33	.013	.00	.00	PIPE	
1132.213	1650.050	.500	1650.550	3.06	5.93	.55	1651.10	.00	.67	1.41	1.500	.000	.00	1	.0
	.553	.0729				.0138	.01	.50	1.73	.33	.013	.00	.00	PIPE	
1132.767	1650.090	.517	1650.608	3.06	5.66	.50	1651.10	.00	.67	1.43	1.500	.000	.00	1	.0
	.431	.0729				.0121	.01	.52	1.62	.33	.013	.00	.00	PIPE	
1133.197	1650.122	.536	1650.658	3.06	5.39	.45	1651.11	.00	.67	1.44	1.500	.000	.00	1	.0
	.354	.0729				.0106	.00	.54	1.51	.33	.013	.00	.00	PIPE	
1133.552	1650.148	.555	1650.703	3.06	5.14	.41	1651.11	.00	.67	1.45	1.500	.000	.00	1	.0
	.273	.0729				.0093	.00	.56	1.41	.33	.013	.00	.00	PIPE	
1133.824	1650.168	.575	1650.743	3.06	4.90	.37	1651.12	.00	.67	1.46	1.500	.000	.00	1	.0
	.200	.0729				.0082	.00	.58	1.32	.33	.013	.00	.00	PIPE	

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 2-3B

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs	Wth Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1134.024	1650.182	.596	1650.778	3.06	4.67	.34	1651.12	.00	.67	1.47	1.500	.000	.00	1	.0
	.150					.0072	.00	.60	1.23	.33	.013	.00	.00	PIPE	
1134.174	1650.193	.617	1650.810	3.06	4.46	.31	1651.12	.00	.67	1.48	1.500	.000	.00	1	.0
	.076					.0063	.00	.62	1.15	.33	.013	.00	.00	PIPE	
1134.250	1650.198	.640	1650.839	3.06	4.25	.28	1651.12	.00	.67	1.48	1.500	.000	.00	1	.0
	.020					.0055	.00	.64	1.08	.33	.013	.00	.00	PIPE	
1134.270	1650.200	.665	1650.865	3.06	4.04	.25	1651.12	.00	.67	1.49	1.500	.000	.00	1	.0

20-750 MERIDIAN UPPER PLATEAU

100 YEAR STORM

LINE 16-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1000.000 | 1671.000 | 1.584 | 1672.584 | 29.64 | 9.04 | 1.27 | 1673.85 | .00 | 1.86 | 2.41 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
15.150 | .0099 | | | | | .0098 | .15 | 1.58 | 1.37 | 1.58 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1015.150 | 1671.150 | 1.585 | 1672.735 | 29.64 | 9.03 | 1.27 | 1674.00 | .10 | 1.86 | 2.41 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
47.120 | .0100 | | | | | .0096 | .45 | 1.69 | 1.36 | 1.58 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1062.270 | 1671.620 | 1.603 | 1673.224 | 29.64 | 8.91 | 1.23 | 1674.46 | .00 | 1.86 | 2.40 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
22.718 | .0101 | | | | | .0093 | .21 | 1.60 | 1.33 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1084.988 | 1671.849 | 1.627 | 1673.476 | 29.64 | 8.76 | 1.19 | 1674.67 | .00 | 1.86 | 2.38 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
24.819 | .0101 | | | | | .0086 | .21 | 1.63 | 1.30 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1109.807 | 1672.099 | 1.698 | 1673.797 | 29.64 | 8.35 | 1.08 | 1674.88 | .00 | 1.86 | 2.33 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
9.600 | .0101 | | | | | .0076 | .07 | 1.70 | 1.19 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1119.407 | 1672.196 | 1.773 | 1673.969 | 29.64 | 7.96 | .98 | 1674.95 | .00 | 1.86 | 2.27 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
2.413 | .0101 | | | | | .0068 | .02 | 1.77 | 1.10 | 1.57 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1121.820 | 1672.220 | 1.856 | 1674.076 | 29.64 | 7.59 | .89 | 1674.97 | .00 | 1.86 | 2.19 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR | .0214 | | | | | .0046 | .02 | 1.92 | 1.00 | | | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1126.490 | 1672.320 | 2.598 | 1674.918 | 21.34 | 4.35 | .29 | 1675.21 | .00 | 1.57 | .00 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
49.304 | .0056 | | | | | .0027 | .13 | .00 | .00 | 1.54 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1175.794 | 1672.594 | 2.500 | 1675.094 | 21.34 | 4.35 | .29 | 1675.39 | 2.50 | 1.57 | .00 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
47.756 | .0056 | | | | | .0025 | .12 | 2.50 | .00 | 1.54 | .013 | .00 | .00 | PIPE
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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:37:25

20-750 MERIDIAN UPPER PLATEAU

100 YEAR STORM

LINE 16-1

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*****
| Invert | Depth | Water | Q | Vel | Vel | Energy | Super | Critical | Flow Top | Height/ | Base Wt | | No Wth
Station | Elev | (FT) | Elev | (CFS) | (FPS) | Head | Grd.El. | Elev | Depth | Width | Dia.-FT | or I.D. | ZL | Prs/Pip
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
L/Elem | Ch Slope | | | | | SF Ave | HF | SE Dpth | Froude N | Norm Dp | "N" | X-Fall | ZR | Type Ch
*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|*****|
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1223.550 | 1672.860 | 2.336 | 1675.196 | 21.34 | 4.47 | .31 | 1675.51 | .00 | 1.57 | 1.24 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
JUNCT STR | .0214 | | | | | .0020 | .01 | 2.34 | .40 | | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1228.230 | 1672.960 | 2.503 | 1675.463 | 16.48 | 3.36 | .18 | 1675.64 | .00 | 1.37 | .00 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
.705 | .0069 | | | | | .0016 | .00 | .00 | .00 | 1.23 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1228.935 | 1672.965 | 2.500 | 1675.465 | 16.48 | 3.36 | .18 | 1675.64 | 2.50 | 1.37 | .00 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
39.406 | .0069 | | | | | .0015 | .06 | 2.50 | .00 | 1.23 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1268.341 | 1673.237 | 2.268 | 1675.505 | 16.48 | 3.52 | .19 | 1675.70 | .00 | 1.37 | 1.45 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
21.224 | .0069 | | | | | .0015 | .03 | 2.27 | .35 | 1.23 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1289.565 | 1673.384 | 2.133 | 1675.517 | 16.48 | 3.69 | .21 | 1675.73 | .00 | 1.37 | 1.77 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
16.842 | .0069 | | | | | .0016 | .03 | 2.13 | .41 | 1.23 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1306.406 | 1673.500 | 2.022 | 1675.523 | 16.48 | 3.87 | .23 | 1675.76 | .00 | 1.37 | 1.97 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
14.243 | .0069 | | | | | .0017 | .02 | 2.02 | .46 | 1.23 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1320.650 | 1673.599 | 1.925 | 1675.524 | 16.48 | 4.06 | .26 | 1675.78 | .00 | 1.37 | 2.10 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
12.516 | .0069 | | | | | .0019 | .02 | 1.93 | .52 | 1.23 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1333.166 | 1673.686 | 1.837 | 1675.523 | 16.48 | 4.26 | .28 | 1675.80 | .00 | 1.37 | 2.21 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
10.885 | .0069 | | | | | .0022 | .02 | 1.84 | .57 | 1.23 | .013 | .00 | .00 | PIPE
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
1344.051 | 1673.761 | 1.757 | 1675.518 | 16.48 | 4.47 | .31 | 1675.83 | .00 | 1.37 | 2.29 | 2.500 | .000 | .00 | 1 | .0
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
9.553 | .0069 | | | | | .0024 | .02 | 1.76 | .62 | 1.23 | .013 | .00 | .00 | PIPE
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Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:37:25

20-750 MERIDIAN UPPER PLATEAU

100 YEAR STORM

LINE 16-1

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1353.604	1673.827	1.683	1675.510	16.48	4.69	.34	1675.85	.00	1.37	2.35	2.500	.000	.00	1	.0
8.305	.0069					.0027	.02	1.68	.67	1.23	.013	.00	.00	PIPE	
1361.908	1673.884	1.614	1675.498	16.48	4.92	.38	1675.87	.00	1.37	2.39	2.500	.000	.00	1	.0
7.121	.0069					.0031	.02	1.62	.73	1.23	.013	.00	.00	PIPE	
1369.030	1673.933	1.549	1675.483	16.48	5.16	.41	1675.90	.00	1.37	2.43	2.500	.000	.00	1	.0
2.681	.0069					.0035	.01	1.55	.79	1.23	.013	.00	.00	PIPE	
1371.711	1673.952	1.488	1675.440	16.48	5.41	.45	1675.89	.00	1.37	2.45	2.500	.000	.00	1	.0
HYDRAULIC JUMP															
1371.711	1673.952	1.225	1675.177	16.48	6.89	.74	1675.91	.00	1.37	2.50	2.500	.000	.00	1	.0
40.934	.0069					.0065	.27	1.23	1.24	1.23	.013	.00	.00	PIPE	
1412.645	1674.235	1.270	1675.505	16.48	6.58	.67	1676.18	.00	1.37	2.50	2.500	.000	.00	1	.0
10.309	.0069					.0057	.06	1.27	1.16	1.23	.013	.00	.00	PIPE	
1422.955	1674.307	1.319	1675.626	16.48	6.27	.61	1676.24	.00	1.37	2.50	2.500	.000	.00	1	.0
1.945	.0069					.0051	.01	1.32	1.08	1.23	.013	.00	.00	PIPE	
1424.900	1674.320	1.372	1675.692	16.48	5.97	.55	1676.25	.00	1.37	2.49	2.500	.000	.00	1	.0
JUNCT STR .0253															
1428.860	1674.420	1.710	1676.130	13.86	3.87	.23	1676.36	.00	1.25	2.32	2.500	.000	.00	1	.0
9.523	.0069					.0018	.02	1.71	.55	1.11	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:37:25

20-750 MERIDIAN UPPER PLATEAU

100 YEAR STORM

LINE 16-1

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1438.383	1674.486	1.638	1676.124	13.86	4.06	.26	1676.38	.00	1.25	2.38	2.500	.000	.00	1	.0
	8.324	.0069				.0021	.02	1.64	.60	1.11	.013	.00	.00	PIPE	
1446.707	1674.544	1.572	1676.116	13.86	4.26	.28	1676.40	.00	1.25	2.42	2.500	.000	.00	1	.0
	7.372	.0069				.0023	.02	1.57	.65	1.11	.013	.00	.00	PIPE	
1454.080	1674.595	1.510	1676.105	13.86	4.47	.31	1676.41	.00	1.25	2.45	2.500	.000	.00	1	.0
	6.534	.0069				.0026	.02	1.51	.70	1.11	.013	.00	.00	PIPE	
1460.613	1674.640	1.451	1676.091	13.86	4.69	.34	1676.43	.00	1.25	2.47	2.500	.000	.00	1	.0
	4.081	.0069				.0030	.01	1.45	.75	1.11	.013	.00	.00	PIPE	
1464.694	1674.668	1.396	1676.064	13.86	4.92	.38	1676.44	.00	1.25	2.48	2.500	.000	.00	1	.0
HYDRAULIC JUMP															
1464.694	1674.668	1.109	1675.777	13.86	6.59	.67	1676.45	.00	1.25	2.48	2.500	.000	.00	1	.0
	216.006	.0069				.0069	1.49	1.11	1.26	1.11	.013	.00	.00	PIPE	
1680.700	1676.162	1.109	1677.272	13.86	6.59	.67	1677.95	.00	1.25	2.48	2.500	.000	.00	1	.0
	53.120	.0069				.0069	.37	1.11	1.26	1.11	.013	.00	.00	PIPE	
1733.820	1676.530	1.112	1677.642	13.86	6.57	.67	1678.31	.00	1.25	2.48	2.500	.000	.00	1	.0
	188.806	.0069				.0069	1.30	1.12	1.26	1.11	.013	.00	.00	PIPE	
1922.626	1677.825	1.112	1678.937	13.86	6.57	.67	1679.61	.00	1.25	2.48	2.500	.000	.00	1	.0
	56.022	.0069				.0068	.38	1.12	1.26	1.11	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:37:25

20-750 MERIDIAN UPPER PLATEAU

100 YEAR STORM

LINE 16-1

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1978.649	1678.209	1.120	1679.329	13.86	6.50	.66	1679.99	.00	1.25	2.49	2.500	.000	.00	1	.0
30.014	.0069					.0063	.19	1.12	1.24	1.11	.013	.00	.00	PIPE	
2008.662	1678.415	1.162	1679.577	13.86	6.20	.60	1680.17	.00	1.25	2.49	2.500	.000	.00	1	.0
7.640	.0069					.0055	.04	1.17	1.15	1.11	.013	.00	.00	PIPE	
2016.302	1678.468	1.206	1679.674	13.86	5.91	.54	1680.22	.00	1.25	2.50	2.500	.000	.00	1	.0
1.808	.0069					.0048	.01	1.21	1.08	1.11	.013	.00	.00	PIPE	
2018.110	1678.480	1.253	1679.733	13.86	5.63	.49	1680.23	.00	1.25	2.50	2.500	.000	.00	1	.0
JUNCT STR	.0214					.0033	.02	1.26	1.00		.013	.00	.00	PIPE	
2022.790	1678.580	1.853	1680.433	4.75	2.69	.11	1680.54	.00	.84	.00	1.500	.000	.00	1	.0
18.230	.0066					.0020	.04	.00	.00	.80	.013	.00	.00	PIPE	
2041.020	1678.700	1.772	1680.472	4.75	2.69	.11	1680.58	.00	.84	.00	1.500	.000	.00	1	.0
JUNCT STR	.0488					.0013	.00	.00	.00		.013	.00	.00	PIPE	
2043.070	1678.800	1.803	1680.604	2.38	1.35	.03	1680.63	.00	.58	.00	1.500	.000	.00	1	.0
28.590	.0105					.0005	.01	1.80	.00	.48	.013	.00	.00	PIPE	
2071.660	1679.100	1.518	1680.618	2.38	1.35	.03	1680.65	.00	.58	.00	1.500	.000	.00	1	.0

WATER SURFACE PROFILE LISTING

20-750 Meridian Upper Plateau

100 Year Storm

Line 16-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1002.470	1672.320	4.330	1676.650	8.30	2.64	.11	1676.76	.00	1.03	.00	2.000	.000	.00	1 .0
96.390	.0203					.0013	.13	4.33	.00	.69	.013	.00	.00	PIPE
1098.860	1674.280	2.500	1676.780	8.30	2.64	.11	1676.89	.00	1.03	.00	2.000	.000	.00	1 .0
27.508	.0199					.0013	.04	.00	.00	.70	.013	.00	.00	PIPE
1126.368	1674.828	2.000	1676.828	8.30	2.64	.11	1676.94	2.00	1.03	.00	2.000	.000	.00	1 .0
5.092	.0199					.0012	.01	2.00	.00	.70	.013	.00	.00	PIPE
1131.460	1674.930	1.900	1676.830	8.30	2.69	.11	1676.94	.00	1.03	.87	2.000	.000	.00	1 .0
JUNCT STR	.0214					.0008	.00	1.90	.25		.013	.00	.00	PIPE
1136.130	1675.030	1.916	1676.946	5.30	1.71	.05	1676.99	.00	.81	.80	2.000	.000	.00	1 .0
6.376	.0214					.0005	.00	1.92	.15	.54	.013	.00	.00	PIPE
1142.506	1675.167	1.778	1676.945	5.30	1.80	.05	1676.99	.00	.81	1.26	2.000	.000	.00	1 .0
4.547	.0214					.0005	.00	1.78	.21	.54	.013	.00	.00	PIPE
1147.053	1675.264	1.678	1676.942	5.30	1.88	.06	1677.00	.00	.81	1.47	2.000	.000	.00	1 .0
3.814	.0214					.0006	.00	1.68	.24	.54	.013	.00	.00	PIPE
1150.867	1675.345	1.593	1676.939	5.30	1.97	.06	1677.00	.00	.81	1.61	2.000	.000	.00	1 .0
3.317	.0214					.0006	.00	1.59	.27	.54	.013	.00	.00	PIPE
1154.183	1675.416	1.518	1676.934	5.30	2.07	.07	1677.00	.00	.81	1.71	2.000	.000	.00	1 .0
2.961	.0214					.0007	.00	1.52	.30	.54	.013	.00	.00	PIPE

20-750 Meridian Upper Plateau

100 Year Storm

Line 16-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1157.144	1675.480	1.450	1676.930	5.30	2.17	.07	1677.00	.00	.81	1.79	2.000	.000	.00	1	.0
	2.697	.0214				.0008	.00	1.45	.33	.54	.013	.00	.00	PIPE	
1159.841	1675.537	1.387	1676.925	5.30	2.28	.08	1677.01	.00	.81	1.84	2.000	.000	.00	1	.0
	2.431	.0214				.0009	.00	1.39	.36	.54	.013	.00	.00	PIPE	
1162.272	1675.589	1.329	1676.919	5.30	2.39	.09	1677.01	.00	.81	1.89	2.000	.000	.00	1	.0
	2.208	.0214				.0010	.00	1.33	.39	.54	.013	.00	.00	PIPE	
1164.480	1675.637	1.275	1676.912	5.30	2.51	.10	1677.01	.00	.81	1.92	2.000	.000	.00	1	.0
	2.030	.0214				.0011	.00	1.28	.42	.54	.013	.00	.00	PIPE	
1166.510	1675.680	1.224	1676.904	5.30	2.63	.11	1677.01	.00	.81	1.95	2.000	.000	.00	1	.0
	1.847	.0214				.0012	.00	1.22	.46	.54	.013	.00	.00	PIPE	
1168.357	1675.720	1.176	1676.896	5.30	2.76	.12	1677.01	.00	.81	1.97	2.000	.000	.00	1	.0
	.130	.0214				.0013	.00	1.18	.49	.54	.013	.00	.00	PIPE	
1168.487	1675.723	1.176	1676.899	5.30	2.76	.12	1677.02	.00	.81	1.97	2.000	.000	.00	1	.0
HYDRAULIC JUMP															
1168.487	1675.723	.541	1676.264	5.30	7.72	.93	1677.19	.00	.81	1.78	2.000	.000	.00	1	.0
	100.740	.0214				.0214	2.16	.54	2.19	.54	.013	.00	.00	PIPE	
1269.227	1677.878	.541	1678.419	5.30	7.72	.93	1679.34	.00	.81	1.78	2.000	.000	.00	1	.0
	49.109	.0214				.0205	1.01	.54	2.19	.54	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:40:23

20-750 Meridian Upper Plateau

100 Year Storm

Line 16-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1318.336	1678.929	.552	1679.481	5.30	7.49	.87	1680.35	.00	.81	1.79	2.000	.000	.00	1	.0
20.075	.0214					.0184	.37	.55	2.10	.54	.013	.00	.00	PIPE	
1338.411	1679.358	.572	1679.931	5.30	7.14	.79	1680.72	.00	.81	1.81	2.000	.000	.00	1	.0
10.100	.0214					.0161	.16	.57	1.97	.54	.013	.00	.00	PIPE	
1348.511	1679.574	.591	1680.166	5.30	6.81	.72	1680.89	.00	.81	1.83	2.000	.000	.00	1	.0
6.121	.0214					.0141	.09	.59	1.84	.54	.013	.00	.00	PIPE	
1354.632	1679.705	.612	1680.318	5.30	6.50	.66	1680.97	.00	.81	1.84	2.000	.000	.00	1	.0
4.162	.0214					.0124	.05	.61	1.72	.54	.013	.00	.00	PIPE	
1358.794	1679.795	.634	1680.429	5.30	6.19	.60	1681.02	.00	.81	1.86	2.000	.000	.00	1	.0
3.044	.0214					.0108	.03	.63	1.61	.54	.013	.00	.00	PIPE	
1361.838	1679.860	.656	1680.516	5.30	5.90	.54	1681.06	.00	.81	1.88	2.000	.000	.00	1	.0
2.202	.0214					.0095	.02	.66	1.51	.54	.013	.00	.00	PIPE	
1364.041	1679.907	.679	1680.586	5.30	5.63	.49	1681.08	.00	.81	1.89	2.000	.000	.00	1	.0
1.586	.0214					.0083	.01	.68	1.41	.54	.013	.00	.00	PIPE	
1365.626	1679.941	.703	1680.644	5.30	5.37	.45	1681.09	.00	.81	1.91	2.000	.000	.00	1	.0
1.040	.0214					.0073	.01	.70	1.32	.54	.013	.00	.00	PIPE	
1366.667	1679.963	.729	1680.692	5.30	5.12	.41	1681.10	.00	.81	1.93	2.000	.000	.00	1	.0
.732	.0214					.0064	.00	.73	1.23	.54	.013	.00	.00	PIPE	

20-750 Meridian Upper Plateau

100 Year Storm

Line 16-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1367.399	1679.979	.755	1680.734	5.30	4.88	.37	1681.10	.00	.81	1.94	2.000	.000	.00	1	.0
	.419	.0214				.0056	.00	.76	1.15	.54	.013	.00	.00	PIPE	
1367.818	1679.988	.782	1680.770	5.30	4.65	.34	1681.11	.00	.81	1.95	2.000	.000	.00	1	.0
	.112	.0214				.0049	.00	.78	1.07	.54	.013	.00	.00	PIPE	
1367.930	1679.990	.812	1680.802	5.30	4.43	.30	1681.11	.00	.81	1.96	2.000	.000	.00	1	.0
JUNCT STR	.0214					.0027	.01	.81	1.00		.013	.00	.00	PIPE	
1372.600	1680.090	1.105	1681.195	2.78	1.99	.06	1681.26	.00	.63	1.32	1.500	.000	.00	1	.0
	2.127	.0207				.0009	.00	1.11	.34	.44	.013	.00	.00	PIPE	
1374.727	1680.134	1.057	1681.191	2.78	2.09	.07	1681.26	.00	.63	1.37	1.500	.000	.00	1	.0
	1.941	.0207				.0010	.00	1.06	.37	.44	.013	.00	.00	PIPE	
1376.668	1680.174	1.012	1681.186	2.78	2.19	.07	1681.26	.00	.63	1.41	1.500	.000	.00	1	.0
	1.766	.0207				.0012	.00	1.01	.41	.44	.013	.00	.00	PIPE	
1378.433	1680.211	.970	1681.181	2.78	2.30	.08	1681.26	.00	.63	1.43	1.500	.000	.00	1	.0
	1.586	.0207				.0013	.00	.97	.44	.44	.013	.00	.00	PIPE	
1380.019	1680.244	.931	1681.175	2.78	2.41	.09	1681.27	.00	.63	1.46	1.500	.000	.00	1	.0
	1.402	.0207				.0015	.00	.93	.48	.44	.013	.00	.00	PIPE	
1381.421	1680.273	.895	1681.168	2.78	2.53	.10	1681.27	.00	.63	1.47	1.500	.000	.00	1	.0
	.179	.0207				.0017	.00	.90	.52	.44	.013	.00	.00	PIPE	



20-750 Meridian Upper Plateau

100 Year Storm

Line 16-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1381.600	1680.276	.860	1681.137	2.78	2.65	.11	1681.25	.00	.63	1.48	1.500	.000	.00	1	.0
HYDRAULIC JUMP															
1381.600	1680.276	.436	1680.712	2.78	6.52	.66	1681.37	.00	.63	1.36	1.500	.000	.00	1	.0
306.651	.0207					.0207	6.36	.44	2.06	.44	.013	.00	.00	PIPE	
1688.251	1686.636	.436	1687.071	2.78	6.52	.66	1687.73	.00	.63	1.36	1.500	.000	.00	1	.0
34.491	.0207					.0213	.73	.44	2.06	.44	.013	.00	.00	PIPE	
1722.742	1687.351	.429	1687.780	2.78	6.65	.69	1688.47	.00	.63	1.36	1.500	.000	.00	1	.0
20.698	.0207					.0234	.48	.43	2.11	.44	.013	.00	.00	PIPE	
1743.440	1687.780	.415	1688.195	2.78	6.97	.75	1688.95	.03	.63	1.34	1.500	.000	.00	1	.0
22.287	.0250					.0250	.56	.45	2.25	.42	.013	.00	.00	PIPE	
1765.727	1688.336	.415	1688.752	2.78	6.97	.75	1689.51	.03	.63	1.34	1.500	.000	.00	1	.0
34.979	.0250					.0239	.84	.45	2.25	.42	.013	.00	.00	PIPE	
1800.706	1689.209	.424	1689.633	2.78	6.76	.71	1690.34	.03	.63	1.35	1.500	.000	.00	1	.0
14.264	.0250					.0215	.31	.46	2.16	.42	.013	.00	.00	PIPE	
1814.969	1689.565	.439	1690.004	2.78	6.45	.65	1690.65	.03	.63	1.37	1.500	.000	.00	1	.0
7.101	.0250					.0188	.13	.47	2.02	.42	.013	.00	.00	PIPE	
1822.070	1689.742	.454	1690.196	2.78	6.15	.59	1690.78	.03	.63	1.38	1.500	.000	.00	1	.0
4.396	.0250					.0165	.07	.48	1.89	.42	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:40:23

20-750 Meridian Upper Plateau

100 Year Storm

Line 16-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Prs/Pip	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type	Ch
1826.466	1689.852	.470	1690.322	2.78	5.86	.53	1690.86	.02	.63	1.39	1.500	.000	.00	1	.0
	.0250					.0144	.04	.49	1.77	.42	.013	.00	.00	PIPE	
1829.456	1689.927	.487	1690.414	2.78	5.59	.49	1690.90	.02	.63	1.40	1.500	.000	.00	1	.0
	.0250					.0126	.03	.51	1.66	.42	.013	.00	.00	PIPE	
1831.655	1689.981	.504	1690.485	2.78	5.33	.44	1690.93	.02	.63	1.42	1.500	.000	.00	1	.0
	.0250					.0111	.02	.52	1.55	.42	.013	.00	.00	PIPE	
1833.245	1690.021	.522	1690.543	2.78	5.08	.40	1690.94	.02	.63	1.43	1.500	.000	.00	1	.0
	.0250					.0097	.01	.54	1.45	.42	.013	.00	.00	PIPE	
1834.410	1690.050	.541	1690.591	2.78	4.84	.36	1690.96	.00	.63	1.44	1.500	.000	.00	1	.0
	.0210					.0088	.00	.54	1.35	.43	.013	.00	.00	PIPE	
1834.823	1690.059	.547	1690.606	2.78	4.76	.35	1690.96	.00	.63	1.44	1.500	.000	.00	1	.0
	.0210					.0081	.01	.55	1.32	.43	.013	.00	.00	PIPE	
1835.751	1690.078	.567	1690.645	2.78	4.54	.32	1690.96	.00	.63	1.45	1.500	.000	.00	1	.0
	.0210					.0071	.00	.57	1.23	.43	.013	.00	.00	PIPE	
1836.332	1690.090	.588	1690.678	2.78	4.33	.29	1690.97	.00	.63	1.46	1.500	.000	.00	1	.0
	.0210					.0062	.00	.59	1.15	.43	.013	.00	.00	PIPE	
1836.699	1690.098	.609	1690.707	2.78	4.12	.26	1690.97	.00	.63	1.47	1.500	.000	.00	1	.0
	.0210					.0055	.00	.61	1.07	.43	.013	.00	.00	PIPE	

Program Package License Serial Number: 1232

WATER SURFACE PROFILE LISTING

Date: 9-14-2021 Time: 8:40:23

20-750 Meridian Upper Plateau

100 Year Storm

Line 16-2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
1836.790	1690.100	.633	1690.733	2.78	3.92	.24	1690.97	.00	.63	1.48	1.500	.000	.00	1 .0

***APPENDIX E***

Catch Basin Efficiency Calculations (FlowMaster)

## Worksheet for Catch Basin 21

### Project Description

Solve For                      Spread

### Input Data

Discharge		4.14	ft <sup>3</sup> /s
Gutter Width		2.00	ft
Gutter Cross Slope		0.06	ft/ft
Road Cross Slope		0.02	ft/ft
Curb Opening Length		7.00	ft
Opening Height		0.67	ft
Curb Throat Type	Inclined		
Local Depression		2.00	in
Local Depression Width		4.00	ft
Throat Incline Angle		45.00	degrees

### Results

Spread		12.62	ft
Depth		0.34	ft
Gutter Depression		0.09	ft
Total Depression		0.25	ft

---

## Worksheet for Catch Basin 22

---

### Project Description

Solve For                              Spread

### Input Data

Discharge		4.14	ft <sup>3</sup> /s
Gutter Width		2.00	ft
Gutter Cross Slope		0.06	ft/ft
Road Cross Slope		0.02	ft/ft
Curb Opening Length		7.00	ft
Opening Height		0.67	ft
Curb Throat Type	Inclined		
Local Depression		2.00	in
Local Depression Width		4.00	ft
Throat Incline Angle		45.00	degrees

### Results

Spread		12.62	ft
Depth		0.34	ft
Gutter Depression		0.09	ft
Total Depression		0.25	ft

---

## Worksheet for Catch basin 23-24

---

### Project Description

Solve For                                  Spread

### Input Data

Discharge		2.38	ft <sup>3</sup> /s
Gutter Width		2.00	ft
Gutter Cross Slope		0.06	ft/ft
Road Cross Slope		0.02	ft/ft
Curb Opening Length		7.00	ft
Opening Height		0.67	ft
Curb Throat Type	Inclined		
Local Depression		2.00	in
Local Depression Width		4.00	ft
Throat Incline Angle		45.00	degrees

### Results

Spread		8.72	ft
Depth		0.25	ft
Gutter Depression		0.08	ft
Total Depression		0.25	ft

---

## Worksheet for Catch Basin 1-4

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	2.41	ft <sup>3</sup> /s
Slope	0.00400	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	92.60	%
Intercepted Flow	2.23	ft <sup>3</sup> /s
Bypass Flow	0.18	ft <sup>3</sup> /s
Spread	10.35	ft
Depth	0.29	ft
Flow Area	1.16	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.08	ft/s
Equivalent Cross Slope	0.05294	ft/ft
Length Factor	0.76	
Total Interception Length	13.08	ft



---

## Worksheet for Catch Basin 7-8

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	0.99	ft <sup>3</sup> /s
Slope	0.01500	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	98.90	%
Intercepted Flow	0.98	ft <sup>3</sup> /s
Bypass Flow	0.01	ft <sup>3</sup> /s
Spread	4.88	ft
Depth	0.18	ft
Flow Area	0.32	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	3.06	ft/s
Equivalent Cross Slope	0.07464	ft/ft
Length Factor	0.92	
Total Interception Length	10.89	ft

---

## Worksheet for Catch Basin 5-6

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### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.18	ft <sup>3</sup> /s
Slope	0.01100	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	98.42	%
Intercepted Flow	1.16	ft <sup>3</sup> /s
Bypass Flow	0.02	ft <sup>3</sup> /s
Spread	5.87	ft
Depth	0.20	ft
Flow Area	0.43	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.75	ft/s
Equivalent Cross Slope	0.06991	ft/ft
Length Factor	0.90	
Total Interception Length	11.11	ft

---

## Worksheet for Catch Basin 9-10

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.33	ft <sup>3</sup> /s
Slope	0.01800	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	91.76	%
Intercepted Flow	1.22	ft <sup>3</sup> /s
Bypass Flow	0.11	ft <sup>3</sup> /s
Spread	5.48	ft
Depth	0.19	ft
Flow Area	0.39	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	3.45	ft/s
Equivalent Cross Slope	0.07176	ft/ft
Length Factor	0.75	
Total Interception Length	13.33	ft

---

## Worksheet for Catch Basin 13-14

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	2.01	ft <sup>3</sup> /s
Slope	0.00400	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	96.56	%
Intercepted Flow	1.94	ft <sup>3</sup> /s
Bypass Flow	0.07	ft <sup>3</sup> /s
Spread	9.58	ft
Depth	0.28	ft
Flow Area	1.00	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.01	ft/s
Equivalent Cross Slope	0.05522	ft/ft
Length Factor	0.85	
Total Interception Length	11.82	ft

---

## Worksheet for Catch Basin 15-16

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### Project Description

Solve For                      Efficiency

### Input Data

Discharge	2.60	ft <sup>3</sup> /s
Slope	0.00400	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	90.71	%
Intercepted Flow	2.36	ft <sup>3</sup> /s
Bypass Flow	0.24	ft <sup>3</sup> /s
Spread	10.69	ft
Depth	0.30	ft
Flow Area	1.23	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.12	ft/s
Equivalent Cross Slope	0.05203	ft/ft
Length Factor	0.73	
Total Interception Length	13.64	ft

---

## Worksheet for Catch Basin 17

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	2.78	ft <sup>3</sup> /s
Slope	0.04000	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	61.89	%
Intercepted Flow	1.72	ft <sup>3</sup> /s
Bypass Flow	1.06	ft <sup>3</sup> /s
Spread	6.54	ft
Depth	0.22	ft
Flow Area	0.51	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	5.43	ft/s
Equivalent Cross Slope	0.06679	ft/ft
Length Factor	0.41	
Total Interception Length	24.10	ft

---

## Worksheet for Catch Basin 19-20

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### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.69	ft <sup>3</sup> /s
Slope	0.00500	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	97.99	%
Intercepted Flow	1.66	ft <sup>3</sup> /s
Bypass Flow	0.03	ft <sup>3</sup> /s
Spread	8.44	ft
Depth	0.25	ft
Flow Area	0.80	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.12	ft/s
Equivalent Cross Slope	0.05900	ft/ft
Length Factor	0.89	
Total Interception Length	11.29	ft

---

## Worksheet for Catch Basin 27-28

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### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.61	ft <sup>3</sup> /s
Slope	0.03000	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	79.90	%
Intercepted Flow	1.29	ft <sup>3</sup> /s
Bypass Flow	0.32	ft <sup>3</sup> /s
Spread	5.37	ft
Depth	0.19	ft
Flow Area	0.37	ft <sup>2</sup>
Gutter Depression	0.08	ft
Total Depression	0.25	ft
Velocity	4.37	ft/s
Equivalent Cross Slope	0.07094	ft/ft
Length Factor	0.59	
Total Interception Length	16.95	ft



---

## Worksheet for Catch Basin 11-12

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### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.47	ft <sup>3</sup> /s
Slope	0.01800	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	89.26	%
Intercepted Flow	1.31	ft <sup>3</sup> /s
Bypass Flow	0.16	ft <sup>3</sup> /s
Spread	5.79	ft
Depth	0.20	ft
Flow Area	0.42	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	3.50	ft/s
Equivalent Cross Slope	0.07030	ft/ft
Length Factor	0.71	
Total Interception Length	14.08	ft

---

## Worksheet for Catch Basin 18

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	0.85	ft <sup>3</sup> /s
Slope	0.04000	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	92.87	%
Intercepted Flow	0.79	ft <sup>3</sup> /s
Bypass Flow	0.06	ft <sup>3</sup> /s
Spread	3.14	ft
Depth	0.15	ft
Flow Area	0.18	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	4.62	ft/s
Equivalent Cross Slope	0.08158	ft/ft
Length Factor	0.77	
Total Interception Length	13.00	ft

## Worksheet for Catch Basin 25-26

### Project Description

Solve For                                      Efficiency

### Input Data

Discharge	1.76	ft <sup>3</sup> /s
Slope	0.00500	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	97.31	%
Intercepted Flow	1.71	ft <sup>3</sup> /s
Bypass Flow	0.05	ft <sup>3</sup> /s
Spread	8.60	ft
Depth	0.26	ft
Flow Area	0.82	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.13	ft/s
Equivalent Cross Slope	0.05845	ft/ft
Length Factor	0.87	
Total Interception Length	11.55	ft

---

## Worksheet for Catch Basin 29-30

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### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.21	ft <sup>3</sup> /s
Slope	0.03000	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	88.29	%
Intercepted Flow	1.07	ft <sup>3</sup> /s
Bypass Flow	0.14	ft <sup>3</sup> /s
Spread	4.47	ft
Depth	0.17	ft
Flow Area	0.28	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	4.25	ft/s
Equivalent Cross Slope	0.07657	ft/ft
Length Factor	0.70	
Total Interception Length	14.36	ft

---

## Worksheet for Catch Basin 35-36

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### Project Description

Solve For                                  Efficiency

### Input Data

Discharge	1.91	ft <sup>3</sup> /s
Slope	0.01100	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	87.84	%
Intercepted Flow	1.68	ft <sup>3</sup> /s
Bypass Flow	0.23	ft <sup>3</sup> /s
Spread	7.45	ft
Depth	0.23	ft
Flow Area	0.64	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.98	ft/s
Equivalent Cross Slope	0.06284	ft/ft
Length Factor	0.69	
Total Interception Length	14.50	ft

---

## Worksheet for Catch Basin 31-32

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### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.74	ft <sup>3</sup> /s
Slope	0.00400	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	98.88	%
Intercepted Flow	1.72	ft <sup>3</sup> /s
Bypass Flow	0.02	ft <sup>3</sup> /s
Spread	8.99	ft
Depth	0.26	ft
Flow Area	0.89	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	1.95	ft/s
Equivalent Cross Slope	0.05710	ft/ft
Length Factor	0.92	
Total Interception Length	10.90	ft

---

## Worksheet for Catch Basin 37-38

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.76	ft <sup>3</sup> /s
Slope	0.00500	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	97.31	%
Intercepted Flow	1.71	ft <sup>3</sup> /s
Bypass Flow	0.05	ft <sup>3</sup> /s
Spread	8.60	ft
Depth	0.26	ft
Flow Area	0.82	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	2.13	ft/s
Equivalent Cross Slope	0.05845	ft/ft
Length Factor	0.87	
Total Interception Length	11.55	ft

---

## Worksheet for Catch Basin 39-40

---

### Project Description

Solve For                      Efficiency

### Input Data

Discharge	1.34	ft <sup>3</sup> /s
Slope	0.00400	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Roughness Coefficient	0.013	
Curb Opening Length	10.00	ft
Local Depression	2.00	in
Local Depression Width	4.00	ft

### Results

Efficiency	100.00	%
Intercepted Flow	1.34	ft <sup>3</sup> /s
Bypass Flow	0.00	ft <sup>3</sup> /s
Spread	7.99	ft
Depth	0.24	ft
Flow Area	0.72	ft <sup>2</sup>
Gutter Depression	0.09	ft
Total Depression	0.25	ft
Velocity	1.85	ft/s
Equivalent Cross Slope	0.06068	ft/ft
Length Factor	1.06	
Total Interception Length	9.42	ft



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## Worksheet for Catch Basin 33-34

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### Project Description

Solve For                          Spread

### Input Data

Discharge		1.33	ft <sup>3</sup> /s
Gutter Width		2.00	ft
Gutter Cross Slope		0.06	ft/ft
Road Cross Slope		0.02	ft/ft
Curb Opening Length		7.00	ft
Opening Height		0.67	ft
Curb Throat Type	Inclined		
Local Depression		2.00	in
Local Depression Width		4.00	ft
Throat Incline Angle		45.00	degrees

### Results

Spread		3.55	ft
Depth		0.20	ft
Gutter Depression		0.09	ft
Total Depression		0.25	ft

